PROJECT NAME: 80 SOUTH BROADWAY

PROJECT ARCHITECT: DOMINICK R.PILLA ASSOCIATES PC
STRUCTURAL ENGINEER: DOMINICK R.PILLA ASSOCIATES PC

PROJECT ADDRESS: 80 SOUTH BROADWAY, NYACK, NY 10960

ISSUED: PLANNING BOARD

SUPPORT OF EXCAVATION PLANS

DATE: 01/27/2026

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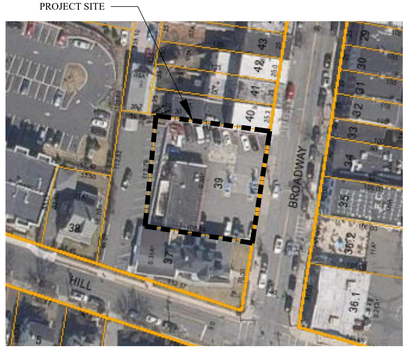
SCOPE OF WORK

SUPPORT OF EXCAVATION ASSOCIATED WITH THE CONSTRUCTION OF NEW 3 STORY PLUS CELLAR MIXED USE BUILDING. WORK TO INCLUDE REGRADING AND INSTALLATION OF SOLDIER PILES ALONG SOUTH AND EAST PROPERTY LINES.

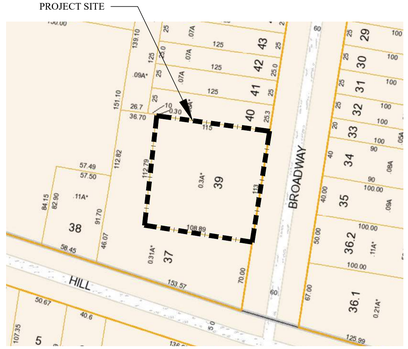
PROJECT SITE



A - VICINITY MAP
NTS

N

PROJECT SITE



B - TAX MAP
NTS

N

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ABBREVIATION

AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
B	BOTTOM REINFORCEMENT
BM	BEAM
BS	BOTH SIDES
BU	BUILT UP MEMBER
BW	BEARING WALL
C	COMPRESSION FORCE IN KIPS
CANT.	CANTILEVER
CL	CENTER LINE
CG	CENTER OF GRAVITY
COL	COLUMN
CONT	CONTINUOUS
COMP LAP	COMPRESSION REINF LAP SLICE
CP	COMPLETE PENETRATION WELD
DB	DEVELOPMENT LENGTH OF REINFORCEMENT BAR
DEL	DELTA OR CHANGE IN ELEVATION
(E)	EXISTING CONSTRUCTION
EF	EACH FACE
EL	ELEVATION
EW	EACH WAY
F	FINISHED SURFACE
GB	GRADE BEAM
H	HORIZONTAL REINFORCEMENT
H	HORIZONTAL FORCE IN KIPS
J1,J2	NEW CODE FORMED STEEL JOISTS
LAP	FULL TENSION CAPACITY LAP SPLICE
LD	TENSION DEVELOPMENT LENGTH FOR REINFORCING BARS
LDC	COMPRESSION SPLICE LENGTH FOR REINFORCEMENT BARS
LLBB	LONG LEGS BACK-TO-BACK
LW	LIGHTWEIGHT CONCRETE
M	BENDING MOMENT IN FOOT-KIPS
MC	MOMENT CONNECTION SHOWN ON DRAWING
MIN	MINIMUM
(N)	NEW CONSTRUCTION
N	BEARING BOLTS THREADS INCLUDED IN SHEAR PLANE
NTS	NOT TO SCALE
OC	ON CENTER
PC	PILE CAP
PL	PLATE
PP	PARTIAL PENETRATION WELD
PL	PROPERTY LINE
SAD	SEE ARCHITECTURAL DRAWINGS/DETAILS
S1,S2	SLAB ON DECK TYPE
SC	SLIP CRITICAL BOLT
SIM	SIMILAR
SPW	SOLDIER PILE LAGGING WALL
SW	SHEAR WALL
T	TENSION FORCE IN KIPS
T	THICKNESS
T	TOP REINFORCEMENT
TBC	TO BE CONFIRMED
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOS	TOP OF STEEL
TYP	TYPICAL
UNO,UON	UNLESS OTHERWISE NOTED
M	MOMENT
V	VERTICAL BEAM END REACTION IN KIPS
VIF	VERIFY IN FIELD
WP	WORKPOINT
WWF	WELDED WIRE FABRIC

SYMBOL LEGEND

SECTION # / XX / SHT # / SHEET #	SECTION SYMBOL
DETAIL # / XX / SHT# / SHEET #	DETAIL REFERENCE SYMBOL
ELEVATION # / A / SHT # / SHEET #	ELEVATION REFERENCE SYMBOL
TITLE FOR PLAN, SECTION OR DETAIL # / XX / SHT # / IF SHOWN POINT TO SHEET WHERE SECTION OR DETAIL WAS TAKEN	SECTION, OR DETAIL TITLE SYMBOL
XXX	ELEVATION SYMBOL
◄ ►	BEAM MOMENT CONNECTION
	POST UP/POST DOWN
LT-#	PRECAST LINTEL
GB-#	CONCRETE GRADE BEAM
F-#.#	FOOTING NUMBER
MW-#	MASONRY WALL NUMBER
CB-#	CONCRETE BEAM
CP-#	CONCRETE PIER
CW-#	CONCRETE WALL NUMBER

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REVISIONS:

NO.	DATE	DESCRIPTION

PROJECT:
80 S BROADWAY
NYACK, NY

TITLE SHEET

SEAL AND SIGNATURE:

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NY P.E. 074213-1 NY R.A. 027028-1

ISSUE:	PLANNING BOARD
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DRAWN/CHK BY:	CE/SM
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SOE-001.00

GENERAL NOTES:

1. ALL ELEVATIONS ARE REFERENCED TO THE BOROUGH OF THE BRONX HIGHWAY DATUM, WHICH IS 2.608 FEET ABOVE U.S.C. AND THE NATIONAL GEODETIC SURVEY VERTICAL DATUM OF 1929 (NGVD29), MEAN SEA LEVEL, SANDY HOOK, NEW JERSEY.
2. PROPOSED FINISHED FIRST FLOOR ELEVATION IS 59.5' (NAVD88) AND CELLAR ELEVATION IS 48.25'.
3. SURVEY DRAWING BY ROBER SORACE OF NEW CITY, NY DATED SEPTEMBER 15, 2023.
4. LOCATIONS AND ELEVATIONS OF ALL STRUCTURAL BUILDING ELEMENTS SHOWN ON THIS DRAWING MAY BE APPROXIMATE AND SHALL BE SUPERSEDED BY FINAL STRUCTURAL AND ARCHITECTURAL DRAWINGS.
5. IT IS THE CONTRACTOR'S RESPONSIBILITY TO LOCATE UTILITIES AND BELOW GROUND STRUCTURES IN THE AREA PRIOR TO COMMENCEMENT OF WORK.
6. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS IN THE FIELD. IF THE CONDITIONS OBSERVED IN THE FIELD DIFFER FROM THESE DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER TO EVALUATE THE CONDITION. MODIFICATIONS TO THESE DRAWINGS MAY BE NECESSARY.
7. THESE DRAWINGS DO NOT ADDRESS SAFETY ISSUES RELATED TO THE EXCAVATION AND SHORING WORK. OTHERS SHALL BE RESPONSIBLE FOR SITE SAFETY AND PROVIDE A SAFETY PLAN CONFORMING TO OSHA AND ALL APPLICABLE LAWS.
8. THE CONTRACTOR IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE AND ALL APPLICABLE LAWS MUST PROVIDE BARRIERS AND FENCING AROUND SITE.
9. IF THE CONDITIONS OBSERVED AS THE EXCAVATION ADVANCES ARE DIFFERENT THAN THE CONDITIONS SHOWN ON THE DESIGN DRAWINGS, THE CONTRACTOR SHALL STOP WORK AND NOTIFY THE CONSTRUCTION MANAGER AND ENGINEER TO ADDRESS FIELD CONDITIONS.
10. OBSERVED MOVEMENTS OF THE SUPPORT OF EXCAVATION OR OTHER STRUCTURES SHALL BE BROUGHT TO THE ATTENTION OF THE CONSTRUCTION MANAGER AND ENGINEER.
11. LOOSE AREAS OF FOUNDATION WALL OR FOOTINGS THAT ARE DAMAGED OR LOOSE SHOULD BE BROUGHT TO THE ATTENTION OF THE ENGINEER FOR EVALUATION AND REMEDIAL MEASURES BY THIS OFFICE OR AT DIRECTION OF FIELD PROFESSIONAL ENGINEER.
12. PINS, WIRE MESH, AND PARGING MAY BE REQUIRED TO STABILIZE THE FOUNDATION WALL OR FOOTINGS NOT INDICATED IN THESE DRAWINGS.
13. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AWS D1.1 USING E-70 ELECTRODES.
14. ALL STRUCTURAL STEEL SHALL BE ASTM A992, GRADE 50 OR ASTM A-572, GRADE 50.
15. ALL PLATES OR MISCELLANEOUS STEEL SHALL BE ASTM A36.
16. 1-BAG MIX SHALL CONSIST OF 1-94 LB. BAG OF CEMENT TO 1 CY OF SAND. QUANTITY OF WATER SHALL BE ADEQUATE TO ALLOW THE MIX TO FLOW.
17. THE DESIGNS OF THESE DRAWINGS ARE INTENDED FOR TEMPORARY SUPPORT OF EXCAVATION ONLY.
18. NOTIFY DOB 24 TO 48 HOURS PRIOR TO EXCAVATION (RULE 52).

CODES AND SPECIFICATIONS

THE DESIGN SHOWN ON THESE DRAWINGS IS BASED ON THE FOLLOWING CODES, SPECIFICATIONS AND STANDARDS:

1. "NEW YORK STATE BUILDING CODE," 2020 EDITION.
2. ASCE 7-10: "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES."
3. AWS D1.1: "STRUCTURAL WELDING CODE," 2011.
4. "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," ACI 318- 2011.
5. "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES," ACI 530-2008.
6. "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS," AISC 360-05,
7. "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES," AISC 303-05.
8. ASCE 37-02: "DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION."

SUPPORT OF EXCAVATION NOTES:

1. THE TEMPORARY SHEETING WALL (SUPPORT OF EXCAVATION) IS DESIGNED WITH AN ADDED ALLOWABLE SURCHARGE LOADING AT SIDEWALK GRADE AT A VALUE OF 600 POUNDS PER SQUARE FOOT (PSF). HEAVY EQUIPMENT OR MATERIAL STORAGE ANTICIPATED SHALL BE PLACED WITHIN A DISTANCE TO THE SHEETING WALL EQUAL TO THE EXCAVATION DEPTH, MUST BE EVALUATED BY THIS OFFICE FOR ACCEPTANCE PRIOR TO PLACING SAID HEAVY EQUIPMENT.
2. STRUCTURAL CONCRETE FOR UNDERPINNING PIERS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS.
3. CONCRETE PIERS AND DRY PACK SHALL BE ALLOWED TO CURE PRIOR TO EXCAVATING ADJACENT PIT, OR ADVANCING THE EXCAVATION IN FRONT OF THE PIT.
4. DRY PACK SHALL CONSIST OF ONE PART CEMENT TO TWO PARTS SAND BY VOLUME. WATER SHALL BE ADDED TO PRODUCE A MIXTURE, WHICH HOLDS ITS SHAPE WHEN FORMED INTO A BALL BY HAND.
5. GROUTING TO STABILIZE SOIL AT UNDERPINNING PITS SHALL BE PERFORMED USING SODIUM SILICATE OR MICROFINE CEMENT. GROUT MIX DESIGN, EQUIPMENT, DRILLING PROCEDURE, AND SEQUENCE SHALL BE PERFORMED BY THE CONTRACTOR AND SUBMITTED FOR REVIEW.
6. TIMBER LAGGING SHALL BE ROUGH CUT, FULL SIZE CONSTRUCTION GRADE, WITH A MINIMUM ALLOWABLE BENDING STRESS OF 1900-PSI FOR 3" & 4", 1950-PSI FOR 5". TIMBER SIZES SHOWN ARE ACTUAL SIZES.
7. DEPTH OF EXCAVATION BELOW FOOTING AND PREVIOUSLY INSTALLED LAGGING BOARDS SHALL NOT EXCEED 36 INCHES, OR AT DIRECTION FIELD PROFESSIONAL ENGINEER, MAINTAIN TIGHT CONTACT BETWEEN SOIL AND LAGGING BOARDS. IF MATERIAL IS CAVING INTO EXCAVATION, DECREASE THE UNBRACED EXCAVATION DEPTH AND /OR GROUT THE MATERIAL TO MINIMIZE LOSS.
8. IF MATERIAL BEHIND LAGGING HAS BEEN LOST OF DISTURBED, LEAVE A 1 TO 1 1/2-INCH SPACE BETWEEN LAGGING BOARDS TO IMMEDIATELY BACKFILL OR GROUT.
9. EXCAVATION FOR UNDERPINNING PIERS MUST BE PERFORMED IN DRY CONDITIONS. DEWATERING MAY BE NECESSARY PRIOR TO EXCAVATION TO MAINTAIN WATER LEVELS A MINIMUM OF 1 FOOT BELOW THE PROPOSED SUBGRADE LEVEL OF THE PIER. HAY OR FILTER FABRIC SHALL BE USED TO MINIMIZE MIGRATION OF FINES INTO THE EXCAVATION.
10. UNDERPINNING PIER SUBGRADE BEARING MATERIAL SHALL BE EQUAL OR BETTER CLASS THAN THE ORIGINAL BEARING MATERIAL.
11. MAXIMUM PIT WIDTH IS 4 FEET UNLESS OTHERWISE NOTED ON THE DRAWINGS.
12. APPROACH PITS FOR UNDERPINNING PITS SHOULD CAUSE MINIMAL DISTURBANCE TO SOIL SUBGRADE BELOW THE FOOTING. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DESIGN THE APPROACH PITS AND EXCAVATE PITS FOLLOWING OSHA AND LOCAL LAWS.
13. EXCAVATE PITS SUCH THAT A MINIMUM OF 12 FEET OF UNDISTURBED SOIL OR CURED UNDERPINNING PIER IS MAINTAINED BETWEEN OPEN PITS UNTIL ALL UNDERPINNING IS COMPLETE.
14. DO NOT LEAVE PITS OPEN OVERNIGHT OR DURING WEEKENDS OR HOLIDAYS.
15. DO NOT START UNDERPINNING WITH A CORNER OR END UNDERPINNING PIER.
16. TOP OF UNDERPINNING PIER SHALL MATCH EXISTING FOOTING THICKNESS OR 3'-0" MAX., AND BASE OF UNDERPINNING PIER THICKNESS SHALL BE 3'-0" MIN. IF FIELD CONDITIONS DO NOT ALLOW TO MEET THESE DIMENSIONS CONTACT FNA OFFICE.
17. UNDERPINNING SHALL BE CONSTRUCTED IN ONE VERTICAL LIFT, NO COLD JOINTS.
18. ROCK BOLTS MAY BE REQUIRED BASED ON ROCK FACE OBSERVATIONS AT DIRECTION OF FIELD PROFESSIONAL ENGINEER.

DRILLED PIPE SOLDIER MICROPILES & LAGGING:

1. SOLDIER PILE CASING SHALL BE INSTALLED USING INTERNAL FLUSH DUPLEX DRILLING METHOD. CONTRACTOR SHALL ADJUST DRILLING PROCEDURE AS REQUIRED TO PREVENT LOSS OF GROUND, SETTLEMENT AND/OR LATERAL MOVEMENT OF BUILDINGS, UTILITIES, AND OTHER STRUCTURES.
2. NO LOSS OF MATERIAL FROM OUTSIDE OF THE SOLDIER PILE WILL BE PERMITTED. THE CONTRACTOR SHALL ADOPT THE NECESSARY DRILLING PROCEDURES TO PREVENT LOSS OF MATERIAL FROM AROUND THE OUTSIDE OF SOLDIER PILE.
3. STEEL CASING SHALL HAVE A MINIMUM WALL THICKNESS OF 0.50-INCHES. SPLICES IN THE CASING SHALL BE THREADED AND FULLY WELDED (ADDITIONAL INTERNAL REINFORCEMENT MAY BE REQUIRED IF SEAMS ARE NOT WELDED.)
4. A HIGH-STRENGTH CUTTING SHOE WITH HARDENED CUTTING EDGE SHALL PROTECT THE BOTTOM OF EACH DRILLED SOLDIER PILE.
5. NO CONCRETE OR GROUT SHALL BE PLACED AT ANY SOLDIER PILE LOCATION UNTIL TIP ELEVATION HAS BEEN CONFIRMED, CLEANED OF MUD AND ANY EXTRANEOUS MATERIAL, AND INSPECTED AND APPROVED BY THE FIELD ENGINEER.
6. CONCRETE OR GROUT SHALL BE PLACED CONTINUOUSLY FOR THE FULL DEPTH OF THE SOLDIER PILE STARTING AT THE BOTTOM. NO COLD JOINT IS ALLOWED.
7. THE FIELD ENGINEER WILL DETERMINE THE FINAL DETERMINATION OF THE ELEVATION OF THE SOLDIER PILE TIP.
8. THE ENGINEER MAY DIRECT AN INCREASE IN SOLDIER PILE DEPTH FROM THAT SPECIFIED HEREIN OR AS SHOWN ON THE DRAWINGS IF INFERIOR SOIL IS ENCOUNTERED ABOVE THE ORIGINAL MINIMUM TIP ELEVATION.
9. NO SOLDIER PILE SHALL BE OUT OF PLUMB MORE THAN ONE PERCENT (1%) OF ITS EMBEDDED LENGTH.
10. BEFORE BRACING IS INSTALLED, MAXIMUM EXCAVATION BELOW BRACING LEVEL IS 2-FT FOR WALERS AND RAKERS UNLESS NOTED ON DRAWING OR AT DIRECTION OF FIELD ENGINEER.
11. LAGGING SHALL BE INSTALLED AS THE EXCAVATION ADVANCES WITH A MAXIMUM DEPTH OF 2-FT PRIOR TO LAGGING INSTALLATION. THE MAXIMUM DEPTH EXPOSURE MAY BE ADJUSTED DEPENDENT ON OBSERVED SOIL CONDITIONS UNDER THE REVIEW OF THE SPECIAL INSPECTOR. NO PERSON SHALL ENTER ADJACENT TO AN UNSHORED VERTICAL SOIL CUT EXCEEDING 5'-0".
12. IF MATERIAL BEHIND LAGGING HAS BEEN LOST OR DISTURBED, LEAVE A 1- TO 1-1/2 INCH SPACE BETWEEN LAGGING BOARDS TO IMMEDIATELY BACKFILL OR GROUT.
13. HAY OR FILTER FABRIC SHALL BE USED TO MINIMIZE MIGRATION OF FINES INTO THE EXCAVATION.

REINFORCED CONCRETE

1. ALL CONCRETE WORK SHALL COMPLY WITH ACI 318.
2. ALL CAST-IN-PLACE CONCRETE SHALL BE CONTROLLED CONCRETE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'c) AT 28 DAYS SHALL BE 4,000 PSI ORMAL WEIGHT CONCRETE.
3. CONCRETE REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60 DEFORMED REINFORCING STEEL.
4. PROVIDE THE FOLLOWING MINIMUM CONCRETE COVER FOR RENFORCEMENT:

   CONCRETE CAST AGAINST SOIL: 3".

   CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH SOIL: 2".

STRUCTURAL STEEL

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", WITH COMMENTARY, AND ALL OSHA REQUIREMENTS.
2. STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS, UNLESS OTHERWISE NOTED ON THE CONTRACT DOCUMENTS:

   ROLLED W SHAPES: ASTM A992, GRADE 50.

   ROLLED HP SHAPES: ASTM A572, GRADE 50.

   ROLLED M, S, C, MC AND L SHAPES: ASTM A36, FY=36 KSI.

   PLATES AND BARS: ASTM A572, GRADE 50 KSI, UON.

   HOLLOW STRUCTURAL SECTIONS:

   ROUND SECTIONS: ASTM A500, GRADE C, FY=46 KSI.

   SQUARE AND RECTANGULAR SECTIONS: ASTM A500, GRADE C, FY=50 KSI.
3. ALL STRUCTURAL STEEL CONNECTIONS BOLTS SHALL BE ASTM A325 OR ASTM A490, UNLESS OTHERWISE NOTED, AND SHALL COMPLY WITH "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS," INCLUDING COMMENTARY. ALL JOINT TYPE SHALL BE 'PT' (PRETENSIONED).
4. BOLT SIZE SHALL BE 3/4" DIAMETER MINIMUM, UNLESS OTHERWISE NOTED.
5. A MINIMUM OF TWO (2) - 3/4" DIAMETER A325 BOLTS SHALL BE PROVIDED AT EACH CONNECTIONS.
6. ANCHOR RODS SHALL BE ASTM F1554 GRADE 55 WITH WELDABILITY SUPPLEMENTARY REQUIREMENT S1, HOOKED OR ANCHOR RODS SHALL BE A449, TYPE 1, THREADED WITH NUTS AND WASHERS EACH END.
7. DO NOT SPLICE STRUCTURAL STEEL MEMBERS EXCEPT WHERE INDICATED ON THE DRAWINGS.

WELDING

1. ALL SHOP AND FIELD WELDING SHALL CONFORM TO THE AWS D1.1. "STRUCTURAL WELDING CODE."
2. WELDING ELECTRODES SHALL CONFORM TO E70XX.
3. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
4. ALL WELDERS SHALL BE LICENSED AND CERTIFIED TO AWS STANDARDS OR THOSE REQUIRED BY APPLICABLE BUILDING CODES.
5. ALL WELD SHALL BE VISUALLY INSPECTED. ALL GROOVE WELDS SHALL RECEIVE RADIOGRAPHIC OR ULTRASONIC TESTING; MAGNETIC PARTICLE TEST 20 PERCENT OF ALL FILLET WELDS.

TIMBER

FOR TIMBER NOTES SEE "SUPPORT OF EXCAVATION NOTES ABOVE.

COORDINATION NOTE:

ALL WORK TO BE PERFORMED SHALL BE COORDINATED BETWEEN THE CONTRACTOR AND APPLICABLE UTILITY COMPANIES AND/OR VILLAGE DEPARTMENTS AS REQUIRED.

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REVISIONS:

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80 S BROADWAY
NYACK, NY

GENERAL NOTES

SEAL AND SIGNATURE:

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NY P.E. 074213-1 NY R.A. 027028-1

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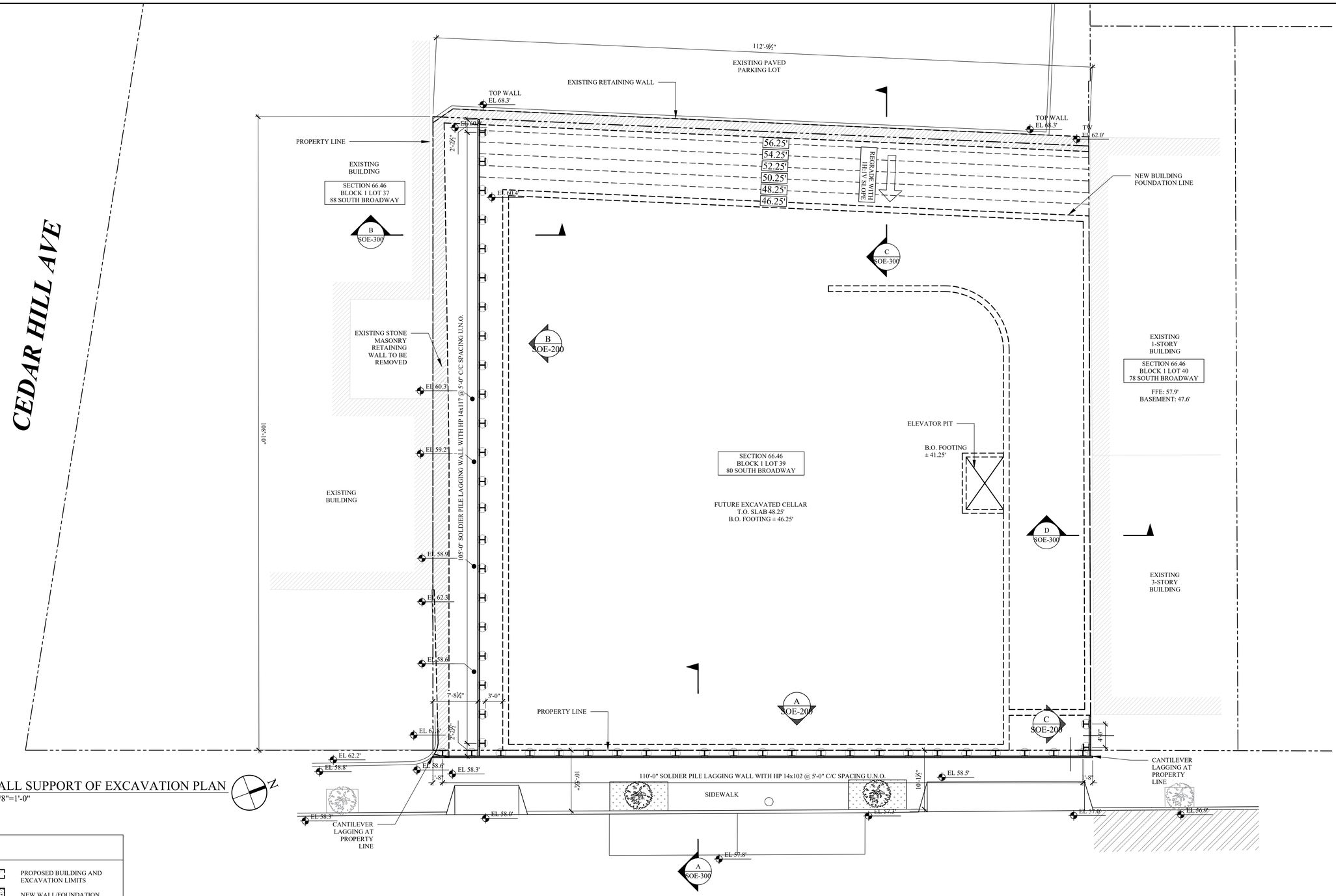

A OVERALL SUPPORT OF EXCAVATION PLAN
SCALE: 1/8"=1'-0"

LEGEND

	PROPOSED BUILDING AND EXCAVATION LIMITS
	NEW WALL/FOUNDATION
	WAILER
	SOE PILE
	PROPERTY LINE
TP-#	TEST PIT LOCATION
B-#	BORING LOCATION

NOTES:
1. CONTRACTOR SHALL OBTAIN NECESSARY PERMITS FOR THE INSTALLATION OF ALL SOLDIER PILES ON THE SIDEWALK.
2. CONTRACTOR SHALL PERFORM FULL UTILITY SCAN PRIOR TO INSTALLATION OF PILES ON SIDEWALK.

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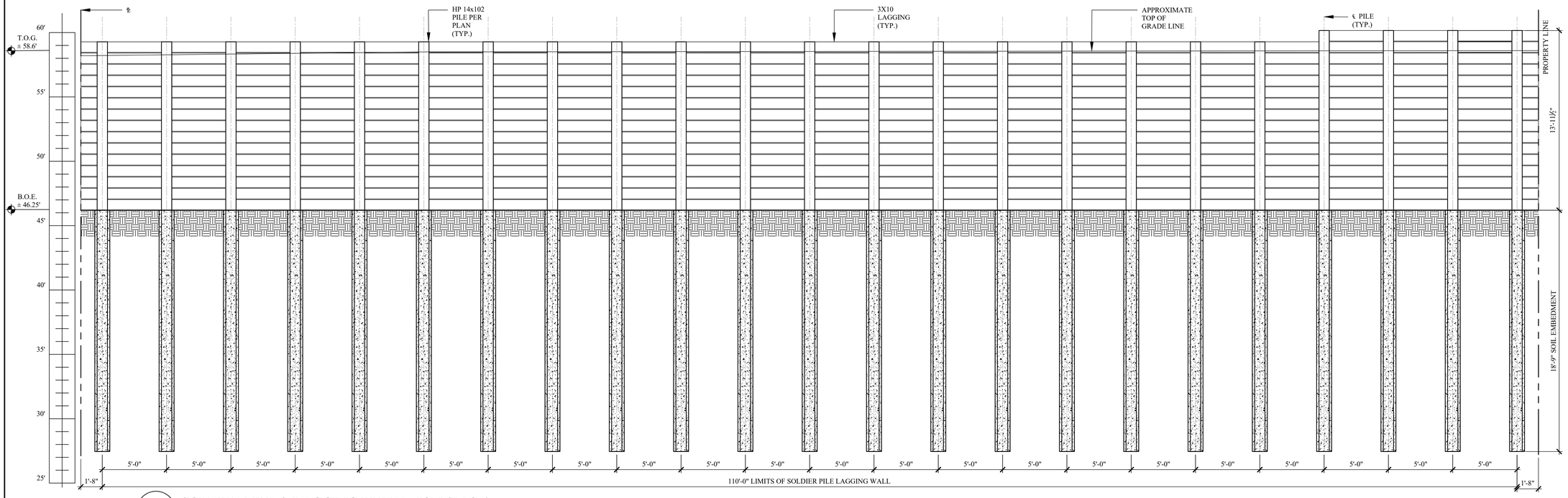
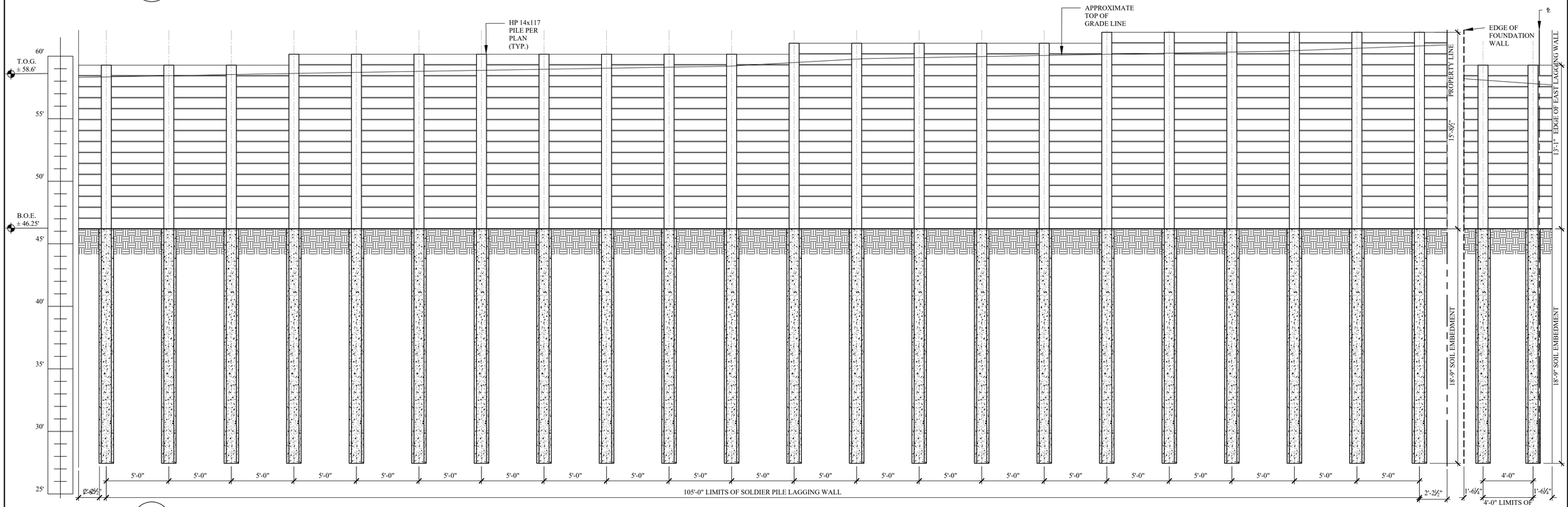


A SOLDIER PILE & LAGGING ELEVATION (EAST)

SCALE: 1/4"=1'-0"



B SOLDIER PILE & LAGGING ELEVATION (SOUTH)

SCALE: 1/4"=1'-0"

C SOLDIER PILE & LAGGING ELEVATION (NORTH)

SCALE: 1/4"=1'-0"

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SOLDIER PILE-LAGGING ELEVATIONS

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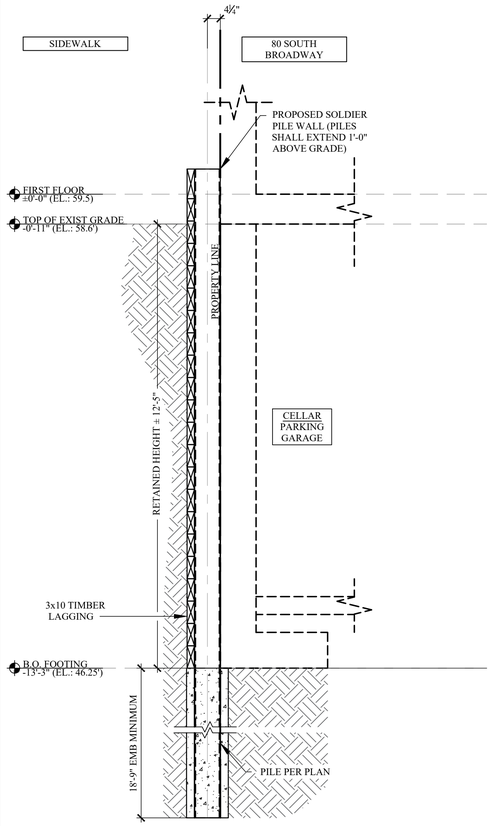

A SECTION A
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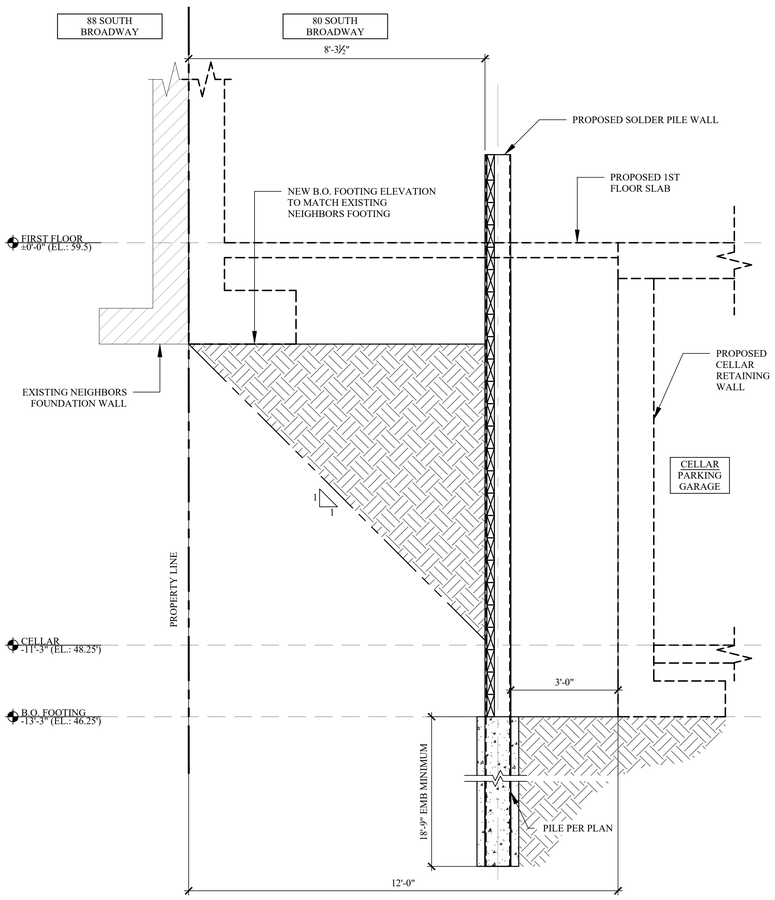

B SECTION B
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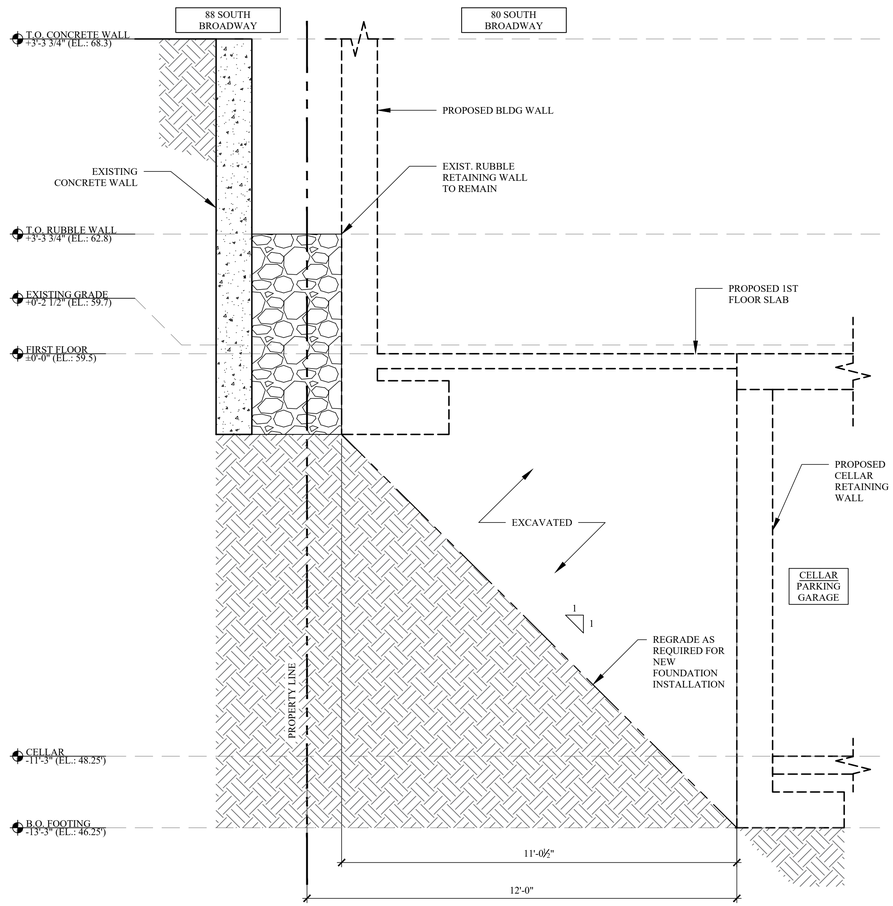
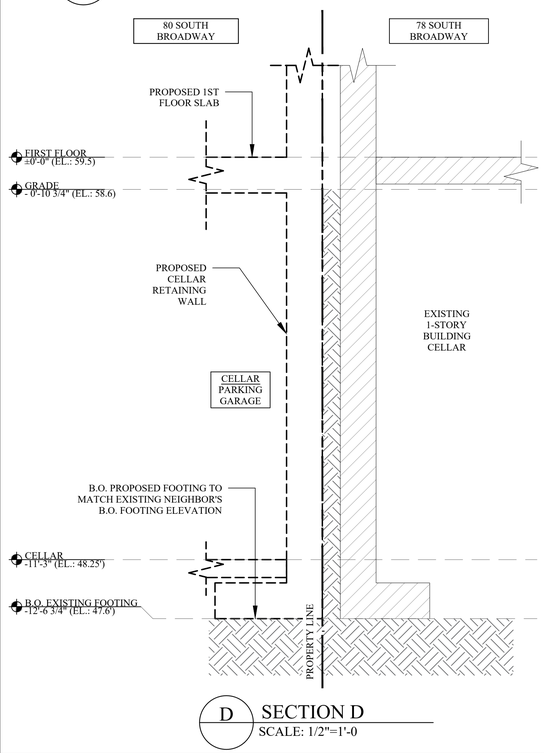


C SECTION C
SCALE: 1/2"=1'-0



D SECTION D
SCALE: 1/2"=1'-0

SHORING INSTALLATION PROCEDURE:

1. INSTALL SOLDIER PILES PER PLAN.
2. EXCAVATE 2'-0" OF SOIL + INSTALL WOOD LAGGING. IMMEDIATELY BACK PACK VOIDS BEHIND WOOD LAGGING AS REQUIRED.
3. CONTINUE PROCESS IN 2'-0" INCREMENTS UNTIL DESIRED ELEVATION IS REACHED.

DRPILLA
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5 Columbus Circle, 11th Fl
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REVISIONS:

NO.	DATE	DESCRIPTION

PROJECT:
80 S BROADWAY
NYACK, NY

SECTIONS

SEAL AND SIGNATURE:

DOMINICK R. PILLA, P.E., R.A.
NY P.E. 074213-1 NY R.A. 027028-1

ISSUE:	PLANNING BOARD
DATE:	01/27/2026
PROJECT NO.:	22-143
DRAWN/CHK BY:	CE/SM
SCALE:	AS NOTED
DWG NO.:	

SOE-300.00