

# MEMORANDUM

CITY OF WATERTOWN, NEW YORK PLANNING AND COMMUNITY DEVELOPMENT DEPARTMENT 245 WASHINGTON STREET, ROOM 305, WATERTOWN, NY 13601 PHONE: 315-785-7741 – FAX: 315-782-9014

TO:	Planning Board Members
FROM:	Michael A. Lumbis, Planning and Community Development Director
SUBJECT:	Site Plan Approval – 514, 528 and 540 State Street
DATE:	December 29, 2022
Request:	Site Plan Approval for the construction of a 2,738 sq. ft. Taco Bell restaurant and associated site improvements at 514, 528 and 540 State Street, Parcel Numbers 12-03-126.000, 12-03-127.000 and 12-03-128.000
Applicant:	Stephanie Albright, P.E. of APD Engineering on behalf of Hospitality Syracuse
Proposed Use:	Restaurant (Sales-Oriented Retail)
<b>Property Owners:</b>	Jon Lennox and North Drive In Theater Corporation

Submitted:	
Property Survey: Yes	Preliminary Architectural Drawings: Yes
Site Plan: Yes	Preliminary Site Engineering Plans: Yes
Vehicle and Pedestrian Circulation Plan: Yes	Construction Time Schedule: Yes
Landscaping and Grading Plan: Yes	Description of Uses, Hours & Traffic Volume: Yes
SEQRA: Unlisted Jefferson County 239-m Review: No	
Zoning Information:	
District: Commercial	Maximum Lot Coverage: N/A
Setback Requirements: F: 20', S: 5', R: 25'	Buffer Zones Required: Yes

**Project Overview:** Hospitality Syracuse is proposing to construct a 2,738 square foot Taco Bell Restaurant and associated site improvements at 514, 528 and 540 State Street, Parcel Numbers 12-03-126.00, 12-03-127.000 and 12-03-128.000. The three parcels are currently under contract and will be

conveyed to Fairlane Drive, LLC once the site plan has been approved. It is proposed that the parcels will then be combined into one prior to construction.

**Existing Conditions:** The existing property consists of two vacant parking lots and a vacant Trailways building and parking lot. The proposed project will demolish the existing building and site features to construct the Taco Bell and associated parking lot, utilities, lighting and landscaping.

**Vehicular and Pedestrian Circulation:** The site is located on the corner of State Street and Winthrop Streets. There are four existing curb cuts along State Street and one existing curb cut on Winthrop Street. The project will reduce the curb cuts on State Street to two, with one being an exit only from the drive through lane and the other being a full entrance/exit. The curb cut on Winthrop Street will be for two-way traffic.

The plans depict the location of the curb cuts to be closed and/or relocated on State Street and Winthrop Street and indicate that the curb is to be replaced with a curb reveal in accordance with City standards. What is not indicated on the plans is the proposed landscape treatment for the area where the driveway aprons are to be removed. Treatment could be either the installation of brick pavers to match the existing pavers that are currently located in the margin area or lawn area. It is the City's preference that the area where the driveway aprons are removed be converted to lawn areas. Additionally, the City recommends that the entire margin area, exclusive of the new driveways, be changed to lawn areas. This would reduce the impervious area of the site even further, eliminate uneven surfaces and trip hazards, match the treatment behind the sidewalk and throughout the site and overall be more aesthetically pleasing.

Winthrop Street is a one-way street, with traffic entering from State Street. As the Planning Board will recall, the applicant was originally requesting that Winthrop Street be changed to two-way traffic, but the request has since been withdrawn.

Section 310-47 of the Zoning Ordinance of the City of Watertown requires five spaces for every 1,000 sq. ft. of commercial floor area, exclusive of utility and storage areas. The required number of spaces for a building of this size is 14, and 25 spaces are being proposed.

The site design will allow for 17 cars to be stacked in the drive through lanes. Cars will enter the drive through at a single-entry point and as they make their way south, the lane will widen and split to allow for two rows of stacking. An additional four vehicles could stack toward State Street and an additional five vehicles could stack toward Winthrop Street if necessary. There will be two order points, one pre-pay window and one pick-up window. The exit lane is proposed to be 24 feet wide to allow both right and left turn stacking to exit onto State Street.

A truck turning movement plan has been included in the submission packet. The plan shows that both delivery trucks and fire trucks will access the site from the main entrance on State Street. The circulation of the site is not ideal for these types of large vehicles, so the trucks will need to do a three-point turn in order to exit the site. This could be problematic if customers are trying to access the site during the same time as a large delivery truck is on site. The applicant should discuss whether deliveries are completed outside of normal operating hours or during business hours.

The Engineering Department reviewed the traffic analysis provided by the consultant and agrees that the development will not have a significant impact on State Street or Winthrop Street with regards to traffic patterns or disruptions to traffic flow on State Street. According to the Traffic Impact Assessment completed by GTS Consulting, the additional traffic generated by the proposed Taco Bell development

will have no notable or significant impact on traffic operations on State Street or at the adjacent Winthrop Street intersection. There are adequate gaps in traffic to accommodate turning movements into and out of the development, adequate sight lines in each direction, no significant queuing concerns from the adjacent signals, and no capacity concerns.

**Zoning:** 528 and 540 State Street were previously split zoned, with Commercial zoning along the front and Residence C in the rear. The applicant applied to the Planning Board at the November 1, 2022 meeting for a zone change from Residence C to Commercial. The Planning Board recommended approval of the zone change, and the City Council approved it at their December 5, 2022 meeting. All three parcels are now zoned Commercial. The proposed use is an allowed use-by-right in a Commercial District.

**Setbacks:** The Commercial District requires that the building is setback a minimum of 20 feet from the front parcel line. The submitted site plan shows the proposed building setback to be 13.4 feet. The applicant applied to the Zoning Board of Appeals for an Area Variance to request relief from the required 20-foot front building setback. The Zoning Board of Appeals approved the request at the December 21, 2022 to allow for the 13.4-foot front building setback. All other required setbacks have been met.

**Landscaping and Buffers:** According to Section 310-59, Paragraph C of the City Zoning Ordinance, each use in a Commercial District shall have a strip of land at least 15 feet in any required front yard and at least five feet in width in any required rear and side yards, which shall be maintained as a landscaped area.

The submitted site plan shows the required five-foot landscape buffers on the sides and rear of the site. However, a portion of the front landscape buffer is only 8.5 due to the reduced building setback. The applicant applied to the Zoning Board of Appeals for an Area Variance to request relief from the required 15-foot front landscape buffer and reduce it to 8.5 feet. The variance application was approved by the Zoning Board of Appeals on December 21, 2022.

According to the Landscaping and Buffer Zone Guidelines adopted by the City of Watertown Planning Board a landscape plan is required as part of every site plan review application. Landscaping is required to minimize the negative impacts from development by creating visual and noise buffers between adjoining property uses and promoting harmonious streetscapes.

As noted above, Section 310-59, Paragraph C of the City Zoning Ordinance requires a 15' strip of land in the front yard which shall be maintained as a landscaped area. The Landscaping and Buffer Zone Guidelines discuss the appropriate treatment for these types of areas. The guidelines recommend in the landscaped strip one large deciduous tree planted every 40' or one small to medium deciduous tree planted every twenty feet with planting beds in between the trees. The applicant has provided a strip of land that is 15.5' wide but no trees are shown along the State Street frontage and only one tree is shown along Winthrop Street. The applicant shall revise the landscaping plan to include one large deciduous tree planted every 40' or one small to medium deciduous tree planted every 40' or one small to medium deciduous tree planted every 40' or one small to medium the landscaping plan to include one large deciduous tree planted every 40' or one small to medium deciduous tree planted every twenty feet with planting beds in between the trees in the areas along the street rights-of-way.

The guidelines also recommend interior parking lot trees and landscaping located in landscaped islands throughout the site at a minimum average density of 1 shade tree for each 15 parking spaces or fraction thereof. With the applicant proposing 25 parking spaces, two interior parking lot trees would be required. The applicant has proposed three interior parking lot trees, one at each end of the southern parking area and one located near the dumpster enclosure, thereby meeting this recommendation.

The guidelines also recommend exterior parking lot landscaping be provided around the perimeter of any parking/paved areas. The landscaped strip should be a minimum of 8' wide and within the strip one large deciduous tree planted every 40' or one small to medium deciduous tree planted every 20' or one large coniferous tree planted every 20' should be provided. The applicant is proposing a 5.5' landscaped strip along the eastern property line (along the drive-thru) with three small to medium maturing new trees and one large maturing tree, which does not quite meet the recommended spacing. The applicant shall revise the landscaping plan to include one large deciduous tree planted every 40' or one small to medium deciduous tree planted every 20' or one large coniferous tree planted every 20' along the eastern property line.

Additionally, Section 310-59, Paragraph A of the City Zoning Ordinance requires a strip of land a minimum of five feet in width up to a maximum of 15' in width be maintained as a landscaped area where the front, side and rear yard in a nonresidential district abuts any land in a residential district. The entire south side of the Taco Bell site abuts a residential district meaning that the buffer zone requirement applies to this area. The applicant has provided the minimum five-foot width along the property line in the area of the drive thru lanes and approximately 12' along the proposed parking spaces that abut the southern property line.

For these types of areas between Non-Residential and Residential Zoning Districts, the Landscaping and Buffer Zone Guidelines state that the landscaped strip shall be a minimum of fifteen (15') wide. Within the landscaped strip, one (1) large deciduous tree (2" caliper minimum) shall be provided every thirty five (35) linear feet, along with planting beds in between the trees containing assorted shrubs or one (1) small to medium deciduous tree (1.5" caliper minimum) shall be provided every twenty (20) linear feet, along with planting beds in between the trees containing assorted shrubs or one (1) large coniferous tree (6' minimum), stagger planted shall be provided every fifteen (15) linear feet. In addition to the required trees and shrubs, a six (6) foot high opaque fence (stockade or equal) should be provided. All fencing shall be in conformance with Section 310-26.1, Fences, of the Zoning Ordinance.

While the width of the buffer along the southern side of the site understandably varies due to various site layout needs, the composition of the buffer is lacking. No trees or shrubs are shown along the parking spaces. They must, however, be provided. Along the drive thru lanes, no shrubs are proposed, and only a few trees have been provided but not at the recommended spacing or correct tree type. The applicant shall revise the landscaping plan to include one large deciduous tree planted every 40' or one small to medium deciduous tree planted every 20' or one large coniferous tree planted every 20' along the southern site boundary to adequately buffer the residentially zoned areas.

The applicant has proposed a six-foot board on board fence along a portion of the southern and eastern side of the site to screen the proposed restaurant from the church and neighboring parcels located to the east.

In order to plant the required trees in the landscaped areas, it may require relocation/shifting of the water and storm sewer lines. The water line can be easily shifted/rotated to the north to provide space for trees along the parking spaces and the catch basins can be shifted slightly to push the alignment of the storm sewer main away from the center of the planting areas.

Finally, when developing the revised landscaping plan, the applicant should keep in mind species diversity. No one tree species may take up more than fifteen percent of the total amount of plantings. If desired by the applicant, Planning staff can provide a list of tree species that will be drought resistant and low maintenance in order to increase survivability in these areas.

**Comprehensive Plan:** The City's adopted Comprehensive Plan recommends the future land use character area of this area as Urban Mixed Use/Downtown Transition. The plan describes the Urban Mixed-Use land use area as follows:

"The Urban Mixed-Use areas are historic areas generally located between the Central Business District (CBD) and residential neighborhoods where land use transitions from intense urban business to lesser intense residential and compatible non-residential uses. These transitional areas begin to have obvious changes in building types, architectural styles, lot sizes, and pedestrian activity. Buildings are generally lower in height and parking may be onsite, preferably behind or at the side of the building to avoid a suburban look. Buildings are designed to be visually appealing with shorter setbacks to address the sidewalk and help reinforce a positive pedestrian experience."

Regarding consistency with the planned future land use character area, a restaurant is one of the uses envisioned for this district, as it fits well between the downtown and residential districts. While the applicant was required to obtain an area variance to reduce the front yard setback, shorter setbacks are mentioned in the description of this character area as a means of creating a pedestrian-oriented quality for those walking between downtown and the residential areas. This proposal is in harmony with the Comprehensive Plan.

**Hydrology:** The Engineering Department has reviewed the engineering report and agrees that the postdeveloped flows from the construction of the project will not increase the stormwater volume or redirect existing flows to adjacent properties. The disturbed area is less than one acre, therefore a SWPPP is not required for the development.

**Lighting:** The photometric plan submitted shows a significant amount of light spillage onto the church property to the rear. This light spillage is the result of a flood light on an existing utility pole at the rear of the site, directing light to the church parking lot. The applicant is proposing to remove this pole as part of the project. The applicant will work with the church to replace the light in order to provide lighting for their parking lot.

The applicant will be removing all other existing site lighting and replacing with new LED fixtures. There is an existing streetlight in the ROW of State Street that will need to be relocated as it conflicts with the proposed site access. The streetlights are owned by National Grid. The applicant must coordinate with and obtain permission from National Grid for the proposed street light relocation.

**SEQR:** The City Council has already issued a Negative Declaration pursuant to SEQRA. The Council adopted a resolution on December 5, 2022, finding that changing the approved zoning classification of a portion of 528 and 540 State Street from Residence C to Commercial and the construction of the site plan will not have a significant impact on the environment. The Council considered the "whole action" of the proposed development, including the granting of Site Plan Approval, when it reached this determination.

**Other:** As required in the site plan application, the applicant has shown a proposed concrete snow storage area on the site, located at the southern most section of the property, below the drive-thru order points. The size of the concrete snow storage area is somewhat excessive, measuring approximately 30' x 60'. The applicant should reduce the size of the snow storage area by 10' on the west, south and east sides to provide a larger landscaped buffer more suitable for the long-term survival of the plant material placed there and to reduce the amount of impervious area on the site.

**Permits:** The applicant must obtain the following permits and other documentation, minimally, prior to demolition and construction: Demolition Permit, Building Permit, General City Permit (for work within the right-of-way), Sanitary Sewer Connection Permit, Water Supply Permit, and a Zoning Compliance Certificate.

**Summary:** The following should be discussed by the Planning Board and included as contingencies in the motion to recommend approval of the site plan:

- 1. The applicant shall combine the lots prior to the issuance of a building permit by way of a new metes and bounds description that is filed with the County Clerk.
- 2. The applicant shall install grass areas or brick pavers in the margin where the driveway aprons are proposed for removal.
- 3. The applicant should discuss whether deliveries are completed outside of normal operating hours or during business hours.
- 4. The applicant shall revise the landscaping plan to include one large deciduous tree planted every 40' or one small to medium deciduous tree planted every twenty feet with planting beds in between the trees in the areas along the street rights-of-way.
- 5. The applicant shall revise the landscaping plan to include one large deciduous tree planted every 40' or one small to medium deciduous tree planted every 20' or one large coniferous tree planted every 20' along the eastern property line and along the southern site boundary to adequately buffer the residentially zoned areas
- 6. The applicant must coordinate with and obtain permission from National Grid for the proposed street light relocation.
- 7. The applicant should reduce the size of the snow storage area by 10' on the west, south and east sides to provide a larger landscaped buffer more suitable for the long-term survival of the plant material placed there and to reduce the amount of impervious area on the site.
- 8. The applicant must obtain the following permits, minimally, prior to demolition and construction: Demolition Permit, Building Permit, General City Permit (for work within the right-of-way), Sanitary Sewer Connection Permit, Water Supply Permit, and a Zoning Compliance Certificate.
- cc: City Council Members Michael Delaney, City Engineer Stephanie Albright, APD Engineering Michael McCracken, Hospitality Syracuse



APD Project No. 22-0408

December 19, 2022

Planning and Community Development Dept. City of Watertown 245 Washington Street, Room 305 Watertown, NY 13601

RE: Taco Bell Watertown – Planning Board Submittal

Mr. Lumbis,

On behalf of our client Hospitality Syracuse Inc., we are pleased to present the enclosed information for your review for the proposed construction of a Taco Bell along State St and Winthrop St in the City of Watertown. Our scope of work includes demolition of an existing building and construction of a 2,700± SF Taco Bell building along with associated site improvements. The three parcels making up the proposed lot will be consolidated into one prior to construction.

Please find enclosed the following per your requirements:

- 1) Fifteen (15) copies of the Letter of Intent (This Letter)
- 2) Fifteen (15) copies of the completed and signed application form
- 3) Fifteen (15) copies of the Letters of Authorization from the property owners
- 4) Fifteen (15) copies of SEQR short form EAF
- 5) Fifteen (15) copies of the Engineer's Report
- 6) Two (2) copies of the Post-Construction Stormwater Management Plan
- 7) Two (2) copies of the Traffic Impact Assessment
- 8) Fifteen (15) copies of the following plans/sets (3 Full Sized, 12 11"x17", unless noted otherwise)
  - a) Site Development Plans
  - b) Truck Turning Movements, TT1
  - c) LSI Photometric Plan
  - d) Building Elevation (15 sets of 11x17)
  - e) Building Floor Plan (15 sets of 11x17)
  - f) Dumpster Enclosure detail (15 sets of 11x17)
- 9) One flash drive containing submittal material
- 10) One (1) \$150 check

Should you have any questions, comments, or need additional information, please feel free to contact me at 585-742-0204 or salbright@apd.com.

Sincerely,

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Stephanie Albright, P.E. Senior Civil Engineer

cc: M. McCracken (Hospitality Syracuse Inc)

October 10, 2022

City of Watertown 245 Washington St Watertown NY 13601

RE: Letter of Authorization – 514 & 528 State St

To Whom it may concern,

This letter serves as formal authorization for APD Engineering and Architecture, Fairlane Drive LLC, and/ or Hospitality Syracuse Inc and its agents or employees to submit on my behalf and represent me on matters relating to applications to the City of Watertown for the building, planning, and zoning requests associated with the proposed Taco Bell Development.

Tax Map #12-03-126 and 12-03-127

Sincerely, may Jon Lennox Propertyowner

October 11, 2022

City of Watertown 245 Washington St Watertown NY 13601

RE: Letter of Authorization – 540 State St

To Whom it may concern,

This letter serves as formal authorization for APD Engineering and Architecture, Fairlane Drive LLC, and/or Hospitality Syracuse Inc and its agents or employees to submit on my behalf and represent North Drive In Theater Corp on matters relating to applications to the City of Watertown for the building, planning, and zoning requests associated with the proposed Taco Bell Development.

Tax Map #12-03-128

Sincerely,

North Drive In Theater Corp Property Owner Alexander Papayanakos



#### City of Watertown SITE PLAN APPROVAL APPLICATION FORM

City of Watertown, Planning and Community Development Dept. 245 Washington Street, Room 305, Watertown, NY 13601 Phone: 315-785-7741 Email: planning@watertown-ny.gov Received:

**Please Note:** The Site Plan Approval Application form is for projects where the building or parking area coverage of the lot will increase by more than 2,500 square feet.

Please provide responses for all sections and submit all required materials as noted on Page 2. Failure to submit all required information by the submittal deadline may result in Staff **not** placing your request on the agenda for the upcoming Planning Board meeting.

#### **PROPERTY INFORMATION:**

PROPOSED PROJECT NAME: <u>Taco Bell - Watertown</u>
TAX PARCEL NUMBER: <u>12-03-126,12-03-127,12-03-128</u>
PROPERTY ADDRESS: <u>State Street; Watertown, NY 13601</u>

ZONING DISTRICT: Commercial

#### **APPLICANT INFORMATION:**

NAME: Hospitality Syracuse Inc. c/o Mike McCracken

ADDRESS: 290 Elwood Davis Road; Liverpool, NY 13088

PHONE NUMBER: (315) 451-1957

E-MAIL ADDRESS: <u>mikem@hrgweb.com</u>

#### **PROPERTY OWNER INFORMATION (if different from applicant):**

NAME: Jon Lennox (514 and 528 State St), North Drive-In Theater Corp (540 State St)

ADDRESS: Jon Lennox: 555 State St, Watertown NY 13601

North Drive In: PO Box 203 Watertown NY 13601

PHONE NUMBER: \_\_\_\_\_\_

E-MAIL ADDRESS:

#### ENGINEER/ARCHITECT/LANDSCAPE ARCHITECT INFORMATION:

NAME: APD Engineering & Architecture c/o Stephanie Albright

ADDRESS: 615 Fishers Run; Victor, NY 14564

PHONE NUMBER: \_(585) 742-0204

E-MAIL ADDRESS: salbright@apd.com

#### **REQUIRED MATERIALS:**

\*\* The following drawings with the listed information <u>ARE REQUIRED, NOT OPTIONAL</u>. If the required information is not included and/or addressed, Planning Staff will not process the Site Plan Application.

All of the following drawings **must** be adequately dimensioned, including radii and must use darker line work and text for proposed features than for existing features.

COVER LETTER: Must clearly and fully explain the proposed project in sufficient detail.

BOUNDARY and TOPOGRAPHIC SURVEY: Depict existing features as of the date of the Site Plan Application. A Professional Land Surveyor licensed and currently registered to practice in the State of New York must perform the survey and create the map. At least one copy must contain the surveyor's original PLS wet stamp and an original signature. The rest may be copies thereof. The survey drawing **must** depict and label all of the following:

- All existing features and utilities on and within 50 feet of the subject property
- All existing property lines (bearings and distances), margins, acreage, zoning, easements, right-of-ways, existing land use, reputed owner, adjacent reputed owners and tax parcel numbers
- One-foot contours are with appropriate spot elevations
- North arrow and graphic scale
- All elevations are North American Vertical Datum of 1988 (NAVD88).

#### DEMOLITION PLAN (if applicable)

- Depict and label **all** existing features on and within 50 feet of the subject property and (using darker text) all items proposed for demolition.
- SITE PLAN: The drawing must clearly label all proposed features as "proposed" and use darker line work and text for all proposed features than for existing features. It must also include a reference to the coordinate system used (NYS NAD83-CF preferred). In addition, the drawing **must** depict and label all of the following:
  - All proposed **above** ground features
  - All proposed easements and right-of-ways
  - Land use, zoning, and tax parcel number
  - Proposed parking and loading spaces, including all required ADA accessible spaces
  - Proposed snow storage areas
  - Refuse Enclosure Area (Dumpster), if applicable. Please note: Section 161-19.1 of the Zoning Ordinance states, "No
    refuse vehicle or refuse container shall be parked or placed within 15 feet of a party line without the written consent of the
    adjoining owner, if the owner occupies any part of the adjoining property."
  - North arrow and graphic scale

**GRADING PLAN:** This drawing must depict and label **all** of the following:

- All proposed **below** ground features, including elevations and inverts
- All proposed **above** ground features, including easements and right-of-ways
- One-foot existing contours (shown dashed and labeled with appropriate spot elevations)
- One-foot proposed contours (shown and labeled with appropriate spot elevations)
- Sediment and Erosion control, unless separate drawings are included as part of a Stormwater Pollution Prevention Plan (SWPPP).
- All elevations are North American Vertical Datum of 1988 (NAVD88).

UTILITY PLAN: This drawing must include a note stating, "All water main and service work must be coordinated with the City of Watertown Water Department. The Water Department requirements supersede all other plans and specifications provided." It must also depict and label **all** of the following:

- All proposed above and below ground features
- All existing above and belowground utilities, including water, sanitary water, stormwater, electric, gas, telephone, cable, fiber optic, etc.
- All existing and proposed easements and right-of-ways.

✓ LANDSCAPING PLAN: This drawing must depict and label all of the following:

- All proposed above ground features
- All proposed trees, shrubs, other plantings and other proposed landscaping additions, keyed to a plant schedule that
  includes the scientific name, common name, size, quantity, etc. Please note: For additional landscaping requirements
  where nonresidential districts and land uses abut land in any residential district, please refer to Section 310-59,
  Landscaping of the City's Zoning Ordinance.
- The Site Plan complies with and meets acceptable guidelines set forth in Appendix A Landscaping and Buffer Zone Guidelines (August 7, 2007).

#### VEHICULAR AND PEDESTRIAN CIRCULATION PLAN

- Depict all vehicular **and** pedestrian traffic circulation, including a delivery or refuse vehicle and a City fire truck entering and exiting the property.
- Sidewalks within the City Right-of-Way **must** meet Public-Right-of-Way (PROWAG) standards.
- The Site Plan is consistent with and, wherever possible, incorporates principles set forth in Appendix B City of Watertown Complete Streets Policy (January 17, 2017).

PHOTOMETRIC PLAN (if applicable): This drawing must depict and label all of the following:

- All proposed above ground features
- Photometric spot elevations or labeled photometric contours of the property. **Please note:** Light spillage across **all** property lines shall not exceed 0.5 foot-candles.

#### CONSTRUCTION DETAILS and NOTES:

- Provide all details and notes necessary to complete the project including, but not limited to, landscaping, curbing, catch basins, manholes, water line, pavement, sidewalks, trench, lighting, trash enclosure, etc.
- Provide maintenance and protection and traffic plans and notes for all required work within City streets including driveways, water laterals, sanitary laterals, storm connections, etc.
- The drawings must include the following note: "All work to be performed within the City of Watertown margin will require sign-off from a Professional Engineer, licensed and currently registered to practice in the State of New York, that the work was built according to the approved site plan and applicable City of Watertown standards. Compaction testing will be required for all work to be performed within the City of Watertown margin and must be submitted to the City of Watertown Codes Department."

PRELIMINARY ARCHITECTUAL PLANS (if applicable): These plans must include all of the following for proposed buildings: Floor plan drawings, including finished floor elevations, exterior elevations including exterior materials and colors, as well as roof outlines depicting shape, slope and direction.

#### ✓ ENGINEERING REPORT

\*\* The engineering report at a minimum must include the following:

- Project location and description
- Existing and proposed sanitary sewer flows and summary
- Water flows and pressure
- Storm Water Pre and Post Construction calculations and summary
- Traffic impacts
- Lighting summary
- Landscaping summary

COMPLETED SEQR ENVIRONMENTAL ASSESSMENT FORM: (Contact us if you need help choosing between the Short EAF and the Full EAF). The Complete EAF is available online at: <u>http://www.dec.ny.gov/permits/6191.html</u>

#### GENERAL INFORMATION

- All items must include a valid stamp and an <u>original</u> signature by a Professional Engineer, Architect, Landscape Architect, or Surveyor licensed and currently registered to practice in the State of New York.
- If required, submit a copy of the Stormwater Pollution Prevention Plan (SWPPP) to the City of Watertown Engineering Department for review to obtain an MS4 SWPPP Acceptance Form.

#### Post Construction SWPPP Requirements to Complete:

In accordance with City Code Section 260, provide the following:

- Submit a detailed as-built topographic and boundary survey of the site with all stormwater practices.
- Perform and submit results of insitu infiltration testing, updated drainage area maps and hydraulic calculations in a comprehensive Engineering Report based on As-Built Conditions.
- Submit a detailed post construction Maintenance Plan for all Stormwater Management Practices (SMP's) and provide a Maintenance Agreement with irrevocable letter of credit for approval. Maintenance Agreement shall be filed at the County Clerk's Office as a deed restriction on the property.
- **\*\*** If required, a copy of all submittals sent to the New York State Department of Environmental Conservation (NYSDEC) for the sanitary sewer extension permit will also be sent to the City of Watertown Engineering Department.
- \*\* If required, a copy of all submittals sent to the New York State Department of Health (NYSDOH) will also be sent to the City of Watertown Engineering Department.
- \*\* When NYSDEC or NYSDOH permitting is required, the property owner/applicant shall retain a licensed Professional Engineer to perform inspections of the proposed utility work and to certify the completed works were constructed in substantial conformance with the approved plans and specifications.\*\*
- Signage is not approved as part of this submission. It requires a Sign Permit from the City Code Enforcement Bureau. See Section 310-52.2 of the Zoning Ordinance.
- For non-residential uses, the applicant must include the proposed Hours of Operation.

#### **OPTIONAL MATERIALS:**

**PROVIDE AN ELECTRONIC (.DWG) COPY OF THE SITE PLAN WITH AS-BUILT REVISIONS.** This will assist the City in keeping our GIS mapping up-to-date.

#### SUBMITTAL INSTRUCTIONS:

Submit 15 complete collated sets of all required materials, addressed to:

Michael A. Lumbis, Planning and Community Development Director City of Watertown 245 Washington Street, Room 305 Watertown, NY 13601

If the application requires Jefferson County Planning Board review, then the applicant must submit 16 "sets." Planning Staff will inform the applicant if this is necessary.

Submissions must be collated and properly folded.

- If the applicant is not the property owner, the submission must include a signature authorization form or letter signed by the owner authorizing the applicant to apply on behalf of the owner.
- For any item(s) not checked in the Site Plan Approval Checklist, attach an explanation and comments.
- Provide an electronic copy of the entire submission in the form of a single, combined PDF file of the entire application, including cover letter, plans, reports, and all submitted material.

Submit the required Application Fee

\$150 for Site Plan Minor

\$250 for Site Plan Major (any proposal to disturb more than 1 acre represents a Site Plan Major)

#### SIGNATURE

I certify that the information provided above is true to the best of my knowledge.

Applicant's name (please	e print)		
Applicant's Signature	My Hand	Date:	
	DF2A2E8CBC134F1		

**Meeting Information:** The Planning Board normally meets at 3:00 p.m. on the first Tuesday of every month in Council Chambers at City Hall, 245 Washington Street. The application deadline is 14 days prior to the scheduled meeting date. Planning Board action does not represent final approval, as the Planning Board only votes to make a recommendation to City Council, which holds the sole authority to grant Site Plan Approval.

Occasionally, due to holidays or other reasons, meetings may occur on other dates and/or times. The City will announce any changes to meeting dates in advance on its website at <u>www.watertown-ny.gov</u>. Planning Staff *strongly* recommends scheduling a pre-application meeting prior to submitting a Site Plan Application. The entire site plan application process typically takes four-to-six weeks, depending on whether the application requires Jefferson County Planning Board review.

### Short Environmental Assessment Form Part 1 - Project Information

#### **Instructions for Completing**

**Part 1 – Project Information. The applicant or project sponsor is responsible for the completion of Part 1.** Responses become part of the application for approval or funding, are subject to public review, and may be subject to further verification. Complete Part 1 based on information currently available. If additional research or investigation would be needed to fully respond to any item, please answer as thoroughly as possible based on current information.

Complete all items in Part 1. You may also provide any additional information which you believe will be needed by or useful to the lead agency; attach additional pages as necessary to supplement any item.

#### Part 1 – Project and Sponsor Information

Name of Action or Project:

Taco Bell - State St Watertown

Project Location (describe, and attach a location map):

514, 528, 540 State St Watertown NY 13601

Brief Description of Proposed Action:

This project proposed to demolish the existing Trailways building and associated appurtenance to construct a +/- 2,600 SF Taco Bell building and associated parking lot, utilities, landscaping, etc. The project includes the rezoning of four parcels from Residence C to Commercial.

Name of Applicant or Sponsor:Telephone: 315-451-1957	Telephone: 315-451-1957			
Hospitality Syracuse, Inc. E-Mail: mikem@hrgweb.com				
Address:				
290 Elwood Davis Road, Suite 320				
City/PO: State: Zip Code	de:			
Liverpool NY 13088				
1. Does the proposed action only involve the legislative adoption of a plan, local law, ordinance, administrative rule, or regulation?	NO	YES		
If Yes, attach a narrative description of the intent of the proposed action and the environmental resources that may be affected in the municipality and proceed to Part 2. If no, continue to question 2.				
2. Does the proposed action require a permit, approval or funding from any other government Agency?				
If Yes, list agency(s) name and permit or approval: City Council - Rezoning and Site Plan Approval, ZBA - Variances (potential), City - Lot consolidation, DOH - Backflow approval				
3. a. Total acreage of the site of the proposed action? +/- 0.9 acres				
b. Total acreage to be physically disturbed?+/- 0.9 acres				
c. Total acreage (project site and any contiguous properties) owned				
or controlled by the applicant or project sponsor? <u>+/- 0.9</u> acres				
4. Check all land uses that occur on, are adjoining or near the proposed action:				
5. 🗹 Urban 🗌 Rural (non-agriculture) 🗌 Industrial 🗹 Commercial 🗹 Residential (suburban)				
Forest Agriculture Aquatic Other(Specify):				
Parkland				

5. Is the proposed action,	NO	YES	N/A
a. A permitted use under the zoning regulations?		~	
b. Consistent with the adopted comprehensive plan?		~	
Is the proposed action consistent with the prodominant character of the existing built or network landscene?		NO	YES
5. Is the proposed action consistent with the predominant character of the existing built or natural landscape?			~
7. Is the site of the proposed action located in, or does it adjoin, a state listed Critical Environmental Area?		NO	YES
f Yes, identify:		✓	
8. a. Will the proposed action result in a substantial increase in traffic above present levels?		NO	YES
b. Are public transportation services available at or near the site of the proposed action?			
c. Are any pedestrian accommodations or bicycle routes available on or near the site of the proposed action?			
Does the proposed action meet or exceed the state energy code requirements?		NO	YES
f the proposed action will exceed requirements, describe design features and technologies:			~
0. Will the proposed action connect to an existing public/private water supply?		NO	YES
If No, describe method for providing potable water:			~
1. Will the proposed action connect to existing wastewater utilities?		NO	YES
If No, describe method for providing wastewater treatment:			~
2. a. Does the project site contain, or is it substantially contiguous to, a building, archaeological site, or district		NO	YES
which is listed on the National or State Register of Historic Places, or that has been determined by the Commissioner of the NYS Office of Parks, Recreation and Historic Preservation to be eligible for listing on the State Register of Historic Places?		✓	
b. Is the project site, or any portion of it, located in or adjacent to an area designated as sensitive for rchaeological sites on the NY State Historic Preservation Office (SHPO) archaeological site inventory?			~
3. a. Does any portion of the site of the proposed action, or lands adjoining the proposed action, contain wetlands or other waterbodies regulated by a federal, state or local agency?		NO	YES
b. Would the proposed action physically alter, or encroach into, any existing wetland or waterbody?			
f Yes, identify the wetland or waterbody and extent of alterations in square feet or acres:			

14. Identify the typical habitat types that occur on, or are likely to be found on the project site. Check all that apply:		
Shoreline Forest Agricultural/grasslands Early mid-successional		
Wetland 🗹 Urban 🗌 Suburban		
15. Does the site of the proposed action contain any species of animal, or associated habitats, listed by the State or	NO	YES
Federal government as threatened or endangered?		
Northern Long-eared Bat		~
16. Is the project site located in the 100-year flood plan?	NO	YES
	~	
17. Will the proposed action create storm water discharge, either from point or non-point sources?	NO	YES
If Yes,		~
a. Will storm water discharges flow to adjacent properties?		<ul> <li>✓</li> </ul>
b. Will storm water discharges be directed to established conveyance systems (runoff and storm drains)? If Yes, briefly describe:		~
Storm will flow to City storm sewer system within State St.		
18. Does the proposed action include construction or other activities that would result in the impoundment of water or other liquids (e.g., retention pond, waste lagoon, dam)?	NO	YES
If Yes, explain the purpose and size of the impoundment:		
	~	
49. Has the site of the proposed action or an adjoining property been the location of an active or closed solid waste	NO	YES
management facility? If Yes, describe:		
	~	
20.Has the site of the proposed action or an adjoining property been the subject of remediation (ongoing or	NO	YES
completed) for hazardous waste? If Yes, describe:		
		~
Former spills identified on the 540 State St parcel. Applicant is currently obtaining a Phase I and Phase II ESA to determine the extent of potential environmental impacts.		
I CERTIFY THAT THE INFORMATION PROVIDED ABOVE IS TRUE AND ACCURATE TO THE BE	ST OF	
MY KNOWLEDGE		
Mike McCracken     11/1/2022       Applicant/sponsor/name:		
DocuSigned by:	ont	
Signature:		

### EAF Mapper Summary Report

6-03-319.000 6-03-318.000 6-03-317.000 6-03-315.000 6-03-307.000 6-03-307.000 6-03-307.000 6-03-307.000 6-08-138.000 6-08-139.000 6-08-126.100 6-08-126.100 6-08-126.100 6-08-126.100 6-08-126.000	<b>Disclaimer:</b> The EAF Mapper is a screening tool intended to assist project sponsors and reviewing agencies in preparing an environmental assessment form (EAF). Not all questions asked in the EAF are answered by the EAF Mapper. Additional information on any EAF question can be obtained by consulting the EAF Workbooks. Although the EAF Mapper provides the most up-to-date digital data available to DEC, you may also need to contact local or other data sources in order to obtain data not provided by the Mapper. Digital data is not a substitute for agency determinations.
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Part 1 / Question 7 [Critical Environmental Area]	No
Part 1 / Question 12a [National or State Register of Historic Places or State Eligible Sites]	Νο
Part 1 / Question 12b [Archeological Sites]	Yes
Part 1 / Question 13a [Wetlands or Other Regulated Waterbodies]	No
Part 1 / Question 15 [Threatened or Endangered Animal]	Yes
Part 1 / Question 15 [Threatened or Endangered Animal - Name]	Northern Long-eared Bat
Part 1 / Question 16 [100 Year Flood Plain]	Digital mapping data are not available or are incomplete. Refer to EAF Workbook.
Part 1 / Question 20 [Remediation Site]	Yes



# TACO BELL - CITY OF WATERTOWN, NY

### ABBREVIATIONS:

	-BOTTOM OF CURB/EDGE OF PAVEMENT
	-BOTTOM OF WALL
	-CABLE TELEVISION
	-CATCH BASIN
	-CONSTRUCTION MANAGER
CPP	-CORRUGATED POLYETHYLENE PIPE
	(SMOOTHED LINE)
DIA	
DIP	-DUCTILE IRON PIPE
DWY	-DRIVEWAY
DYSL	-DOUBLE YELLOW SOLID LINE (4" WIDE EACH
	UNLESS OTHERWISE NOTED)
ELEV	-ELEVATION
	-EDGE OF PAVEMENT
EOW	-ENGINEER OF WORK
FC	-FLUSH CURB
	-FIBER OPTICS
FES	-FLARED END SECTION
F.F.	-FINISH FLOOR
FFE	-FINISH FLOOR ELEVATION
FG	-FINISH GRADE
FH	-FIRE HYDRANT
FS	-FINISH SERVICE
FT	-FEET
INV	-INVERT
IE	-INVERT ELEVATION
G	
G.C.	-GENERAL CONTRACTOR
	-GROUND ELEVATION
HDPE	-HIGH DENSITY POLYETHYLENE PIPE
HP	-HIGH POINT

- ΗP -HIGH POINT -LENGTH L

LF	-LINEAR FEET
LB	-POUNDS
MAX	-MAXIMUM
	-MINIMUM
No.	-NUMBER -NOT TO SCALE
N.T.S.	-NOT TO SCALE
O.C.	-ON CENTER
OH	-OVERHEAD ELECTRIC
OHE	-OVERHEAD ELECTRIC
	-PROTECT DURING CONSTRUCTION
P.C.	-PORTLAND CEMENT
P.S.I.	-POUNDS PER SQUARE INCH
PVC	-POLYVINYL CHLORIDE
PVT	-PRIVATE
RTE	-ROUTE
R&R	-REMOVE AND REPLACE
S	-SLOPE
SQ. FT.	-SQUARE FEET
SQ. YD	SQUARE YARD
SF	-SQUARE FEET
SYSL	-SINGLE YELLOW SOLID LINE (4" WIDE UNLESS
	OTHERWISE NOTED)
Т	-TELEPHONE
	-THRUST BLOCK
TC	-TOP OF CURB
TF	
TG	-TOP OF GRATE
TW	-TOP OF WALL
	-TYPICAL
W	
W/	-WITH
W/L	-WATERLINE



### DRAWING LIST:

CS	Cover Sheet
SV	Survey
C1	Demolition Plan
C2	Site Plan
C3	Grading & Drainage Plan
C4	Utility Plan
C5	Planting & Lighting Plan
C6	Details Sheet
C7	Details Sheet
C8	Details Sheet
C9	Specifications

NOTES:

1. REFER TO LSI PLAN FOR LIGHTING PHOTOMETRICS.



CLIENT:

615 FISHERS RUN VICTOR, NY 14564 (585) 742-0204

# SITE DEVELOPMENT PLANS

FOR

STATE STREET WATERTOWN, NY 13601



LOCATION SKETCH <u>N.T.S.</u>

HOSPITALITY SYRACUSE, INC. 290 ELWOOD DAVIS ROAD - SUITE 320 LIVERPOOL, NY 13088

(315) 451-1957 CONTACT: MIKE MCCRACKEN

APD ENGINEERING & ARCHITECTURE

CONTACT: STEPHANIE ALBRIGHT, P.E.

AS REQUIRED BY NEW YORK STATE LAW, CONTRACTOR SHALL CONTACT "DIG SAFELY NEW YORK'' (UFPO) @ 1-800-962-7962 FOR LOCATION STAKE-OUT OF ALL UTILITIES, AT LEAST 2 FULL WORKING DAYS PRIOR TO ANY EXCAVATION.

### AGENCY & MUNICIPALITY CONTACT

CITY OF WATERTOWN PLANNING AND ZONING

JEN VOSS SENIOR PLANNER 245 WASHINGTON STREET WATERTOWN, NY 13601 (315) 785-7724 JVOSS@WATERTOWN-NY.GOV

MIKE LUMBIS PLANNING & COMMUNITY DEVELOPMENT DIRECTOR 245 WASHINGTON STREET WATERTOWN, NY 13601 (315) 785-7741 MLUMBIS@WATERTOWN-NY.GOV

MIKE DELANEY CITY ENGINEER 245 WASHINGTON STREET WATERTOWN, NY 13601 (315) 785-7740 MDELANEY@WATERTOWN-NY.GOV

4	C	rs:	

### UTILITY CONTACTS:

ELECTRIC SERVICE: NATIONAL GRID ADDRESS: PHONE: CONTACT: TBD

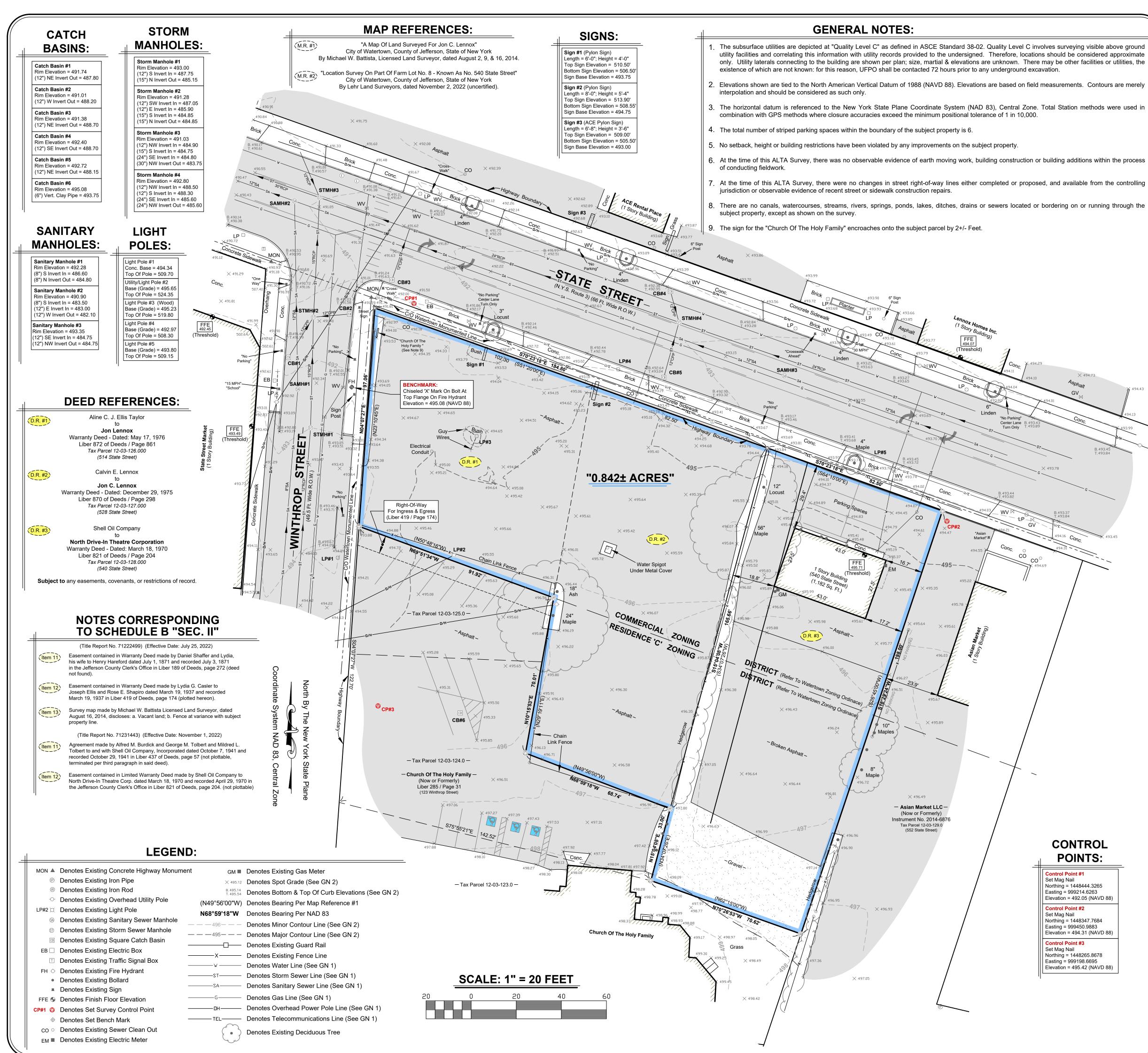
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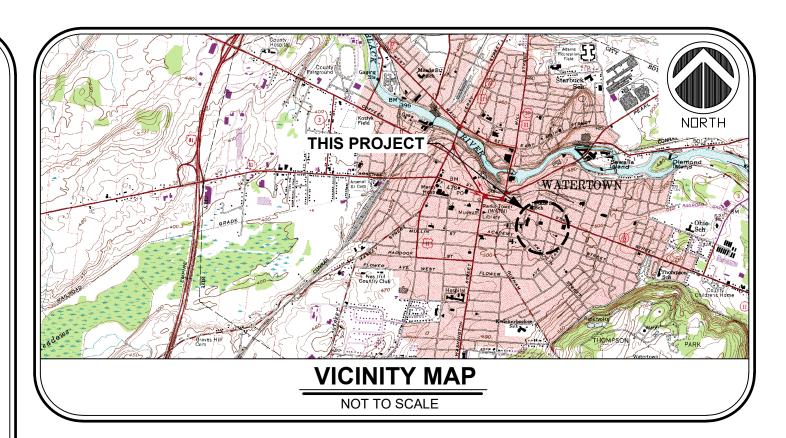
CONTACT: TBD

WATER SERVICE: DPW ADDRESS: 245 WASHINGTON STREET ROOM 202 WATERTOWN, NY 13601 PHONE: (315) 787-7757 CONTACT: VICKY MURPHY

SANITARY SEWER SERVICE: DPW ADDRESS: 245 WASHINGTON STREET WATERTOWN, NY 13601 PHONE: (315) 785-7842 CONTACT: TBD

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### **RECORD DESCRIPTIONS:**

THAT TRACT OR PARCEL OF LAND situate in the City of Watertown, County of Jefferson and State of New York, and bounded as follows, viz: Commencing at a stake in the S. margin of State Street (3 rods in an easterly direction from the N.E. corner of the "Park") the N.W. corner of Winthrop Street; running thence S. 12 1/2° W, along the east margin of Winthrop Street 2 chs. 32 97/100 lks. to the N.W. corner of Henry Hareford lot; thence S. 76 1/4° E. along said Hareford north line 1 ch. 31 1/2 lks. to the land of the late Juda Lord; thence N.18 1/2° E. along said Lord's west line 1 ch, 90 2/3 lks. to the margin of State Street; thence N. 62 1/4° W. along the margin of State Street 1 ch, 55 lks, to the place of beginning. EXCEPTING and RESERVING from the foregoing premises all the land lying southerly of a line defined by the southerly margin of a concrete walk running easterly and westerly along the southerly side of a brick building known as the Winthrop Apartments; said line being further described as passing through a point in the easterly margin of the concrete sidewalk on the easterly side of Winthrop Street 98.2 ' feet southerly of the intersection of the inside edges of the sidewalk on State and Winthrop Streets and also being 95.65 feet from the southerly edge of the sidewalk on the southerly side of State Street; said 95.65 feet being measured along the westerly line of a lot formerlyowned by Judah Lord and located easterly of the premises herein described.

#### Tax Parcel 12-03-127.000

Tax Parcel 12-03-126.000

ALSO, ALL THAT TRACT OR PARCEL OF LAND situate in the City of Watertown, County of Jefferson and State of New York, bounded and described as follows: COMMENCING at a point in the south margin of State Street in said City of Water town, 1 chain 55 links east from the intersection of said margin with the east margin of Winthrop Street; running THENCE S, 18½;0 W, 3 chains 3 links; THENCE S, 64½;0 E, 1 chain; THENCE N, 22\0 E. 3 chains 1 link to the south margin of said State Street; THENCE N, 62\0 W, along said margin I chain 25 links to the place of beginning, containing 34/100 of an acre of land be the same more or less. EXCEPTING AND RESERVING from the above described lands and premises so much thereof as was conveyed by said Judah Lord and wife to Henry Hareford by deed dated Sep ember 17, 1872, and recorded in the Jefferson County Clerk's Office on September 18, 1872, in Book 193 of Deeds at Page 311, and which was also conveyed by Mary J, DeLong and others to said Henry Hereford by Quit Claim Deed, Said premises being known as No, 528 State Street

### Tax Parcel 12-03-128.000

Beginning at a point in the south margin of the State Road two (2) chains twenty-eight (28) links westerly from the intersection of said south margin with the west margin of William Street and running thence south 26 3/4 degrees west three (3) chains; thence north 62% degrees west one (1) chain seven (7) links to the east margin of a lot once owned and occupied by Peter Snyder; thence north 22 degrees east three (3) chains one (1) link to the south margin of the State Road; thence south 64½ degrees east along said south margin one (1) chain twenty-five links to the place of beginning, containing 35/100 of an acre of land, being the same more or less.

**PHOTOS:** 



### ALTA/NSPS LAND TITLE SURVEY Lands Of — Jon C. Lennox — And Also Lands Of North Drive-In Theatre Corporation — 514, 528, & 540 State Street City of Watertown, County of Jefferson State of New York Surveyor's Certification TO: Fairlane Drive LLC TO: Stewart Title Insurance Company, its successors and/or assigns TO: Wells Fargo Bank NA., its successors and/or assigns

The survey shown and depicted hereon is an accurate survey of the real property legally described hereon and correctly and accurately indicates and locates all buildings, structures and other improvements situated on the subject property; and that the property described hereon is the same as the property described in Title Report No. 71222499, prepared by Stewart Title Insurance Company, having an effective date of July 25, 2022, and also Title Report No. 71231443, prepared by Stewart Title Insurance Company, having an effective date of November 1, 2022, and that all easements, rights-of-way, servitude's and covenants and restrictions referenced in said commitment have been plotted hereon or otherwise noted as to their effect on the subject property; that there are no encroachments or violations of zoning restriction lines on the subject property or upon adjacent land abutting said subject property unless shown and depicted hereon. This survey is subject to the findings of a current abstract of title.

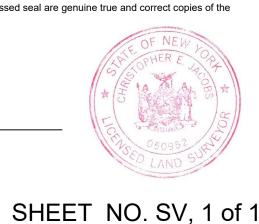
This is to certify that this map or plat and the survey on which it is based were made in accordance with the 2021 Minimum Standard Detail Requirements for ALTA/NSPS Land Title Surveys, jointly established and adopted by ALTA and NSPS, and includes Items 1, 2, 4, 5, 7(a), 7(b)(1), 8, 9, 11(a), 12-14, 16, 17, & 19 of Table A hereof. The field work was completed on September 30, 2022. Unauthorized alteration or addition to a survey map bearing a licensed land surveyor's seal is a violation of section

7209, sub-division 2, of the New York State Education Law. The alteration of boundary survey maps by anyone other the the original preparer is misleading, confusing and not in the general welfare and benefit of the public. Only boundary survey maps with the surveyor's embossed seal are genuine true and correct copies of the

Christopher E. Jacobs

surveyor's original work and opinion.

Registered Land Surveyor No.050952 In The State Of New York Date Of Survey: October 4, 2022 Date Of Last Revision: Project Number: 2022.123.001



**FINAL SURVEY** 

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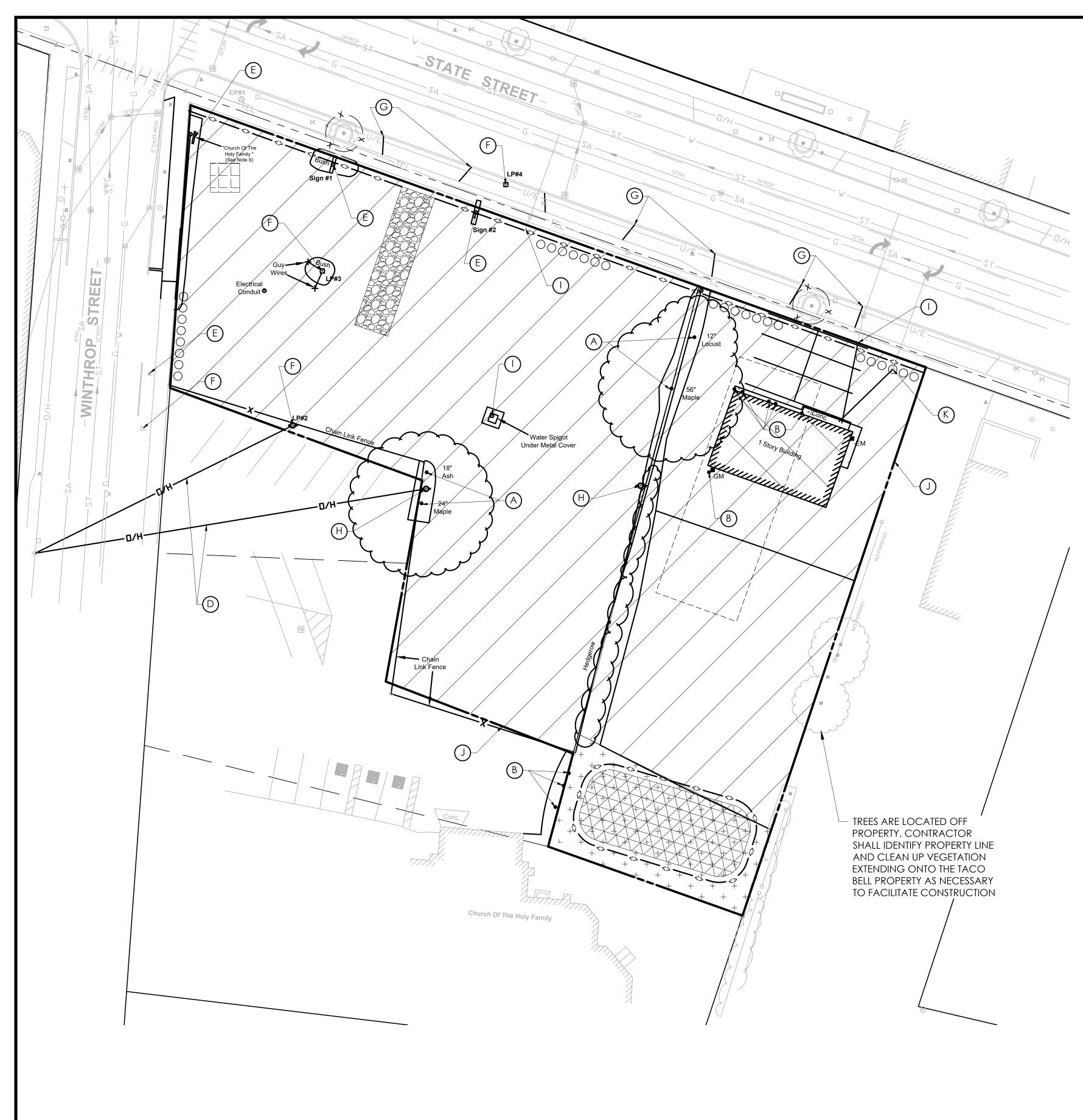


**POINTS:** 

Northing = 1448444.3265 Elevation = 492.05 (NAVD 88)

Northing = 1448347.7684 Elevation = 494.31 (NAVD 88)

Elevation = 495.42 (NAVD 88)



### **DEMOLITION LEGEND:**

- (A) TREE TO BE REMOVED
- B BOLLARD TO BE REMOVED
- C CHAIN LINK FENCE TO BE REMOVED
- (D) OVERHEAD LINES TO BE REMOVED
- E SIGN TO BE REMOVED
- (F) LIGHTPOLE TO BE REMOVED/RELOCATED
- G CURB CUTS TO BE REMOVED AND REPLACE WITH CURB REVEAL TO MATCH EXISTING. REFER TO GRADING AND
- DRAINAGE PLAN FOR ADDITIONAL INFORMATION
- (H) UTILITY POLE TO BE REMOVED
- ABANDON OR REMOVE UTILITY TO ROW LINE IN ACCORDANCE WITH CITY REQUIREMENTS
- (J) SAW-CUT PAVEMENT AT PROPERTY LINE
- K REMOVE SANITARY LATERAL TO CLEANOUT. CLEANOUT

### **DEMOLITION NOTES:**

- 1. PRIOR TO DEMOLITION OCCURRING, ALL EROSION CONTROL DEVICES ARE TO BE INSTALLED. EROSION CONTROL SHALL BE PERFORMED IN ACCORDANCE WITH FEDERAL, STATE AND LOCAL REQUIREMENTS. REFER TO EROSION CONTROL PLANS FOR ADDITIONAL INFORMATION. ALL DEMOLITION WORK SHALL BE COORDINATED WITH EROSION CONTROL PLANS, INCLUDING LIMITS OF DISTURBANCE, PLACEMENT CONSTRUCTION FENCING, AND STABILIZED CONSTRUCTION EXIT.
- 2. ALL SIDEWALKS, SLABS, FOUNDATIONS, DEBRIS, AND MISCELLANEOUS DEMOLITION ALL ITEMS SHOWN IN CONSTRUCTION DOCUMENTS SHALL BE SPOILED OFF-SITE IN A LEGAL MANNER STUMPS AND BRUSH MAY NOT BE BURIED AND MUST BE REMOVED AND DISPOSED OF OFFSITE. PAVEMENT REMOVED WILL BE ALLOWED AS RECYCLED FILL ONLY AFTER REVIEW AND APPROVAL BY CLIENT AND GEOTECHNICAL ENGINEI THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED FOR DEMOLITION, TRANSPORTING, AND DISPOSAL. PROOF OF LEGAL DISPOSAL SHALL E PROVIDED UPON CLIENT REQUEST.
- 3. ALL EXISTING ON-SITE UTILITIES SHALL REMAIN UNLESS DESIGNATED FOR REMOVAL OF ABANDONMENT. ALL UTILITIES EXISTING WITHIN THE PROPOSED BUILDING ENVELOPING SHALL BE REMOVED PROTECTED OR RELOCATED AS NECESSARY TO ALLOW FOR TH PROPOSED IMPROVEMENTS. PROTECT ALL EXISTING UTILITIES TO REMAIN.
- 4. CONTRACTOR TO REMOVE, RELOCATE AND/OR PROVIDE TEMPORARY UTILITY SERVICES, WHEN APPLICABLE. ALL EXISTING BUILDINGS, FOUNDATIONS, BASEMENTS CONNECTING IMPROVEMENTS, DRAIN PIPES, SANITARY SEWER PIPES, POWER POLES AND GUY WIRES, WATER METERS AND WATER LINES, WELLS, SIDEWALKS, SIGN POLES UNDERGROUND GAS, TANKS, VAULTS, STRUCTURES, ASPHALT, ETC. SHOWN AND NO SHOWN, WITHIN THE CONSTRUCTION LIMITS AND WHERE NEEDED, SHALL BE REMOV OR RELOCATED TO ALLOW FOR NEW CONSTRUCTION, AS SHOWN. ALL WORK SHAL BE IN ACCORDANCE WITH GOVERNING AUTHORITIES SPECIFICATIONS AND SHALL E APPROVED BY SUCH. ALL COST SHALL BE INCLUDED IN BASE BID. REFER TO THE SURVEY FOR ADDITIONAL INFORMATION. ALL FACILITIES TO BE REMOVED SHALL BE UNDERCUT TO SUITABLE MATERIALS AND BROUGHT TO GRADE WITH SUITABLE COMPACTED FILL MATERIAL PER THE SPECIFICATIONS. RECYCLED FILL SHALL NOT BE USED IN BUILDING PAD.
- 5. THE CONTRACTOR SHALL COORDINATE WITH RESPECTIVE UTILITY COMPANIES AND PROVIDE PROPER NOTIFICATION PRIOR TO THE REMOVAL AND/OR RELOCATION O UTILITIES. THE CONTRACTOR SHALL COORDINATE WITH THE UTILITY COMPANY CONCERNING PORTIONS OF WORK WHICH MAY BE PERFORMED BY THE UTILITY COMPANY'S FORCES AND ANY FEES WHICH ARE TO BE PAID TO THE UTILITY COMPA FOR THEIR SERVICES. THE CONTRACTOR IS RESPONSIBLE FOR PAYING ALL FEES AND CHARGES, INCLUDING INSPECTION AND TESTING AND INCLUDE IN BASE BID. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL RELOCATIONS, INCLUDING BUT NOT LIMITED TO, ALL UTILITIES, STORM DRAINAGE, SIGNS, ETC. AS REQUIRED. SOME UTILI MAY BE CONSIDERED PRIVATE AND NOT LOCATED BY UTILITY COMPANIES. VERIFY T LOCATIONS OF ALL LATERALS, SERVICE CONNECTIONS, LIGHTING CIRCUITS, SIGN CIRCUITS, AND OTHER UTILITIES AND PROCEED WITH CAUTION AROUND ANY ANTICIPATED FEATURES.
- 6. UTILITIES DETERMINED TO BE ABANDONED AND LEFT IN PLACE SHALL BE GROUTED W LOW DENSITY CELLULAR CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF PSI. ALL ABANDONED PIPES WITHIN 2' OF THE FINISHED GRADE OR WITHIN THE PROPOSED BUILDING FOOTPRINT SHALL BE REMOVED. PIPES OUTSIDE OF THE PROPOSED BUILDING FOOTPRINT MAY BE REMOVED OR ABANDONED, AT CONTRACTORS DISCRETION. ALL STRUCTURES NOT BEING REUSED SHALL BE COMPLETELY REMOVED. AT MANHOLES AND STRUCTURES TO REMAIN, ABANDONE PIPES SHALL BE CUT AND PLUGGED AND THE REMAINING MANHOLE/STRUCTURE REPAIRED TO PROVIDE A SOLID, SOUND AND WATERPROOF STRUCTURE.
- 7. CAUTION: NOTICE TO CONTRACTOR: THE CONTRACTOR IS SPECIFICALLY CAUTION THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THES PLANS IS BASED ON THE LISTED REFERENCES, RECORDS OF VARIOUS UTILITY COMPANIES AND, WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE AND THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THEIR ACCURACY. CONTRACTOR IS RESPONSIBLE TO VERIFY GRADES AND UTILITIES (INCLUDING INTEGRITY) SHOWN ON SURVEY PLAN PRIOR TO START OF ANY WORK. ANY AND ALL DISCREPANCIES ARE 1 BE DOCUMENTED AND SUBMITTED TO THE OWNER AT THE TIME OF DISCOVERY. THIS WORK SHALL BE COMPLETED EARLY ENOUGH TO AVOID DELAYS AND ALLOW FOR REDESIGN IF REQUIRED. THE CONTRACTOR SHALL MAKE EXPLORATION EXCAVATIO TO LOCATE EXISTING UNDERGROUND UTILITIES SUFFICIENTLY AHEAD OF CONSTRUCTION TO PERMIT REVISIONS AS REQUIRED TO MEET EXISTING CONDITIONS IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.

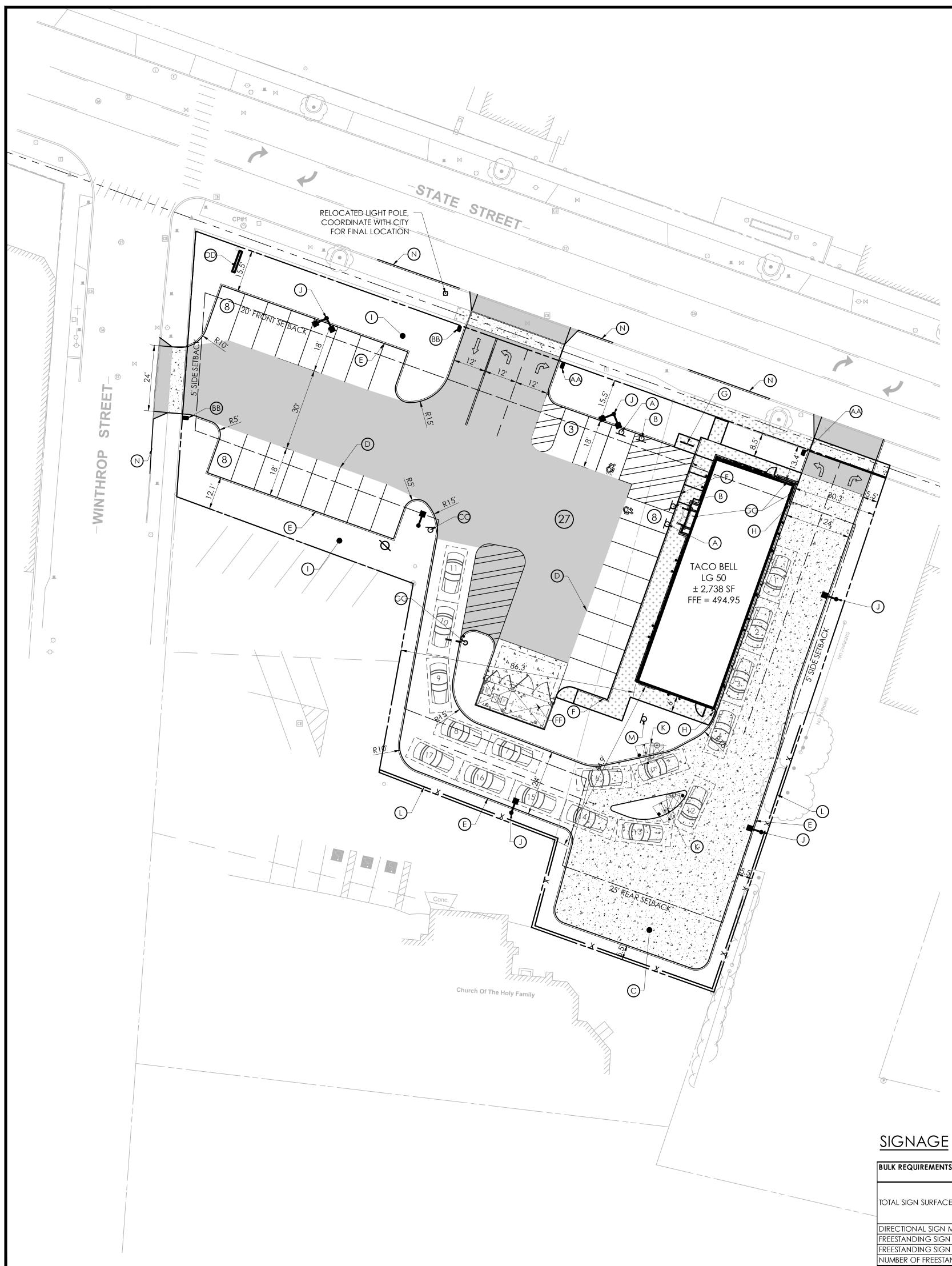
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REVISED ON OCT BY JACOBS LANE

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1 1 1 1 1	<ul> <li>SOME UTILITIE COMPANIES. LIGHTING CIR CAUTION AR SHALL BE TEST USE. TESTING TESTING NECT NOTED IN PIP AN ACTIVE U</li> <li>CONTRACTO WHERE IT IS R REQUIRED TO ANY DAMAG CONTRACTO PAVEMENT A</li> <li>MANHOLES, O GRATES TO R APPLICABLE.</li> <li>ABANDON EP OPERATIONA STRUCTURES) SAFE SEQUEN</li> <li>FOR ALL UTILIT STRUCTURAL STATE REQUIRE</li> <li>CONTRACTO RESPONSIBLE</li> <li>THE CONTRA REQUIRED AB ASSOCIATED EASEMENTS, O</li> <li>REMOVE EXIS PROPOSED ST OPERATIONS</li> </ul>	CE LINES AND CAP ALL LIN IS MAY BE CONSIDERED PR VERIFY THE LOCATIONS O CCUITS, SIGN CIRCUITS, AN OUND ANY ANTICIPATED F TED PRIOR TO ABANDONIN SHALL INCLUDE FLOW TEST ESSARY TO VERIFY PIPES AN ES THE CONTRACTOR SHA TILITY OR DRAINAGE CON R MAY LIMIT SAW-CUT & P EQUIRED AS SHOWN ON TO PROVIDE A SMOOTH, PRO EQUIRED AS SHOWN ON TO PROVIDE A SMOOTH, PRO E IS INCURRED ON ANY O R SHALL BE RESPONSIBLE F ND CONCRETE SHALL BE S CATCH BASINS, CLEANOUT EMAIN SHALL BE PROTECTION KISTING UTILITIES ONLY AFTING L (I.E. STORM DRAINAGE, Y . IT IS THE CONTRACTOR'S ICING OF ABANDONMENT TY LINES DESIGNATED TO B BACKFILL WITHIN TRENCH A R SHALL MAINTAIN POSITING FOR ALL DEWATERING, IF CTOR SHALL BE RESPONSIBLE OVE AND BEYOND THOSE FEES. THIS INCLUDES ANY OR GRADING RELEASES. TING PAVEMENT MARKING IN SUCH A MANNER THAT PR LEFT IN A PATTERN THAT	21VATE AND NOT LC F ALL LATERALS, SEI D OTHER UTILITIES A EATURES. SANITAR IG THEM TO VERIFY ING, AIR TESTING, T RE PROPERLY DISCO LL TRACK DOWN SO NECTION. AVEMENT REMOVA HESE CONSTRUCTIO DPERLY DRAINING, F THE SURROUNDIN OR ITS REMOVAL A AW-CUT PRIOR TO TS, VALVE BOXES, FI ED AND ADJUSTED ER CRITICAL NEW S WATER, SANITARY S RESPONSIBILITY TO OF UTILITIES. E REMOVED, PLAC AFTER REMOVAL. F VITHIN R.O.W. (E DRAINAGE AT AI REQUIRED. SLE TO SECURE ALL PREVIOUSLY OBTA TEMPORARY ACCE SS WHICH INTERFER DING, SCRAPING, S, THE FINISHED PAVE	DCATED BY UTILITY RVICE CONNECT IND PROCEED WI Y AND STORM SE THEY ARE NO LC TELEVISING, AND DNNECTED. IF FLC OURCE AND VER AL TO ONLY THOS DN PLANS OR AS PAVEMENT SURF, IG PAVEMENT, ET IND REPAIR. EXIST REMOVAL. RAMES, COVERS TO FINAL GRADE YSTEMS ARE IN PL SERVICES TO EXIST PROVIDE FOR PR E AND COMPACE OLLOW ANY LOC LL TIMES AND IS PERMITS AND EAX INED AND PAY A ESS PERMITS, CON PERMITS AND EAX INED AND PAY A ESS PERMITS, CON	Y IONS, TH WERS DINGER IN OTHER DW IS IFY IT IS NOT E AREAS MAY BE ACE. IF C. THE ING AND S, IF ACE AND ING COPER AND S, IF CAL OR SEMENTS NY ISTRUCTION WITH THE R OTHER S NOT	direction of Architect, any way. A to affix his of by his or here Copying, Print these protection Therefore, Engineer Engineer	Drawing lation of law for any p f licensed Architect, P or Land Surveyor to all ny licensee who alters r her seal and to add signature and the spa- or rev DO NOT SC hting, Software and ot ints can stretch or shril scaling of this drawin of Record with any ne clarific Second Second Second Second Second Child Second Second Child Second	Alteration person, unless acting un rofessional Engineer, La ter any item on this do the notation "Altered B ecific description of the vision. CALE PLANS her processes required nk the actual paper or g may be inaccurate. eed for additional dime	andscap cument ired by lo By follow e alteration layout. Contac ensions of Contac E 4
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### SITE LEGEND:

- ACCESSIBLE PARKING SIGN, POST & BOLLARD (REFER TO A DETAIL)
- "NO PARKING ANY TIME" SIGN, POST & BOLLARD (REFER B TO DETAIL)
- C SNOW STORAGE AREA
- D SYSL/4" PARKING STALL STRIPING
- (E) CONCRETE CURB (REFER TO DETAIL)
- (F) NOT USED
- G BIKE RACK (REFER TO DETAIL)
- (H) BOLLARDS (REFER TO ARCH. PLAN DETAIL)
- ☐ LAWN/MULCH AREA (REFER TO PLANTING PLAN FOR
- (J) LIGHT POLE (REFER TO DETAIL)
- MENU BOARD, CANOPY & SPEAKER BOX (REFER TO K MENU BO DETAIL)
- (L) 6' BOARD ON BOARD FENCE (REFER TO DETAIL)
- (M) GREASE INTERCEPTOR SIGN
- HEADER CURB TO BE REPLACED WITH CURB REVEAL IN ACCORDANCE WITH CITY REQUIREMENTS, MATCH (N) GRADE AND ALIGNMENT
- (AA) TACO BELL EXIT SIGN (PROVIDED BY SIGN VENDOR)
- TACO BELL ENTRANCE SIGN (PROVIDED BY SIGN
- BB VENDOR)
- CC TACO BELL VENDOR) TACO BELL DRIVE THRU SIGN (PROVIDED BY SIGN
- D TACO BELL PYLON SIGN (PROVIDED BY SIGN VENDOR)
- (EE) CLEARANCE BAR (REFER TO DETAIL)
- CONCRETE DUMPSTER PAD AND ENCLOSURE (REFER TO
- ARCH. PLANS)
- TACO BELL BUILDING MOUNTED SIGNAGE (REFER TO GG ARCH. PLANS)

### **GENERAL NOTES:**

- 1. ALL IMPROVEMENTS SHALL BE IN ACCORDANCE WITH THE MOST RECENT STANDARDS AND SPECIFICATIONS OF THE CITY OF WATERTOWN AND/OR THE APPROPRIATE WATER, SEWER AND/OR DRAINAGE DISTRICTS, AND/OR OTHER AUTHORITIES HAVING JURISDICTION.
- 2. ALL EXISTING BUILDING(S), SITE, ROADWAY, UTILITY, BOUNDARY, AND TOPOGRAPHY INFORMATION SHOWN ON THIS PLAN IS REPRESENTED BASED ON USE OF THE LISTED REFERENCES. CONTRACTOR TO VERIFY LOCATION AND LIMITS OF WORK PRIOR TO STARTING. ANY CHANGES OR CONFLICTS DISCOVERED SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER AND ENGINEER. IF CONTRACTOR DOES NOT ACCEPT EXISTING TOPOGRAPHY AS SHOWN ON THE PLANS, WITHOUT EXCEPTION, HE SHAL HAVE MADE, AT HIS EXPENSE, A TOPOGRAPHIC SURVEY BY A REGISTERED LAND SURVEYOR AND SUBMIT IT TO THE OWNER FOR REVIEW.
- 3. CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR EXACT LOCATIONS AND DIMENSIONS OF VESTIBULE, SLOPED PAVING, EXIT PORCHES, RAMPS, PRECISE BUILDING DIMENSIONS, AND EXACT BUILDING UTILITY ENTRANCE LOCATIONS. ALL PAVING, CURBING, FLATWORK, SIDEWALKS, FENCING, BOLLARDS, ETC., WHICH CONFLICT WITH NEW CONSTRUCTION ARE TO BE DEMOLISHED AND DISPOSED OF IN ACCORDANCE WITH ANY LOCAL, STATE, OR FEDERAL REGULATIONS.
- 4. CONTRACTOR MUST PROTECT THE PUBLIC AT ALL TIMES WITH FENCING, BARRICADES, ENCLOSURES, ETC. CONTRACTOR SHALL MAINTAIN ALL EXISTING PARKING. SIDEWALKS, DRIVES, ETC. OUTSIDE OF WORK LIMITS CLEAR AND FREE FROM ANY CONSTRUCTION ACTIVITY AND/OR MATERIAL TO ENSURE EASY AND SAFE PEDESTRIAN AND VEHICULAR TRAFFIC TO AND FROM THE SITE.
- 5. REFER TO THE SURVEY FOR THE PROPERTY BOUNDARY INFORMATION (E.G. LOT AREA, BEARINGS, DISTANCES, ETC).
- 6. ANY DAMAGE TO EXISTING SIDEWALKS ON WINTHROP STREET AS A RESULT OF GENERAL CONSTRUCTION MUST BE REPAIRED TO THE SATISFACTION OF THE CITY ENGINEER.

### SITE DATA:

### LOCAL JURISDICTION:

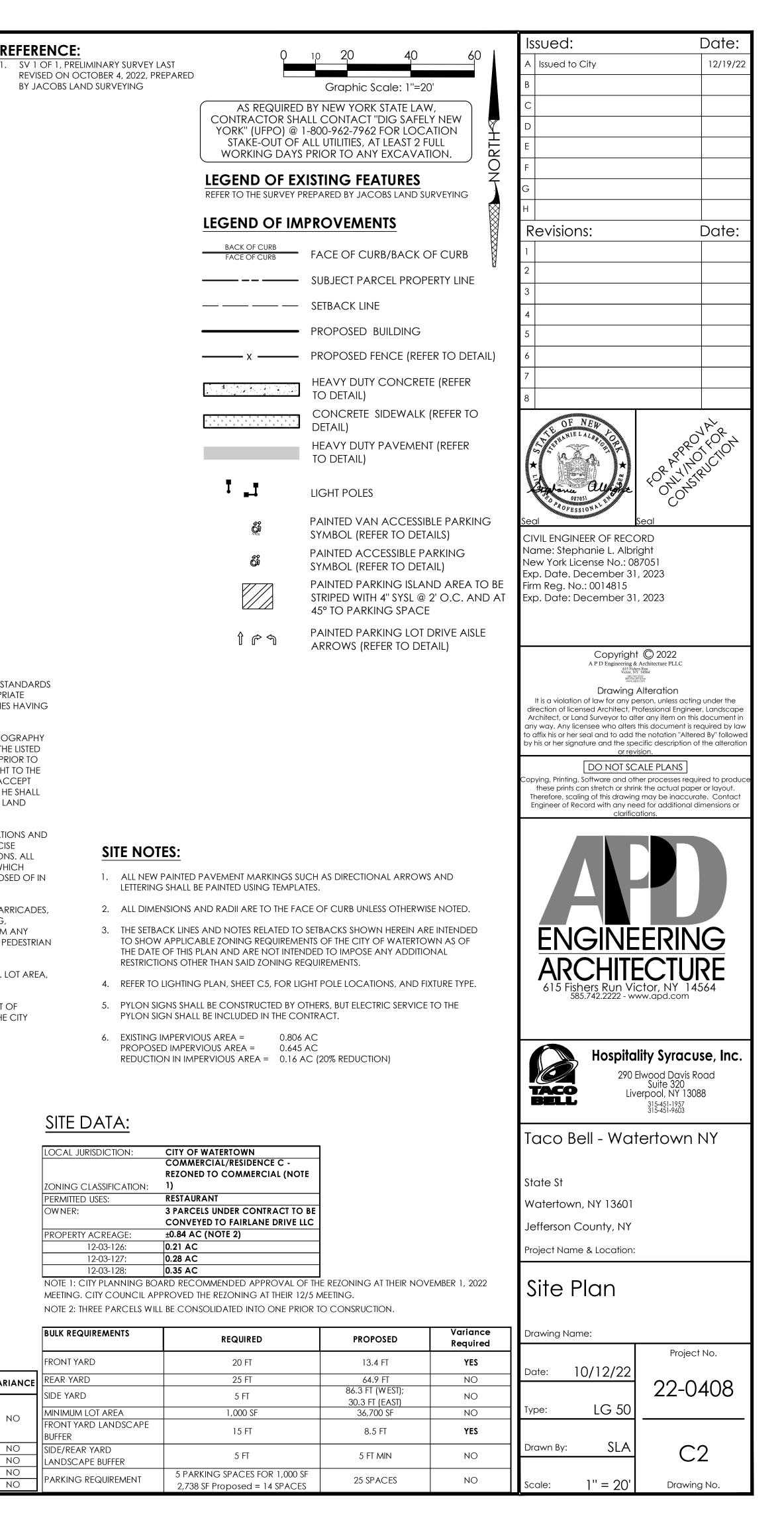
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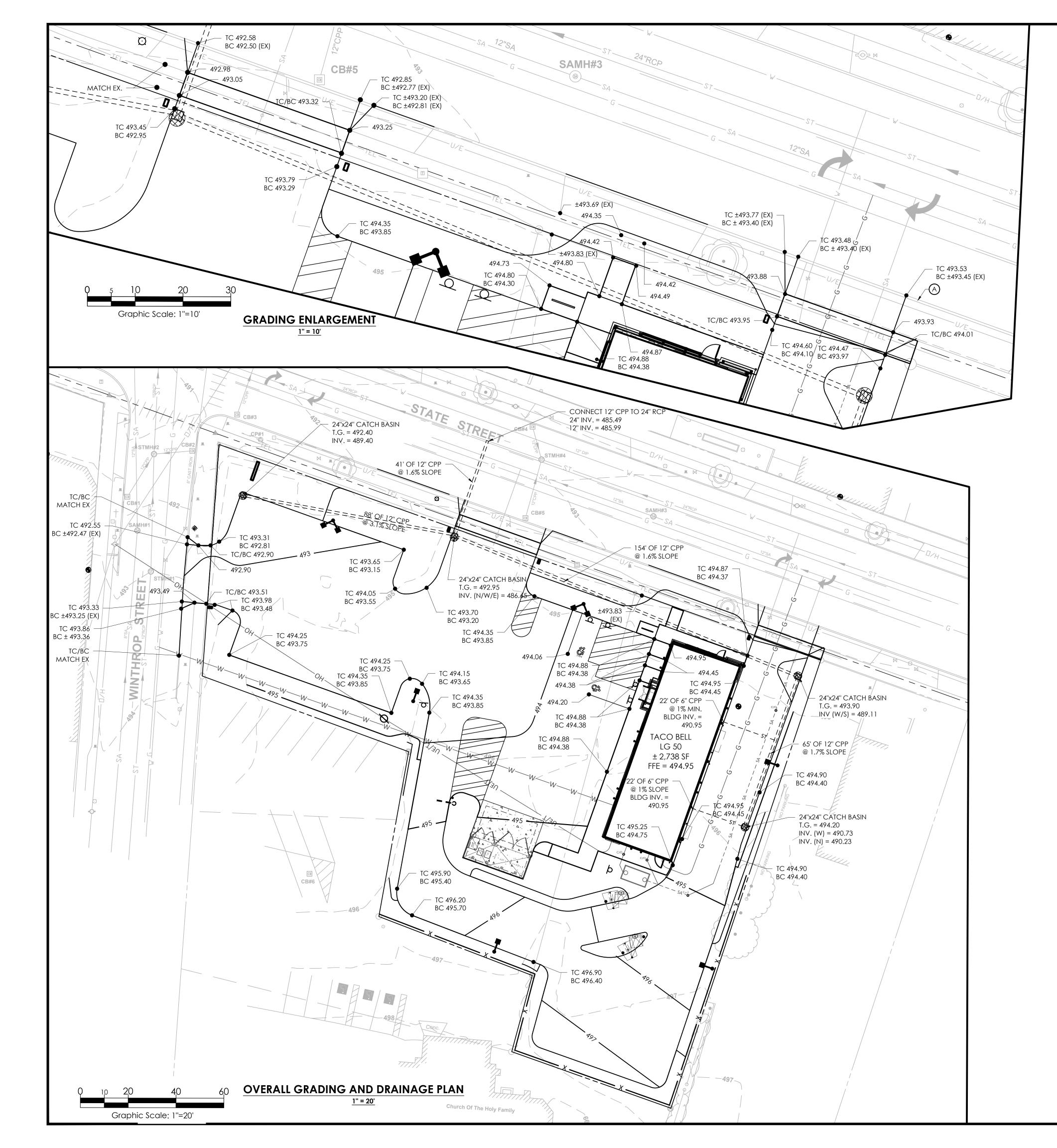
OW NER:

FRONT YARD

BULK REQUIREMENTS	ALLOWED	PROPOSED	VARIANCE	REAR YARD
TOTAL SIGN SURFACE AREA	2 SF PER 1 LF OF BUILDING FRONTAGE, MAX SIGN SURFACE AREA IS 200 SF (87 LF +30 LF =	(2x27.26)+13.58 +105.6 + (5x4) = 193.7 SF	NO	SIDE YARD MINIMUM LO FRONT YARI
DIRECTIONAL SIGN MAX AREA	200 SF ALLOWED) 4 SF	4 SF	NO	BUFFER SIDE/REAR
FREESTANDING SIGN AREA	NA	105.6 SF	NO	LANDSCAP
FREESTANDING SIGN MAX HEIGHT	50 FT	40 FT	NO	PARKING R
NUMBER OF FREESTANDING SIGNS	1	]	NO	

## **REFERENCE:**





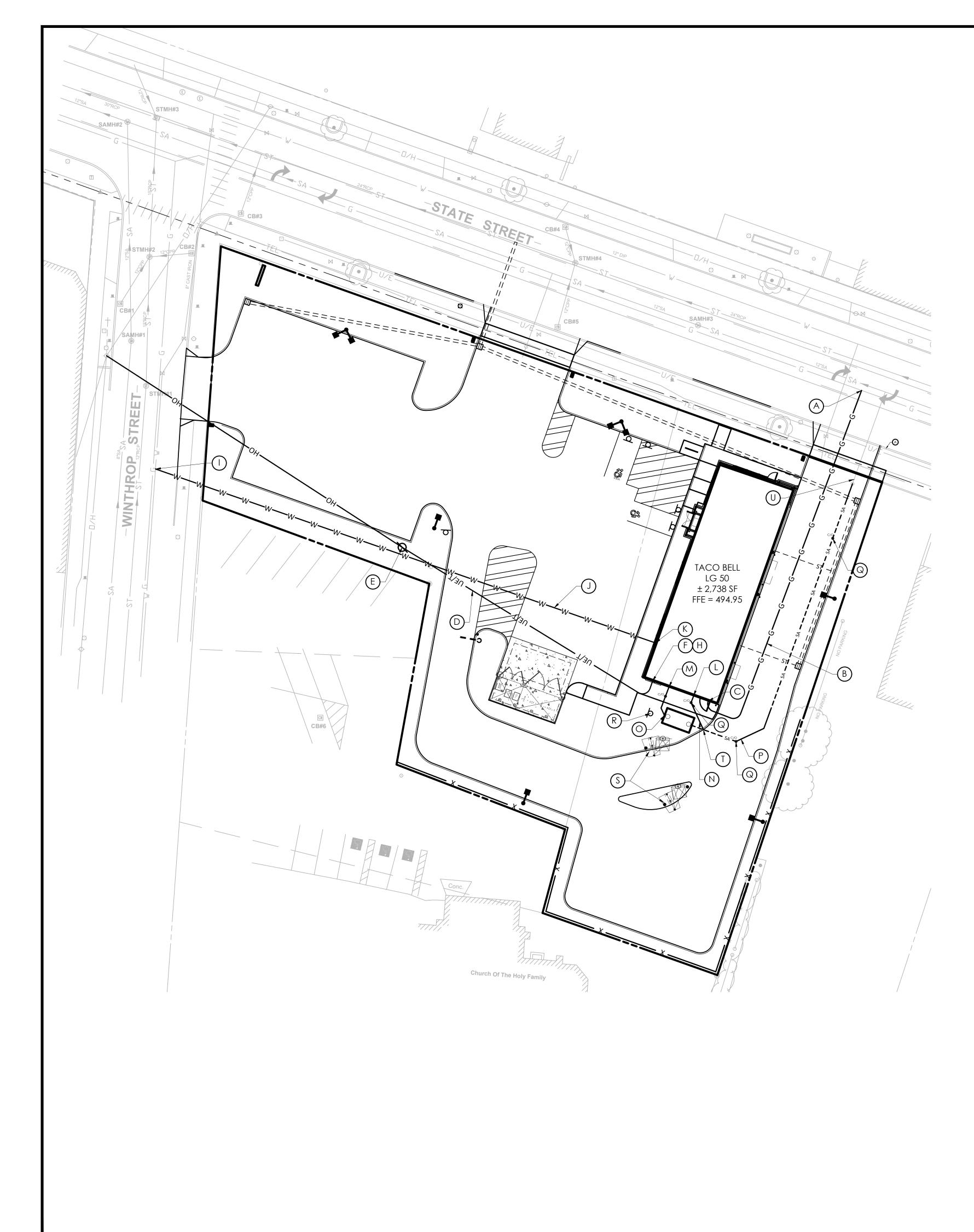
#### REFERENCE: 1. SV 1 OF 1, PRELIM

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# GRADING AND DRAINAGE

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### **UTILITY LEGEND:**

- CONNECTION TO EXISTING GAS MAIN. CONTRACTOR (A) TO COORDINATE WITH GAS COMPANY FOR
- INSTALLATION AND FINAL CONNECTION
- (B) UNDERGROUND GAS SERVICE
- (C) GAS METER AND SERVICE ENTRY UNDERGROUND ELECTRIC AND TELEPHONE SERVICE.
- CONTRACTOR SHALL COORDINATE WITH ELECTRIC AND (D) TELEPHONE COMPANIES FOR FINAL CONNECTION AND INSTALL CONDUIT AND REQUIRED PULL BOXES PER THEIR REQUIREMENTS.
- POLE-MOUNTED ELECTRIC TRANSFORMER. CONTRACTOR SHALL COORDINATE EXACT LOCATION, (E SIZE, INSTALLATION, AND PROTECTION OF TRANSFORMER WITH ELECTRIC COMPANY.
- (F) ELECTRIC METER AND SERVICE ENTRY.
- G NOT USED
- (H) TELEPHONE SERVICE ENTRY
- (I) CONNECT TO EXISTING WATER SERVICE
- (J) 1.5" TYPE K DOMESTIC WATER SERVICE
- (K) DOMESTIC WATER SERVICE ENTRY
- SANITARY SERVICE ENTRY, CONNECT TO TWO WAY
- BUILDING SEWER CLEANOUT. BLDG INV. = 489.45
- M GREASE INTERCEPTOR LATERAL CONNECT TO TWO WAS BUILDING SEWER CLEANOUT. BUILDING INV. = 489.45 GREASE INTERCEPTOR LATERAL CONNECT TO TWO WAY
- (N) 15' OF 4" PVC @ 2.0% MIN. SLOPE
- O 13' OF 6" PVC @ 1.0% SLOPE ; GREASE INTERCEPTOR INV. IN = 489.32
- GREASE INTERCEPTOR INV. OUT = 489.07 129' OF 6" PVC @ 1.0% MIN. SLOPE
- Q CLEANOUT (SPACED AT 90' MAX)
- (R) GREASE INTERCEPTOR SIGN
- REFER TO BUILDING PLANS FOR SITE ELECTRICAL
- (S) COMPONENT (MENU BOARD, SPEAKER BOX, CLEARANCE BARS, SITE LIGHTING, ETC.)
- (T) CONNECT 4" & 6" WITH WYE FITTING
- GENERAL CONTRACTOR TO VERIFY EARLY IN THE CONSTRUCTION PROCESS THAT THE EXISTING LATERAL IS
- (U) 6" PVC AND THAT PROPOSED DESIGN CAN TIE INTO EXISTING INVERT. EXISTING CLEANOUT INVERT ASSUMED TO BE ± 485.80

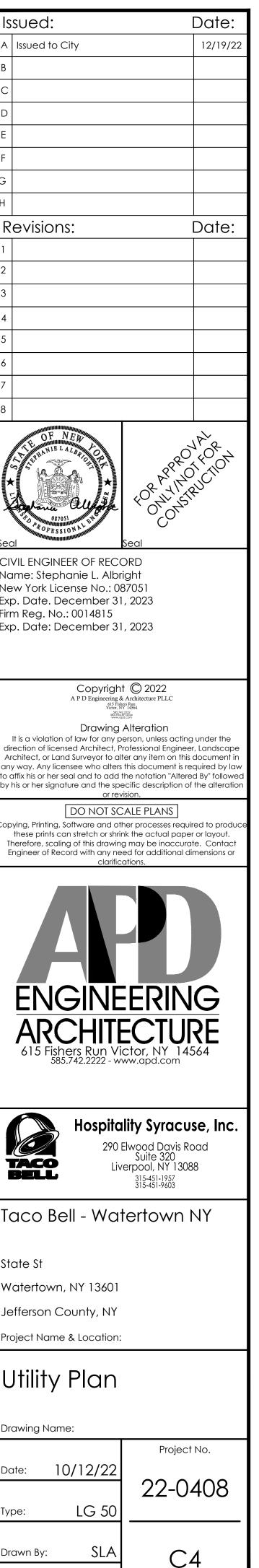
### UTILITY NOTES:

- 1. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY FOR NOTIFYING ANY UTILITY COMPANY WHICH MAINTAINS A UTILITY LINE WITHIN THE BOUNDARIES OF THE PROJECT PRIOR TO THE START OF CONSTRUCTION. THE LOCATION AND/OR ELEVATION OF ALL EXISTING UTILITIES SHOWN ON THESE PLANS IS BASED ON LISTED REFERENCES, RECORDS OR THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD AND ARE GIVEN FOR THE CONVENIENCE OF THE CONTRACTOR. DRAWINGS DO NOT PURPORT TO SHOW ALL EXISTING UTILITIES. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THEIR ACCURACY. THE CONTRACTOR SHALL ASSUME THE RESPONSIBILITY FOR ANY DAMAGE TO EXISTING UTILITY LINES AS A RESULT OF HIS ACTIVITIES, WHETHER THESE LINES ARE SHOWN ON THE PLANS OR NOT. CONTRACTOR IS RESPONSIBLE FOR REPAIRS OF DAMAGE TO ANY EXISTING UTILITY DURING CONSTRUCTION AT NO COST TO THE OWNER. THE CONTRACTOR SHALL TAKE WHATEVER MEASURES NECESSARY TO LOCATE AND PROTECT EXISTING UTILITIES, STRUCTURES, AND OTHER FACILITIES TO REMAIN. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.
- 2. CONTRACTOR TO VERIFY EXISTING UTILITIES PRIOR TO INSTALLATION OF PROPOSED IMPROVEMENTS. CONTRACTOR SHALL VERIFY THE LOCATION, DEPTH, SIZE, MATERIAL, AND INTEGRITY OF EXISTING UTILITIES. ANY CONFLICTS WITH PROPOSED WORK SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION IN SUFFICIENT TIME TO ALLOW FOR REDESIGN WITHOUT IMPACT TO PROJECT SCHEDULE.
- 3. SEE SPECIFICATIONS FOR BACKFILLING AND COMPACTION REQUIREMENTS ON UTILITY TRENCHES.
- 4. THE CONTRACTOR SHALL ADJUST TOPS OF CLEANOUTS, MANHOLES, VALVES, HANDHOLES, ETC. TO REMAIN TO PROPOSED FINISHED GRADE, AS NECESSARY.
- 5. CONTRACTOR TO COORDINATE AND APPLY FOR ALL SERVICE REQUESTS TO ALL NECESSARY UTILITY COMPANIES AS EARLY AS POSSIBLE IN THE CONSTRUCTION PROCESS IN ORDER TO AVOID DELAYS IN SERVICE.
- 6. CONTRACTOR IS RESPONSIBLE FOR COORDINATING AND COMPLYING WITH THE SPECIFICATIONS OF THE UTILITY COMPANIES AND LOCAL AUTHORITIES WITH REGARDS TO MATERIALS, INSTALLATION, INSPECTION, TESTING, CLEANING, CERTIFICATION, RECORD MAP, AND AS-BUILT REQUIREMENTS OF THE UTILITY COMPANIES AND AUTHORITIES HAVING JURISDICTION. UNDERGROUND UTILITIES SHALL BE INSTALLED, INSPECTED AND APPROVED BY ALL AUTHORITIES HAVING JURISDICTION BEFORE BACKFILLING. THE CONTRACTOR SHALL CONDUCT ALL REQUIRED TESTS TO THE SATISFACTION OF THE RESPECTIVE UTILITY COMPANIES AND THE OWNER'S INSPECTING AUTHORITIES.
- 7. CONTRACTOR SHALL NOTIFY THE UTILITY AUTHORITIES INSPECTORS 48 HOURS BEFORE CONNECTING TO ANY EXISTING LINE. ALL NECESSARY INSPECTIONS AND/OR CERTIFICATIONS REQUIRED BY CODES AND/OR UTILITY SERVICE COMPANIES SHALL BE PERFORMED PRIOR TO ANNOUNCED BUILDING POSSESSION AND THE FINAL CONNECTION OF SERVICE.
- 8. CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS AND SPECIFICATIONS FOR ACTUAL LOCATION, SIZE, AND INVERT OF ALL UTILITY ENTRANCES. CONTRACTOR SHALL COORDINATE INSTALLATION OF UTILITIES, IN SUCH A MANNER AS TO AVOID CONFLICTS AND ENSURE PROPER DEPTHS ARE ACHIEVED AS WELL AS COORDINATING WITH UTILITY COMPANIES FOR REQUIREMENTS AS TO LOCATION AND SCHEDULING FOR TIE-INS/CONNECTIONS PRIOR TO CONNECTING TO EXISTING UTILITIES.

#### **REFERENCE:** 1. SV 1 OF 1, PRELIMINA

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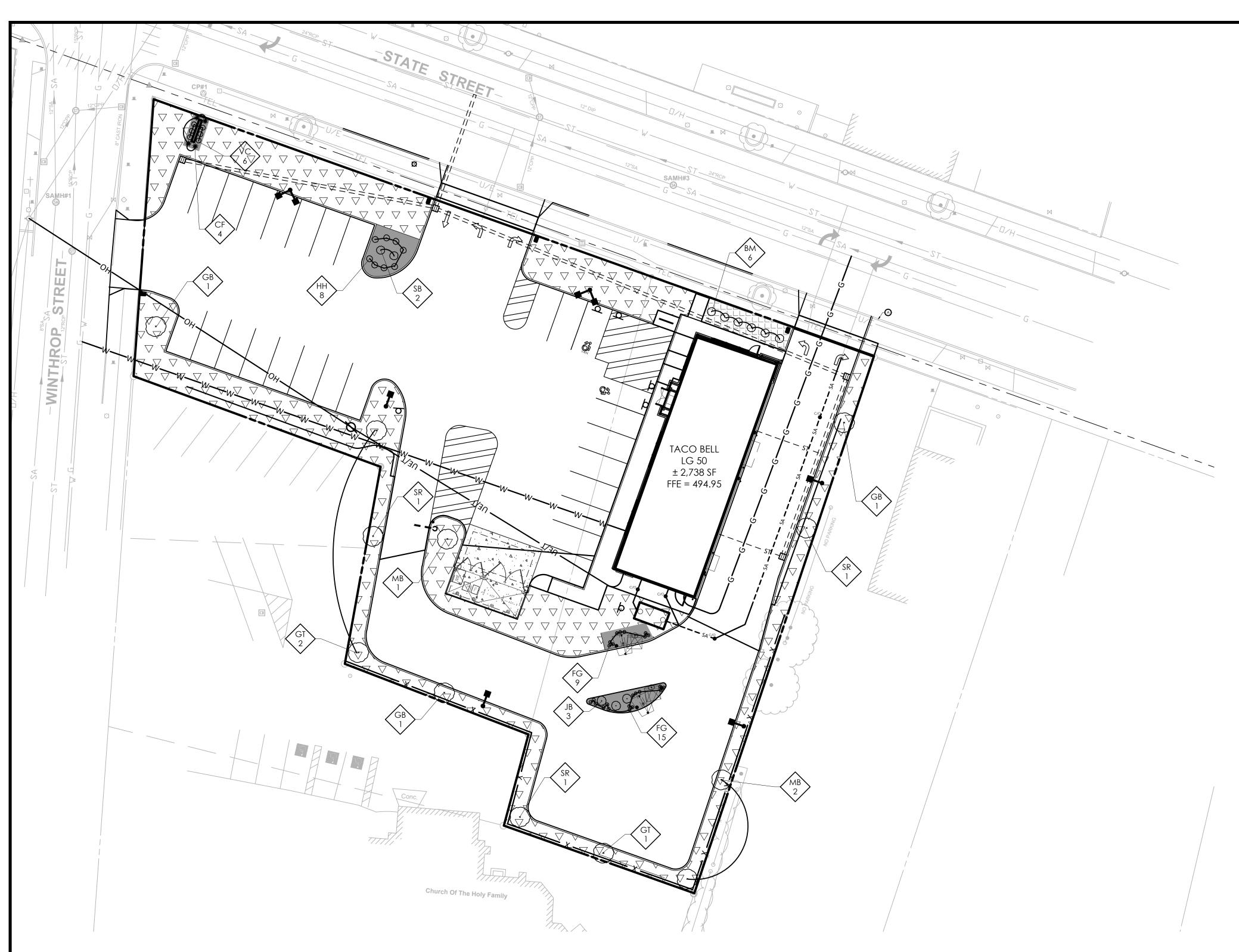
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13. ALL STORM A	ON WILL BE PROVIDED. ND SANITARY SEWER CO IN ACCORDANCE WITH	ONNECTIONS TO EX		SHALL BE Water
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	NY AND PROVIDE ANY ET C5 FOR LIGHTING DE		1.	Drawn E



1'' = 20'

Drawing No.

Scale:



### LANDSCAPE NOTES:

- 1. REFER TO SPECIFICATIONS FOR SEED MIXES AND ADDITIONAL INFORMATION.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HIS OWN QUANTITY TAKEOFF.
- 3. THE CONTRACTOR SHALL PERFORM A ROUGH FIELD STAKE OUT OF ALL PLANT MATERIAL AND SHRUB BEDS. LOCATIONS SHOWN ON THE PLAN CONVEY DESIGN INTENT ONLY. ACTUAL LOCATIONS WILL BE AS DIRECTED BY THE OWNER AT THE TIME OF INSTALLATION.
- 4. THE CONTRACTOR IS HEREBY NOTIFIED THAT UNDERGROUND UTILITIES EXIST. CONTRACTOR SHOULD OBTAIN CURRENT UTILITY RECORD MAPS AND NOTIFY ALL UTILITY COMPANIES PRIOR TO COMMENCING WORK.
- 5. STAKE PLANTS AS INDICATED OR AS APPROVED IN THE FIELD. IF OBSTRUCTIONS ARE ENCOUNTERED THAT ARE NOT SHOWN ON THE DRAWINGS, DO NOT PROCEED WITH PLANTING OPERATIONS UNTIL ALTERNATIVE PLANT LOCATIONS HAVE BEEN SELECTED. STAKES AND WRAPPING ARE TO BE REMOVED BY THE CONTRACTOR AT THE END OF THE GUARANTEE PERIOD.
- 6. SHRUBS SHALL NOT BE PLACED WITHIN TWO (2) FEET OF A CURB.
- 7. TREES SHALL BE A MINIMUM OF 5' FROM ROOT BALL TO UNDERGROUND UTILITIES AND 20' FROM OVERHEAD UTILITIES.
- 8. REFER TO EROSION CONTROL PLAN FOR LIMITS OF STEEP SLOPE SEED MIX. FOR ANY DISCREPANCIES BETWEEN LIMITS OF STEEP SLOPE SEED, LAWN SEED, AND/OR MULCH BETWEEN THE PLANTING PLAN AND THE EROSION CONTROL PLAN, THE PLANTING PLAN SHALL TAKE PRECEDENCE.

### LIGHTING NOTES:

	PLANT LIST								
QUANT.	KEY	LATIN NAME	COMMON NAME	CAL.	ROOTS	HT. OR. SP			
3	MB	MALUS BACCATA	SNOW CRAB APPLE		B&B	20'-25' HT			
4	CF	CALAMAGROSTIS X ACUTIFLORA 'KARL FOERSTER'	KARL FOERSTER FEATHER REED GRASS		#2 CONT.	24" HT, 4' O.C.			
24	FG	FESTUCA GLAUCA	ELIJAH BLUE FESCUE		#2 CONT.	12" HT, 18" O.C.			
6	VC	VIBURNUM CARLESII	KOREANSPICE VIBURNMUM		B&B	24" HT (AT PLANTING), 5' O.0			
6	BM	BUXUS MICROPHYLLA 'WINTERGREEN'	WINTERGREEN BOXWOOD		#2 CONT.	18" MIN, 3' O.C.			
2	SB	SPIRAEA BUMALDA 'GOLD FLAME'	GOLD FLAME SPIREA		#2 CONT.	18" HT, 3' SP.			
8	НН	HEMEROCALLIS 'HAPPY RETURNS'	HAPPY RETURNS DAYLILY		#2 CONT.	2' O.C.			
3	JB	JUNIPERS CONFERTA 'BLUE PACIFIC'	BLUE PACIFIC JUNIPER (OR ANDORRA)		#2 CONT.	18" SP, 4' O.C.			
3	SR	SYRINGA RETICULATA	IVORY PILLAR JAPANESE TREE LILAC	2"	B&B	20'-25' HT			
3	GB	GINKGO BILOBA	AUTUMN GOLD GINKO	2"	B&B	30'-50' HT			
3	GT	GLEDITSIA TRIACANTHOS	THORNLESS HONEYLOCUST	2"	B&B	35' HT			

1. CONTRACTOR TO REFER TO DETAIL FOR LIGHT POLE BASES.

2. CONTRACTOR TO REFER TO ELECTRICAL PLANS FOR CONDUIT ROUTING OF LIGHT POLES AND PYLON SIGN AND FOR LIGHTING AND WIRING SCHEDULE.

3. LIGHT POLES PLACED BEHIND CURBING SHALL BE A MINIMUM OF 2' FROM THE FACE OF CURB TO THE OUTER EDGE OF THE LIGHT POLE BASE.

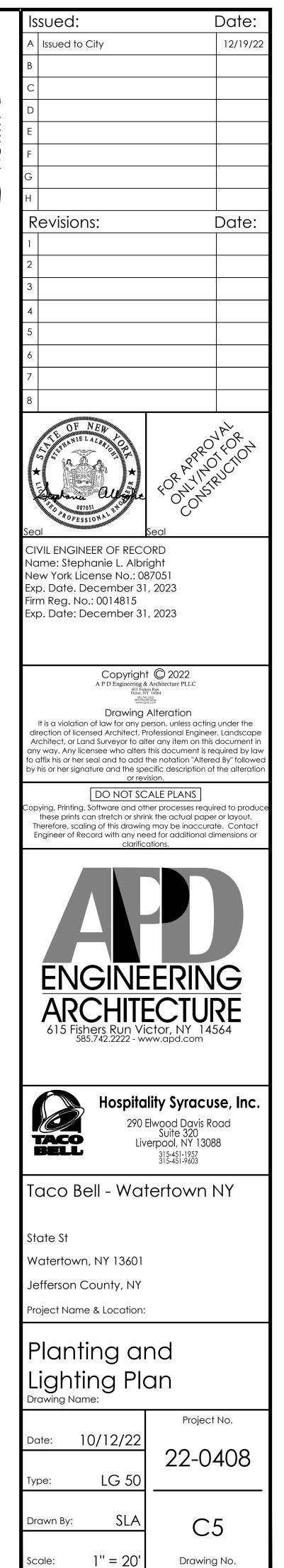
4. FIXTURES SHALL BE BRONZE. LIGHT POLES SHALL BE LSI INDUSTRIES 4SQB3-S11B-22-BRZ.

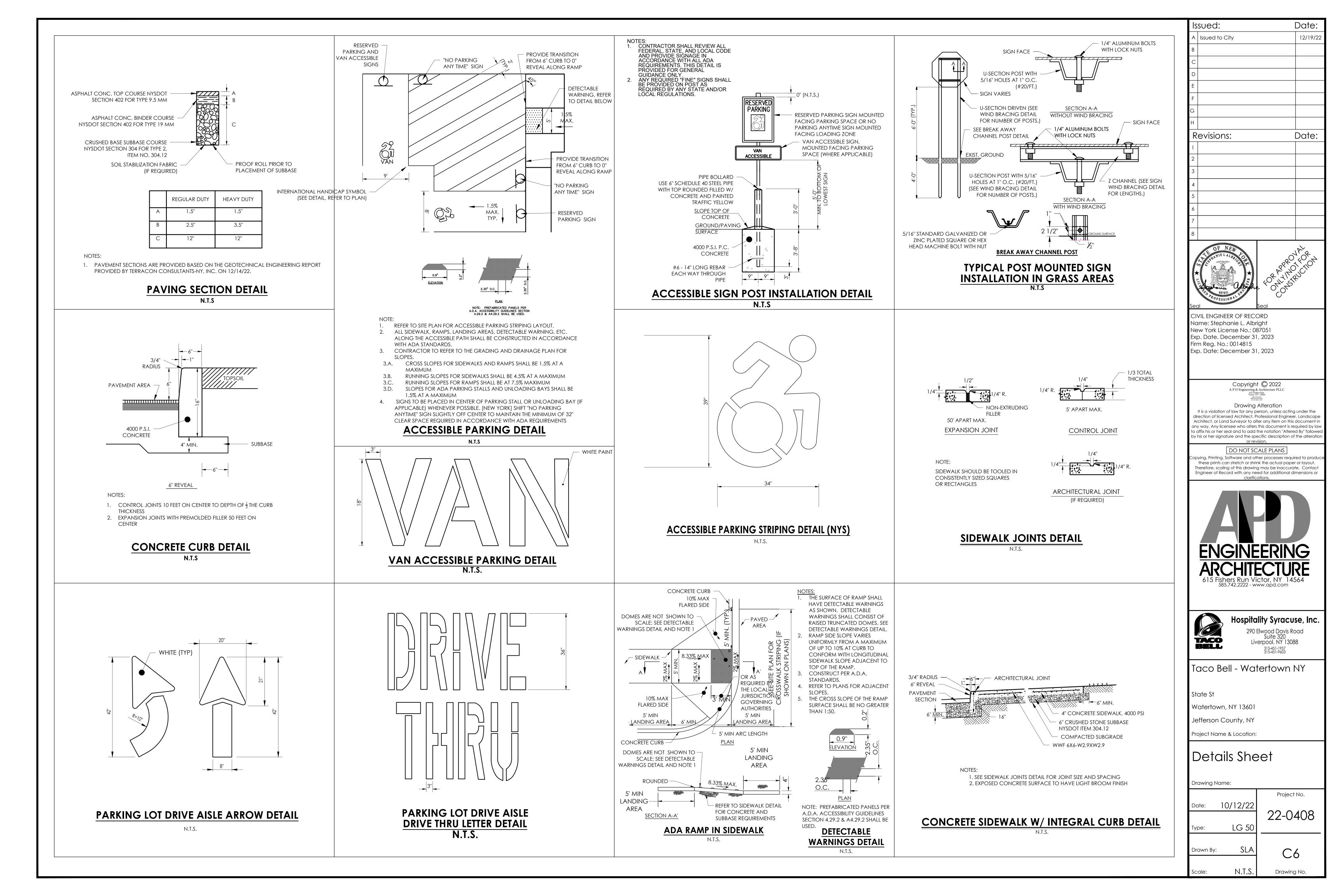
Calculation Summary								
Label	CalcType	Units	Avg	Мах	Min	Avg/Min	Max	
CALCULATION POINTS @ GRADE	Illuminance	Fc	2.44	16.1	0.0	N.A.	N.A	
PARKING & DRIVING SUMMARY	Illuminance	Fc	5.17	16.0	1.0	5.17	16.	

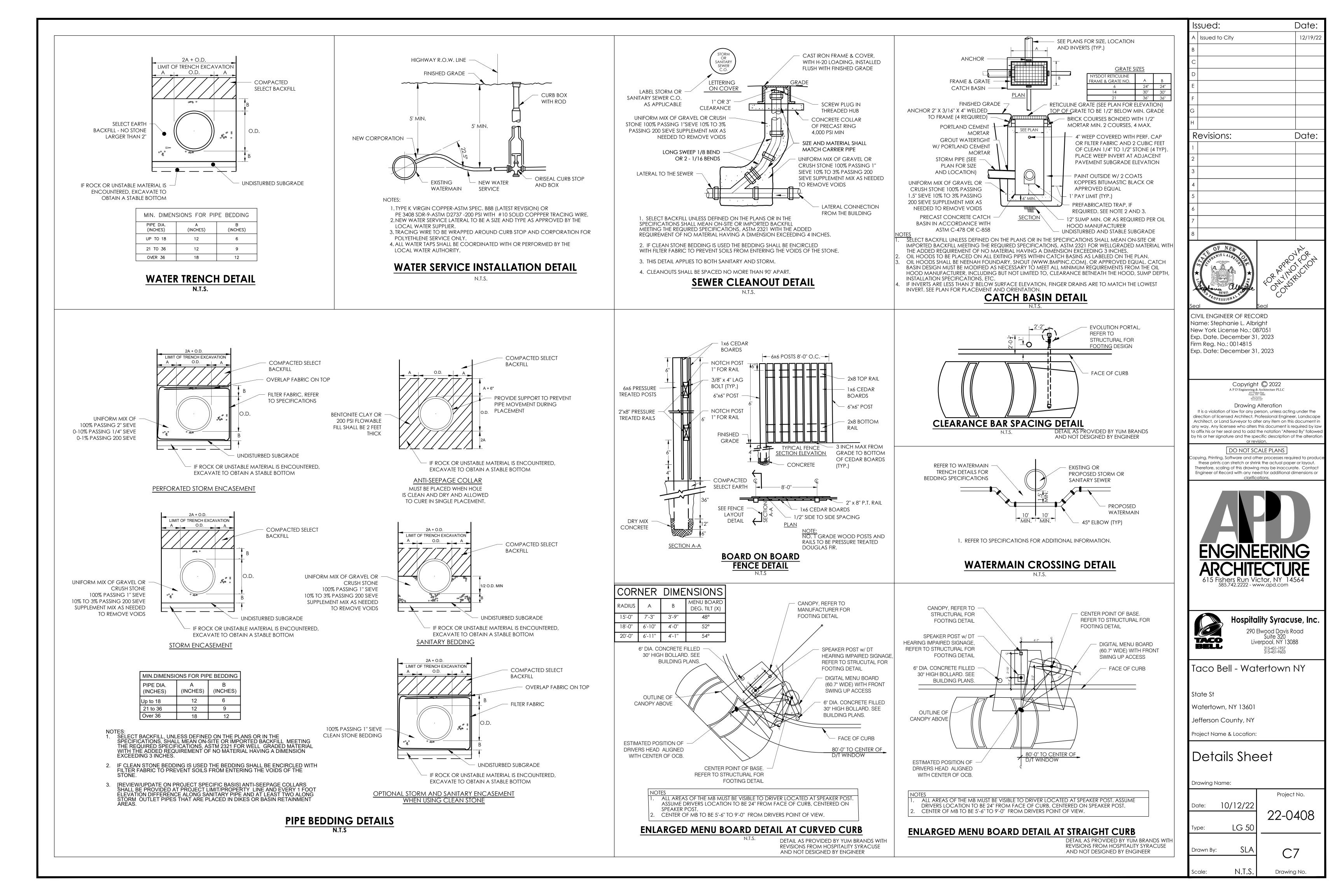
Luminaire Schedule								
Symbol Qty	Label	Arrangement	Description	LLD	LDD	LLF	Arr. Lum. Lumens	Arr. Watts
2	A	3 @ 90°	MRM-LED-24L-SIL-FT-50-70CRI-T90-25' MH	1.000	1.000	1.000	77892	528
1	В	2 @ 90°	MRM-LED-24L-SIL-FT-50-70CRI-D90-25' MH	1.000	1.000	1.000	51928	352
<b>—</b> 2	D	SINGLE	MRM-LED-24L-SIL-FT-50-70CRI-IL-SINGLE-25' MH	1.000	1.000	1.000	16436	176

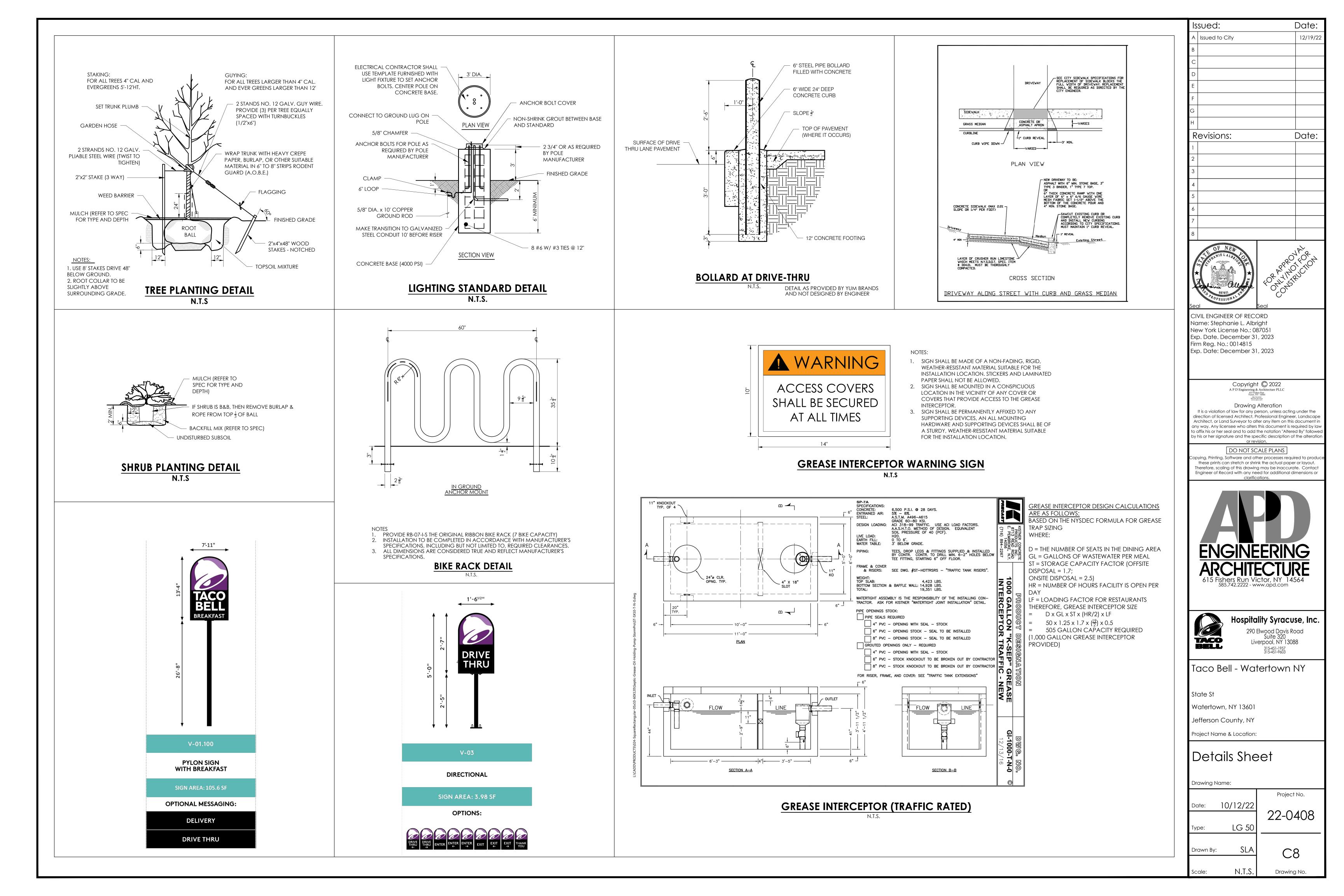
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	UE/T		UNDE	RGROUNI	D ELECTRIC &	& TELEPHONE	
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THE SPECIFICATIONS ARE NOT PROVIDED AS AN INDICATION OF WORK, BUT PROVIDE REQUIREMENTS AND STANDARDS OF WORK REQUIRED, OR COULD BECOME REQUIRE

#### SENERAL CONSTRUCTION CONDITIONS

- The term of Owner as used in these specifications and notes shall include the owner of the property, the company or party that hired the Contractor, the company or party that signed the contract for this work, and the agents of each. The Owner's representative shall be the individual or party assigned by the Owner to be the Owner's representative. Owners of adjacent properties shall include the property owner, lessee, legal occupier, and operator of any business on that property.
- All work and materials shall comply with all local, state, and federal regulations, codes, and O.S.H.A. standards and be constructed to meet or exceed those codes
- The Contractor shall be responsible for all temporary permits, connection permits, fees, inspections and record keeping required by all municipal, utility, health, environmental, state, or federal agencies that may have jurisdiction. Furthermore, the Contractor shall be responsible to meet or exceed all requirements of the agencies or authorities having jurisdiction over his work. All conflicts in requirements of different agencies, authorities, and/or the design shall be brought to the attention of the owner's representative before proceeding.
- The Contractor shall be responsible to locate and maintain the property and project limits throughout the project. All conflicts between the design and the project/property limits shall be brought to the attention of the owner's representative before proceeding. Unless described in the contract documents or shown on the drawings the Owner has not secured any right of ways, easements or agreements with other property owners or property users. Therefore, it shall be the Contractor's responsibility to secure and maintain any temporary right of ways, easements, permits, or agreements he may need to perform his work. All such agreements shall hold the Owner, Engineer of Record, and his agents harmless and the responsibility of the Contractor to bear all costs. The Contractor shall copy the Owner on releases of all agreements prior to final payment by the Owner to the Contractor. The Contractor shall not interfere with operations of adjacent businesses and work shall be completed off-hours, as necessary. Coordinate with Municipality for any restrictions on allowable working hours.
- Unless otherwise noted on the drawings or in the contract documents the Contractor shall be responsible for all construction survey, layout, and record drawings for this contract. Any conflicts in survey/layout and the design or agencies requirements shall be brought to the attention of the owner's representative prior to proceeding with the work. The Contractor shall protect and safeguard all existing survey corners, monuments, control and tie-downs. The Contractor shall pay all costs to repair or replace damaged survey monuments, control and tie-downs. Record drawings shall be provided in accordance with any requirements of the authorities having jurisdiction including the required information to be provided, and signatures, seals, and certifications that may be required.
- No changes to the design or materials specified may be made without written authorization by the Engineer of Record or in the case of utilities or road work to be dedicated, the authority receiving dedication. At the end of the contract, the Contractor shall provide to the Owner a record set of drawings reflecting all changes made by the Contractor during construction.
- Erosion control is necessary whenever sediment, dust, erosion, or contaminated run-off may occur. The Contractor shall be responsible to place and maintain whatever erosion control or run-off protection is required to protect his work, the work of others, the project, adjacent properties and the health and well being of the workers, public and surrounding natural resources. This shall include additional measures beyond the project ES plans, as necessary. They shall be familiar with all federal, state and local requirements regarding erosion and run-off control.
- The Contractor shall be familiar with the project site and all adjacent pedestrian, traffic, and business uses. The Contractor shall take whatever precautions and steps necessary to maintain safety and operation of these uses in accordance with federal, state, county, and local requirements. The Contractor shall be responsible for costs and damages caused from his failure to take proper and adequate precautions. The Contractor shall be familiar with all federal, state, and local requirements regarding these uses.
- The Contractor shall be responsible for costs and delays associated with weather, groundwater, and other occurrences that could be expected or are common with this type of work. The Contractor shall review all pertinent documents including soils reports, soils barings, and other soil or site data.
- The Contractor shall be responsible to save and protect his work throughout the contract. Any damages requiring repairs or replacement shall be corrected by the Contractor at his expense.
- When work is done within a road, utility or private easement, right of way, or other property agreement, the Contractor shall do all work within that area per the authority having jurisdiction.
- When separate site and building contracts are performed, the site Contractor shall be responsible to bring utilities to within 5 feet of building face unless noted otherwise on drawings or contract documents.
- All utilities are shown per surface surveys and/or record maps and may vary from actual in-field locations. The Contractor is responsible for all utility stake outs and locating utilities prior to commencing work. Any damage to utilities due to improper stake out, lack of stake out or the failure to verify differences between drawings and actual field conditions will be the responsibility of the Contractor to repair, replace, or pay damages at no expense to the contract.
- Contractor shall comply to the fullest extent with the latest standards of OSHA directives or any other agency having jurisdiction for excavation and trenching procedures. The Contractor shall use support systems, sloping, benching, and other means of protection. This includes, but is not limited to, access and egress from all excavation and trenching. Contractor is responsible to comply with performance criteria for OSHA. Trench excavation requiring sheeting, shoring or other stabilizing devices shall be designed by a Professional Engineer and meet all O.S.H.A. requirements. All excavations shall maintain safe side slopes in accordance with local, state and O. S. H. A. requirements. No stocking of material close to an open cut or steep slope will be permitted in an effort to prevent cave-ins.
- The contractor shall select the means and methods for providing support of excavations in accordance with safety requirements, plans, and project specifications. The contractor must evaluate soil conditions during excavations since variations in the soil can occur across the site. The excavations should be monitored continuously for signs of deterioration such as seepage of water or sloughing of soil into the excavation. The contractor is ultimately responsible for excavation safety.
- The Contractor shall notify the Owner immediately and stop all work in areas where hazardous materials are discovered. When required, the Contractor shall notify the appropriate environmental and health agencies.
- The Contractor shall coordinate with the Authority having jurisdiction for all required inspections and be responsible to hire any required third party inspectors.
- For any testing, inspections, and/or certifications requiring a Professional Engineer, the Contractor shall be responsible to hire a third party engineer. A copy of all tests shall be provided to the Engineer of Record.
- Any discrepancies between plans, details, and specifications shall be immediately brought to the attention of the Engineer of Record.
- Stabilizing fabric (woven geotextiles), if required, shall meet the following requirements "modulus (load at 10% elongation) =115lb per ASTM D1682-64", "Grab tensile strength 2001b per ASTM D 1682-64", "mullen burst strength = 400psi per ASTM D 3786-87", "trapezoid tear strength when applicable = 1151b per ASTM D1117-80", "coefficient of permedbility K CM/SEC = .015 per ASTM D 4491-85", "water flow rate CPM/SF= 60 per ASTM D 4491-85". When stabilization fabric is used it shall be pulled tight and all wrinkles removed. Overlaps shall be in accordance with manufacturer's recommendations. Refer to Geotechnical Engineers report, if available, for additional information.
- Filter fabric (non-woven geotextile), if required, shall meet the following requirements "grab tensile elongation =50% per ASTM D1682-64", "Grab tensile strength 70lb per ASTM D 1682-64", "mullen burst strength = 200psi per ASTM D 3786-87", "trapezoid tear strength when applicable = 35lb per ASTM D1117- 80", "coefficient of permedbility K OM/SEC = .2 per ASTM D 4491-85", "water flow rate GPM/SF= 180 per ASTM D 4491-85". When filter fabric is used it shall be pulled tight and all wrinkles removed. Overlaps shall be in accordance with manufacturer's recommendations.

#### DEMOLITION

- The Contractor shall inspect all structures, facilities and areas slated for demolition to gain a full understanding of the work required. The Contractor shall take whatever measures necessary to protect the safety of the public, his employees and agents during the inspections and subsequent work. The Owner, Client, and Engineer of Record are not responsible for the condition of the buildings, facilities, or other areas slated for demolition.
- All materials not slated for reuse must be disposed of off site in a legal manner. The Contractor may salvage any equipment or materials not designated by the Owner to be saved. All salvaged material or items shall be removed from the site immediately upon removal. No such materials shall be stored on the site. Absolutely no sales of salvaged materials will be allowed on the project site. All salvaged material must be removed, transported, and disposed of in a lead manner.
- Upon approval by Owner, the Contractor shall be responsible to remove and store safely all materials slated to be saved or reused. The Contractor shall document existing conditions using photographs prior to start of work and notify Owner of any existing damage prior to construction start. The Contractor shall be responsible for all costs to repair or replace existing features to remain (including but not limited to fencing, lighting, curbing, pavement, utilities, storm structures, landscaping, etc.) that are damaged due to his work or failure to protect throughout the duration of his contract.
- No burning, explosives, or other potentially dangerous methods of demotion will be allowed unless written permission is granted by the Owner and all appropriate permits are granted.
- The Contractor will provide whatever safety equipment and devices are necessary to protect the adjacent properties, structures and other areas slated to remain. This will also include erosion control, dust control, and settlement.
- All areas shall be brought back to their original grade or that of the surrounding area, which ever is closer to the final grades of the project for that area. All areas requiring fill shall be compacted to the requirements of the area but in no case less than 90% of modified proctor (ASTM D 1557).
- All demolition within the proposed building footprint shall be coordinated with the building drawings.
- Light pole removal shall include complete removal, backfill of concrete base, and capping of any conduit/wiring in to be abandoned in place. CLEAR AND GRUE

- Clearing and grubbing shall not commence until erosion control plans, including applicable BMP's, are in place.
- The Contractor shall review plans and identify and safely mark all plants and trees to be saved. The Contractor shall protect all plants and trees to be saved throughout the contract. This shall include prohibiting any work within the drip line of the tree, except under the supervision of a licensed Landscape Architect.
- All areas to be cleared and grubbed shall be surveyed in the field to establish the appropriate limits of work.
- The Contractor shall take whatever measures necessary to locate and protect existing utilities, structures, wetlands, and other facilities to remain. All trees, shrubs, stumps, roots, and other debris shall be removed from site and disposed of in a legal manner
- No burning will be allowed on site.

#### PAVEMENT AND STRUCTURAL SUBBASE

- The type of subbase required for each use shall be called out on the drawings. If no reference is made on the drawings or details to the type of subbase required the following shall be used:
- The source of the material shall be one approved for use by the State Department of Transportation.
- The material shall be a crushed stone conforming to AASHTO M 147-65 (1980 or latest revision), grade A 1.b. 1.c. Gravel or other materials can only be substituted for crushed stone when approved in writing by the Owner and Engineer of Record.
- 1.d. Material supplied for use as subbase shall have 100% passing the 2 inch sieve, 30% to 65% passing the 3/8 inch sieve, 25% to 55% passing the No. 4 sieve, 15% to 40% passing the No. 40 sieve and 2% to 10% passing the No. 200 sieve.
- Subbase shall be placed in lifts not to exceed 8 inches and compacted to the requirements stated in the soils report. If not stated, the compaction requirement shall be 95% of maximum dry density per ASTM D1557 (modified proctor).
- The Contractor will be responsible for all costs in preparing the subgrade to receive subbase. This shall include fine grading and compacting as necessary to meet the requirements stated here and under Earthwork.
- The amount of testing required to verify the compaction shall be the same as stated under Earthwork.
- Refer to General Construction Conditions for filter fabric requirements, if applicable.

#### EARTHWORK

- Earthwork shall not commence until erosion control plans, including applicable ( Refer to Project Geotechnical Report for full project recommendations. Where C following may be used.
- Prior to starting any cuts or fills the Contractor shall strip and stockpile all to operations are complete and all erosion control devices are in place in that are approved by the owner's representative. The Contractor shall review the soils re to be familiar with the depth of topsoil. The Contractor shall take all reasonable
- Unless otherwise noted, the grades shown on the plans are finished grades. The
- subtracted to calculate subgrade elevations. The Contractor shall maintain a survey grid of not less than 100' x 100' or oth
- location and amount of cut or fills remaining. At subgrade this grid shall be 50 completed demonstrating that the subgrade is +/- 0.1 feet of required subgrade Unless otherwise noted on the drawings or in the contract documents, the Con
- performed by an independent testing laboratory. For each lift placed, compactic testing shall be done every other lift with at least 1 test every 50 LF.
- Structural fill placed 2 feet or deeper below the finished subgrade elevation or inches. Structural fill placed within the upper 2 feet of proposed subgrade or
- 8. Compaction requirements shall be those outlined in the soils report, if provided following will be used:
- 8.a. Under and to 20 feet outside the building envelope the soils shall be o (modified proctor).
- Under proposed or future pavement areas, including 10 feet outside suc 8.b. dry density per ASTM D 1557 (modified proctor).
- All landscape and lawn areas shall be compacted to 90% maximum dry 8c. 8.d. The testing lab shall test soils in accordance with ASTM D 2922 (nuclear
- 8.e. Constructed berms shall be compacted to 95% maximum dry density p All material to be used for fill shall be free of organics, frozen material, conto
- placement within 1 foot of subgrade, no rock shall be greater than 2 inches in segregating, or required methods to treat soils to meet compaction and other
- All fill placed within berms that detain/retain water shall be a minimum of 20 maximum particle size of 6 inches. Any on-site cut areas could be utilized as specifications were met (placement, compaction, gradation, permeability, etc.). as well as placing of embankment material on a frozen surface is prohibited. E placed in layers not thicker than 4 inches before compaction with particle size required density of fill material for berm. Anti-seep collars are required for all
- The Contractor shall take all necessary precautions to protect earthwork opera divert drainage, dewatering, and sealing disturbed areas with a steel drum roller
- 12. If imported material is required, the source and a random composite sample s The testing laboratory shall test for percent passing the 200 sieve that does n They shall also verify consistency with existing on site materials and all other Engineer of Record and the Geotechnical Engineer that prepared the soils report
- 13. The testing lab may restrict some on site materials from being used as fill in not meet requirements stated here. If such conditions do exist and other mate writing the use of import material unless there will be no additional cost to the
- 14. Fills shall be placed in lifts not to exceed 8 inches in mass fills and 6 inches proofrolled using a smooth drum roller with a minimum static drum weight of one direction, followed by 2 overlapping passes in a direction perpendicular to stabilized with repeated compactive effort shall be overexcavated to a suitable backfilling is complete the resulting subgrade surface is firm and stable under subgrade elevation to replace the removed unsuitable material. If imported str undercuts within the building or pavement areas, a woven geotextile should be
- Contractor is to remove any debris or surficial organic soils (ie. topsoil, organic 15. building footprint, floor slabs, and pavement areas prior to the placement of ar
- 16. All final subgrade under proposed pavement, building, or other structure shall t found to be unacceptable shall be scarified, dried, and re- compacted. Retest
- 17. All fill material is to be in place and compacted prior to installation of propos width will only be allowed when compaction equipment limitations require and a open in one day than can be properly backfilled in that same day to minimize uniform support at invert and proper compaction under, along, and over the pip to the pipes including: placing backfill/bedding by hand, using hand operated p fills are a minimum of 2 feet or manufactures recommend depth, which ever i relieved in these areas and will remain as stated on the drawings or above. If a placed between the natural soils and backfill and the stone to prevent miaration details. The Contractor is cautioned against the migration of fines from soils or wrap those areas with filter fabric to prevents files from migrating into w
- 18. If rock is encountered that was not indicated on the plans or soils report, the representative prior to rock removal. Rock will be defined as the natural earth i equipment.
- 19. Where rock is adjacent to a structure or utility, the rock shall be removed to than 1 foot or greater than 3 feet on any side
- 20. No explosives will be allowed until all permits are granted and the Owner has s structures within the area of the blast must receive a pre-blast survey. All bl
- 21. Unless otherwise noted on the drawings, the Contractor shall remove all excess

#### TRAFFIC SIGNAGE AND PAVEMENT MARKINGS

lead manner.

- Pavement markings shall be the type, color, size, and locations shown on the pavement markings. If the information on the plans and details is not complete regarding this, use the following:
- 1.a. Paint shall be supplied in accordance with AASHTO: M 248 latest addition 1.b. Colors shall be as follows:
- 1.b.1. YELLOW- parking stalls, parking islands, and fire lanes 1.b.2. WHITE - stop bars and lettering, pedestrian crossings, handicap characters
- 1.b.3. BLUE - background of handicap parking symbol
- The pavement shall be clean and free of dirt, dust, moisture, oils, and other f unless paints are compatible and overlay identically. The surface of the pavement manufacturer's recommendations are greater. All painting shall be applied in app and in accordance with manufacturer's recommendations.
- The signage and pavement markings shall be the type and at the general locat shall be provided and located in accordance with the Local Highway, County Hig state codes do not exist use MUTCD.
- Posts, brackets, and frames shall be steel per ASTM A-36, A-242, A-441, A-ASTM A123. All cutting, drilling, or other pole modifications shall be painted with
- Post holes in pavement shall be a minimum of four feet deep and 12 inches in depth. Sign posts shall be kept plumb, 6 inches off bottom and centered as 44 post system should be able to withstand 33 pounds per square foot.
- 6. Contractor can place signs on posts after concrete has cured for seven days
- All handicap striping and signage, including spaces, crosswalk, accessible path, requirements. Fire lane striping and signage shall meet the requirements of the
- SITE CONCRETE INCLUDING CURB, SIDEWALKS AN
- The dimensions shall be those shown on the drawings. The Concrete mix shall ASTM C 150 and aggregates meeting State Department of Transportation requir inch +/-1/2 inch and for formed concrete the slump shall be 3 inch +/-1C 260 4% +/- 1 1/2% for slip form work and 6% +/- 1 1/2% for formed and C 494, type A. Ouring compounds shall conform with ASTM C309, type I, class at 200 sq ft per gallon.
- Sidewalks, gutters and curbs shall be placed on compacted subbase consistent subbase details are missing and no agency has jurisdiction use the following: of compacted subbase and curbs shall be placed on a minimum of 4 inches of
- 3. All forming, placement, materials and curing shall conform to the latest addition
- and all similar State Department of Transportation requirements.
- practices". Reinforcing steel shall be ASTM A 615, grade 60, deformed. Welded wire fabric shall be ASTM A 185, welded wire steel fabric.
- Sidewalks, and gutters shall have a broam finish perpendicular to flow with a picture frame edge joint all the way around. Curbs shall have a smooth finish or light rub finish but consistent throughout the project.
- 6. Expansion joints shall be placed as per details and at adjoining structures such as walls, manholes and vaults. Expansion joint material shall be premolded, 1/2 inch material with 23/64 inch cap in accordance with ASTM D1751. After concrete has set the cap should be removed and void filled with waterproof joint filler. Ourb and gutter shall be cut or tool jointed to 1/3 the depth every 10 feet. Sidewalks should have tooled or cut joints to 1/3 the depth in squares or as close to square as possible not exceeding 5ft x5ft.

		PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION. WHEN THESE SPE ENGINEER AND AUTHORITY HAVING JURISDICTION.	CIFICA	ations <i>f</i>
	WA	TER SYSTEMS AND SERVICES	<u>SE</u>	EDING
le BMP's, are in place. e Geotechnical Report is not clear or does not give requirements, the	1. 2.	The water systems and services shall be supplied and placed in accordance with all local, state and federal requirements. When the requirements of the authority having jurisdiction differ from those shown on this plan, Contractor shall adhere to the more stringent standards. Refer to Pipe Bedding Detail for pipe bedding requirements.	1.	Topsoil Iawn are import o
I topsoil. Stripping of topsoil can only commence after the clear and grub area. Topsoil shall be stockpiled in areas designated on the plans or s reports, baring logs, and, when necessary, his own field verification so as nable precautions to prevent over and under removal.	3.	All water piping, fittings and appurtenances shall be placed a minimum of 6 inches below frostline or with a minimum 5 feet of cover in lawn areas and 6 feet of cover in paved areas, whichever is greater. Pipe sizes 4 inches and up shall be ductile iron or polyvinyl chloride as indicated on the drawings (if not shown use ductile iron). Pipe sizes below 4 inches shall be copper or polyethylene as indicated on the drawings (if not shown use copper). All piping shall be extended vertically through building floor slab. Provide radius/bends in accordance with manufacturer's recommendations. Refer to building plans for additional information.	2	Topsoil larger til approve Contract deposits
Therefore, pavement, floors, subbase, and other improvements must be other means acceptable to the Owner's representative that will indicate $e 50' \times 50'$ with location and final grade marked clearly or survey shall be ograde.	4.	The minimum separation between water services and sewer lines shall be 18 inches measured vertically from outside to outside of pipe at the crossing. A standard length of water pipe shall be centered at the crossing to maximize the distance between the crossing and the nearest water service pipe joint. The sanitary line shall be ductile iron pipe with mechanical joints at least 10 feet on both sides of crossing, the waterline shall have mechanical joints with appropriate thrust blocking as required to provide a minimum of 18" dearance meeting requirements of ANSI A21.10 or ANSI 21.11 (AWWA C-151) (Class 50). Contractor shall adjust elevation of water as needed to maintain adequate separation and buricil depth. When the water service runs under the sewer line, a gravel or crushed stone	3. 4. 5.	Mow all Loosen sod, rub Preparat
Contractor shall retain and pay all cost for soil compaction testing to be iction testing shall be done every 2000 sq. ft. In trenches, compaction		backfill meeting the requirements of subbase shall be placed and compacted around the water pipe up half the diameter of the sewer pipe to provide adequate support to the sewer line. Ductile iron pipe shall be provided in accordance with AWWA C151, (6 inch diameter and greater shall be Class 50 and 6 inches and smaller shall be Class 51). Ductile iron pipe shall be lined with a cement mortar and sed coated in accordance with AWWA C104. Caskets shall be provided in smaller shall be Class 51). Ductile iron pipe shall be lined with a cement mortar and sed coated in accordance with AWWA C104. Caskets shall be provided in	6.	stripping specified Clean d
or finished grade of graded areas shall have a maximum particle size of 6 or finished grade of graded areas shall have a maximum particle size of $3$	5.	accordance with AWWA C111. Fittings shall be ductile iron in accordance with AWWA C153 compact fittings with a pressure rating of 350 psi. Water services and sewer lines running parallel shall have a minimum separation of 10 feet measured from outside of pipe to outside of pipe. At crossings, one full length of water pipe shall be located so both joints will be as far from the sewer as possible. Special structural support for the water and sewer pipes may be required. All pipes shall be installed per manufacturer's recommendations. Ten gauge copper tracer wire shall be placed with all plastic pipe. Pipe material shall be as follows:	7. 8	diamete Roll, rak Lime: n
ded. If the soils report is not clear or does not give requirements, the e compacted to a minimum of 95% maximum dry density per ASTM D 1557	5.0	PVC (Polyvinyl Chloride) pipe shall be furnished in accordance with AWWA C900 for pipe 4 inches or greater and ASTM D 1785, schedule 40, gaskets per ASTM F 477- elastomeric seal, solvent cement per ASTM D 2564 for pipes smaller than 4 inches.	9.	passes The top sieve, 8
such areas, the soil shall be compacted to a minimum of 93% maximum	5.b 5.c	pipes.	10.	Lawn fe at a rai
dry density per ASTM D 1557 (modified proctor). uclear method) with proctors for each soil type. y per ASTM D1557.		in accordance with AWWA C111. Fittings shall be ductile iron in accordance with AWWA C153 compact fittings with a pressure rating of 350 psi. Standard ductile iron or cast iron fittings shall be supplied in accordance with AWWA C110 with a pressure rating of 250 psi. The lining and gaskets for the fitting shall meet the same requirements as the pipe. If recommended in the soils report, ductile iron pipes shall be encased in polyethylene in accordance with AWWA C105 and tar coat all fitting bolts whenever soils are primarily day or not pH balanced.	11. 12.	Lawn se Pennlaw seeded Steep s
ntaminated material, debris, and any rocks larger than 4 inches. For fill s in diameter. The Contractor shall bear all cost associated with drying, her requirements.	5.d 6.	Copper water pipe shall be supplied in accordance with ASTM B 88- type K, seamless with fittings per AWWA C800. Gate Valves shall be nonrising stem, double disc, bronze disc Resilient seated, cast iron or ductile iron body and bonnet in accordance with AWWA C509 and	13.	Creeping Hydrose acre, w
20 percent by weight of material passing the No. 200 sieve, and a as fill material for the berm, as long as all construction requirements and .). Inclusion of vegetation, organic material, or frozen soil in the embankment, d. Bedding material for all pipes and conduits within berm area shall be	7.	pressure rated for 250 psi. Ten gauge copper tracer wire will be placed with all pipes. Valve box shall be cast iron with a base compatible with valve, 5 inches in diameter, screw type extension at top and a cover that reads "WATER".		additive mixture underlyir
size limited to 3 inches in the greatest dimension, and compacted to all pipes/utilities within the berm area.	8. 9.	All tap and/or connection material and work shall be done in accordance with and coordinated with the local Water Authority and Health Department. When the Authority so requires, the taps and/or connections shall be done by the Authority themselves and paid for by the Contractor. Thrust restraints shall be used at all fittings, plugs and appurtenances that cause a change in direction, flow or are subject to thrust or hammering by water flow.	14.	The Cor uniform minimur inch hei
oller prior to indement weather. e shall be reviewed by the testing laboratory prior to being brought to site. is not exceed the existing on site material or in no case greater than 10%	10.	Thrust restraints will include concrete thrust blocks (3000 psi), anchoring joints and tie rods. Concrete thrust blocks shall be used unless space, access or maintenance restraints prohibit their use. Curb stops shall have a bronze body, ground key plug or ball with wide tee head. The curb stop shall be compatible with adjoining pipes. The service box shall	15.	contract mowing
in building or pavement areas when it is their opinion that the material will	11.	have a telescoping top section with a length that will place the adjustment centered when buried to the appropriate depth. The service box shall be of a size and type that is compatible with the curb stop. The cover shall have the lettering "WATER". All meters, vaults and backflow shall meet the requirements of the health department and other agencies having jurisdiction.	15.	Where s Remove Planting
naterial is not available on site, the owner's representative must authorize in the contract. les in trench or restricted areas. All subgrades shall be thoroughly	12.	Fire hydrants shall conform to the requirements of the local water authority, fire department and AWWA C502. Drain stone shall have 100% passing the 1 1/2 inch sieve, 90 – 100% passing the 1 inch sieve, 35 – 95% passing the 1/2 inch sieve and 0 – 15% passing the 3/8 inch sieve. All hydrants will include a gate valve and box located at the hydrant branch to shut off the hydrant line.	17.	free of Plants s drainage
of 20 tons, operated in static mode. A minimum of 2 overlapping passes in to the first 2 passes. Areas which are unsuitable and which cannot be ble subgrade. The undercut should be of adequate depth such that, after	13. 14.	All bedding and encasements shall be compacted with care to achieve proper compaction without damaging the pipe, fittings, or appurtenances. If dean stone is required by the local authority having jusidiction and is approved by Owner and/or Engineer of Record, then the bedding material shall be wrapped	18.	properly Two lay
ler proofrolling. Onsite structural fill may be used to attain proposed structural fill, base, or subbase course materials are used to backfill the be placed at the bottom of the undercut area prior to placement of the fill.	15.	in filter fabric and anti-seep collars shall be provided to prevent the migration of fines. All water mains fittings and valves shall be tested for pressure and leakage in accordance with AWWA 0600. Test water shall be potable. Test pressures shall not	19. 20.	All trees Mulch a areas w
anic subsoil, reworked soil) which may be encountered within the proposed f any fill.		be less than 1.25 times the working pressure at the highest point and 1.5 times the working pressure at the testing point. The pressure may not drop more than 5 psi during the 2 hour test. Leakage will not exceed more than (L=SD(P)1/2 /133,200) where "L = allowable leakage, in gallons per hour" "S= length of pipe tested, in feet" " D= nominal diameter of pipe, in inches" "P= average test pressure during test, in pounds per square inch (gauge) during the same 2 hour duration.	21.	uniform All land: replacen
Il be proof rolled as described above for the identifying of soft areas. Areas est by proof roll as necessary. Dosed utilities. Refer to pipe bedding details for trench dimensions. Additional	16. 17.	Other fitting and appurtenances not part of the main line testing shall be tested by visual inspection for leakage under normal working pressures. All main lines and appropriate appurtenances shall be flushed and disinfected in accordance with AWWA C651 and the requirements of the appropriate health		shall inc insectici germina
d only after approval of the Engineer of Record. No more trench shall be ize weather and safety concerns. When backfilling around pipes, provide e pipe. Care shall be given while backfilling around pipes to prevent damage d plate tamps or jumping jacks, and other load restrictive techniques until	18. 19.	department. The Contractor will coordinate all testing and disinfecting with the water authority and health department. Any testing failure shall require the Contractor to repair or replace the failed section at no additional expense to the contract.	22.	and rem per wee Following Antidesi
r is greater, above the top of the pipe. Compaction requirements are not If clean stone is used as a bedding or encasement, filter fabric shall be ation of fines. Anti-seep collars shall be provided in accordance with the s adjacent to voids. Where such conditions exist the Contractor shall install		DRM WATER SYSTEM	22.	accorda
voids.	<u>01 (</u> 1.	The storm water system shall be supplied and placed in accordance with all local, state and federal requirements.		
the area for removal should be measured and reviewed with the owner's th materials that can not be removed with conventional earth working	2 3.	Storm design includes many variables, such as pipe roughness coefficient, that can affect the actual final run—off. If no alternative materials are listed on the utility drawings, no substitutions may be made by the Contractor unless first reviewed and accepted by the Engineer of Record. Refer to Pipe Bedding Detail for pipe bedding and anti—seep collar requirements.		
to a minimum of 6 inches below and 1 times the diameter, but not less	4.	Storm pipe material shall be as follows:	<u>S</u> A	NITAR
is signed off. Pre and post blast reports must be kept and recorded. All Il blasting must be performed by a licensed blaster. xess topsoil, cut material, or waste material from site and dispose of in a	4. 4. 5.	with a manning friction number (n) of 0.013 or less. Install in accordance with ASTM F449 and the manufacturer's recommendations.	1. 2. 3.	The san Refer to Unless o elastom
	6.	Increase size of manhole if in the same horizontal plane there is two areas where the area between two pipes is less than 8 inches or ½ of the circumference is supported by less than ½ of the diameter of the manhole. Inverts shall be smooth cast in place concrete. Caskets between risers shall be rubber per ASTM C 443. Adjustment rings shall be precast concrete 4000 psi and 5 to 8% air entrainment.	4.	Forcema 3139, a passing
ne plans. Contractor shall provide two (2) coats of paint for all Nete and the authority having jurisdiction does not have requirements	7. 8.	Inlets shall meet the same requirements as those listed for manholes, except sumps shall be provided as per details, rather than a smooth invert. Grates shall be galvanized per ASTM A123. Minimum grate opening size will be 24 inches x 24 inches and design for a minimum of H-20 loading. Refer to details for additional information.	5.	water, s All pipe given du
klition.	9.	Dry wells shall meet the same requirements as those listed for manholes with the addition of openings of approximately 15% of the rings interior surface. The openings shall be 1 x 3 inch slots or 1 inch diameter on the inside surface. Dry wells shall be backfilled with a minimum of 1 foot of clean stone sized between 3 and 4 inches. Outside the stone, the entire structure shall be wrapped in filter fabric to prevent outside soils from entering the stone and dry	6. 7.	All syste All taps the tap
cap parking symbol and characters, and traffic control lettering and	10.	well. Unless otherwise noted, underdrains and trench drains shall be made with 4 inch perforated corrugated polyethylene pipe encased in clean stone sized between 2 inch and ¼ inch and then wrapped in filter fabric. Outside dimensions of the trench drain will not be less than 1 foot.		manhole be core invert sl
r foreign materials. Any old pavement markings shall be removed ement prior to application shall be 45 degrees F and rising unless appropriate weather conditions (e.g. temperature, wind, precipitation),	11. 12.	All storm pipe entering structures shall be grouted to ensure connection at structure is watertight and structurally sound. All storm sewer pipes entering and exiting structures shall be flush with the inside of the structure wall. All pipe shall be placed in accordance with the manufacturer's recommendation and to the lines and grades shown on the drawings. Care shall be given	8. 9.	Sanitary has bee Gravity :
ocation shown on the drawings. The signage and pavement markings	13.	All pipe shall be proceed in accordance with the maintacture's recommendator and to the links and grades shown of the addinings. One shall be given during backfill operations not to move or damage pipe or appurtenances while achieving the appropriate compaction requirements. All systems shall be visually inspected for alignment and workmanship. All debris, dirt or other foreign objects shall be removed from system by a method of than flushing and material removed shall be disposed of property.	10.	Manhole head)(#
Highway, and State Department of Transportation. If local, county or A-572, A588, Grade 50, and hot dip galvanized in accordance with with galvanizing paint. All bolts, nuts, and washers shall be stainless	14.	Any pipes found with diameter deflections greater than 5% of the specified pipe diameter will be repaired or replaced. Any dignment differentials greater than 5% of the diameter of the pipe will be corrected or replaced.	11. 12.	The san necessa Any tes
is in diameter unless poor soils or frost conditions require greater s 4000 psi concrete is placed around the post. The overall sign and	15.	Any deaning, repairs, or replacement required due to failure of testing or poor workmanship shall be done by the Contractor at no additional expense to the contract.	13. 14.	After all dean ar
	<u>AS</u>	SPHALT PAVEMENT Asphalt shall be the type or types specified on the drawings. If no type is indicated the Contractor shall use a mix specified by the State Department of	15.	Grease · The pipe
ys or 3/4 strength is achieved. th, and curb ramps, shall meet Americans with Disabilities Act (ADA) the local building inspector and fire department.	ı. 2	Asphalt shall be the type of types specified on the advinings. In the type is indicated the contractor shall use of this specified by the state beport than to it Transportation for top and binder. In New York State that would be type Asphalt Concrete Binder Course Type 19 and Asphalt Concrete Surface Course Type 12.5. All asphalt shall be produced in state approved plants with state approved products. Asphalt will only be placed when the outside temperature is 45 degrees F and rising. Asphalt will never be placed on frozen material, during medium or	16.	Standard Increase circumfe
AND GUTTERS	_	heavy precipitation or when preceding precipitation has saturated the subbase and/or subgrade.		shall be structur
all be 4000 psi at 28 days made with type I or type II cement per quirements, unless otherwise noted. Sump for slip forming shall be 1	3.	Surfaces that will abut the new asphalt shall be tack coated prior placement of asphalt including curbs, gutter, existing asphalt and structures. Tack coat shall be applied neatly to match the lines and grades of the proposed abutting asphalt at a rate of .05 to .15 gallons per square yard.		
- 1 inch. Air entraining mixture shall meet the requirements of ASTM and placed concrete. Water reducing agent shall conform to ASTM ass A moisture loss of not more than .055 gr/sq cm when applied	4.	When binder is used as a working surface during construction, or there is a prolonged time period between binder and top placement, the surface must be power washed, not just swept, and a tack coat should be applied prior to installation of top course. In addition, any yielding area of pavement binder should be removed and replaced prior to application of the top course.		
ent with the pavement subbase as shown on the drawings. When ; sidewalks and gutters shall be placed on a minimum of 6 inches s of compacted subbase.	5.	Asphalt shall be placed in layers equal to those specified on the plans. Thickness of each layer or the thickness of all layers combined shall not vary more than 1/4 inch for thickness of 0 to 4 inches and 1/2 inch for thickness of 4 inches or greater, from those specified on the drawings. The asphalt shall also be tested for smoothness by laying a 16 foot straight edge on the pavement and verifying that there are no gaps greater than 1/4" in any direction.		
lition of ACI 318 "Building code requirements for reinforced concrete"	6.	Placement and compaction requirements shall be the same as those specified by the State Department of Transportation of which the project is located. The rolling shall be done in such a manner that will match joints and leave a smooth uniform surface while providing the proper compaction which will be 95% of laboratory density.		
the Concrete Reinforcing Steel Institute (CRSI) "manual of standard ed wire fabric shall be ASTM A 185, welded wire steel fabric.	7.	When matching into existing pavement, all match joints shall be saw cut to provide a straight smooth joint. The asphalt depth at the match point shall be equal to that of the proposed or existing which ever is greater.		

Reinforcing shall be in accordance with that specified on the drawings and the Concrete Reinforcing Steel Institute (CRS) "manual of standard

pover. The binder joints and the top joints shall be offset. The top course shall be placed parallel to the direction of travel. Asphalt shall be transported in covered trucks and scheduled in such a manner that will maintain asphalt temperature. Asphalt shall be rejected when temperatures fall below 250 degrees F or the minimum temperatures specified by the State Department of Transportation.

8. Poving equipment shall be of good condition and quality. Asphalt shall be placed by mechanical equipment except in small areas that are inaccessible to a

All sub-base, asphalt, curb or other work performed in a State, County or Municipal right-of-way shall be furnished, installed, inspected and completed ir accordance with their specifications, details and other requirements.

S ARE IN CONFLICT WITH THE REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION TH	E MORE

#### G AND LANDSCAPING

soil shall be removed from stockpiles and spread in the areas shown on the plans. The depth of topsoil shall be a minimum of 4 inches in n areas and a minimum of 12 inches in landscape planting areas. If enough topsoil is not available onsite, the Contractor is required to ort as necessary. All disturbed lawn areas are to receive topsoil, seed, mulching, and water until a healthy stand of grass is established. soil shall consist of fertile, natural agricultural soil substantially free of subsoil, stumps, roots, brush, stone, clay lumps, or similar objects r than 2 inches in the greatest diameter. Topsoil for reuse shall be screened if required to meet size and debris removal. Topsoil shall be oved by the owner at its source prior to transporting. The topsoil shall be fine graded to the lines and grades shown on the plans. The tractor is responsible for keeping topsoil, seed, fertilizer, etc. off structures, pavements, and other site amenities; and will clean up unwanted osits, at his expense.

v all areas to be cleared & seeded to 6" height maximum prior to beginning any new lawn work.

peen and till subgrade of lawn areas to a minimum depth of four inches, remove stones measuring 1.5 inches in any dimension, remove sticks, rubbish, and other extraneous matter. Limit preparation to areas which will be planted promptly after preparation.

paration of unchanged grades; where lawns are to be planted in greas that have not been altered of disturbed by excavating, grading, or oping operations, prepare soil for lawn planting as follows: till to a depth of six inches. apply soil amendments and initial fertilizers as fied, till soil to a homogenous mixture and fine texture and complete fine grading.

all new lawn areas to be seeded of all debris, branches, stumps, brush, logs, metal, sticks, stones, etc. larger than two inches in

, rake, and/or drag lawn areas to remove ridges and fill depressions to meet finish grades and to create a smooth, mowable lawn surface. natural dalamitic limestane containing at least 85% of total carbonates, and 30% magnesium carbonates; ground so that at least 90% sses a ten mesh sieve, and at least 50% passes a 100 mesh sieve.

topsoil shall have a pH of 6.0 to 6.8 and an organic content of 3 to 20%. The gradation of the topsoil shall be 100% passing 2 inch , 85 to 100% passing the 1 inch sieve, 65 to 100% passing the 1/4 inch sieve and 20 to 80% passing the No. 200 sieve.

fertilizer shall be 55% nitrogen, 10% phosphorus and 10% potash where 50% of the nitrogen is derived from ureaform source. Work into soil rate of 100 lbs per acre before seeding.

seed shall be "50% by weight, 85% purity, 85% germination of Pennfine Perennial Rye", "30% by weight, 97% purity, 85% germination of nlawn Red Fescue", "20% by weight, 85% purity, 80% germination of Common Kentucky Bluegrass" at a rate of 200 lbs per gare. Mulch all aded areas with approved straw at rate of 4000 lbs per acre. Maintain mulch as necessary and clean up upon satisfactory germination.

slape mix (Type B unmowed – 11/3H or steeper) apply at a rate of 100 lbs. per acre using the following proportions by weight: 15% aping Red Fescue, 35% Chemung Crownvetch, 25% Kentucky 31 Tall Fescue, and 25% Empire Birdsfoot Trefoil.

oseeding shall be applied in accordance with the following; fertilizer shall be placed at 80 pounds per acre, hydromulch at 1,200 pounds per water at 500 gallons per acre, and seed at a minimum of 220 pounds per acre. Inoculate at 4x manufacturer's rate. A non-harmful color itive which colors the hydroseed mixture green shall be added to the mixture to allow visual metering of its application. The hydroseed ture shall be sprayed upgrade and uniformly on the surface of the soil to form an absorbent cover, allowing percolation of water to the erlying soil.

Contractor will be responsible to water, reseed, or any other means necessary to ensure the growth of the lawn until a complete and orm stand of grass has grown and been cut at least three times. Water by approved means immediately after mulching and thereafter a imum of two times each week, or more when weather conditions require to a depth of one inch soil saturation. Mow all seeded areas to two height until find acceptance. In the event grass becomes too long, resulting in excessive dippings that could damage the lawn, the tractor shall remove all dippings at his expense. Lawn shall be presented to Owner in a condition that it may be maintained with standard wing equipment.

e substantial lawn remains (but is thin), mow, rake, aerate (if compacted), fill low spots, remove bumps, and scarify soil, fertilize, and seed. nove weeds before seeding, if extensive. Apply selective chemical weed killers as required. Apply mulch if required to maintain moist condition. ntings shall be supplied in accordance with the plans and ANSI 260.1 "American Standard for Nursery Stock" in good health, vigorous, and

e of insects, larvae, eggs, defects and disease. ts shall be located per the plans. The holes shall be excavated per the details on the drawings with the center slightly higher to promote nage. Use a topsoil backfill mix of 4 parts topsoil, 1 part peat moss, 1/2 part well rotted manure, 2 pounds 5–10–5 planting fertilizer

perly mixed per cubic yard. Berm around plants to form a bowl shape. layers of weed barrier made from fiberglass and ultraviolet light resistant shall be placed under all planting beds prior to mulching.

rees and shrubs shall be staked as detailed on the drawings. Tree wrapping will be provided at the base of all trees as detailed. all beds adjacent to building with 3 inch river rock graded gravel, 1" to 2" size range on fiber mat week barrier. Mulch all remaining with 50% shredded bark and 50% wood chips, 3/4 to 2 inch in size, uniformly mixed and free of elm wood. Mulch shall be placed

xmly over the planting bed allowing no weed barrier to be seen to a minimum depth of 3". Color to be chosen by Owner. indscaping shall be guaranteed for one year after final acceptance. Any plantings needing replacement will be guaranteed from the time of

perment if after final acceptance. Contractor shall maintain plants until completion and final acceptance of the entire project. Maintenance include pruning, cultivating, edging, remulching, fertilizing, weeding, watering as required for healthy growth, and application of appropriate cticides and fungicides necessary to maintain plants free of insect and disease. Repair all washouts, gullies, and areas of unsatisfactory rination by replacing topsoil, restaking, and reseeding, as required. Reset settled plants to proper grade and position. Restore planting saucer remove dead material. Tighten and repair quide wires and deficiencies within the first 24 hours of initial planting, and not less than twice week until final acceptance. Contractor shall request an inspection by the Owner upon establishment of the uniformly germinated lawn. owing the final acceptance, the Owner shall be responsible for maintenance of all landscaping on the premises.

desiccant: protective film emulsion, providing a protective film over plant surfaces, but permeable to permit transpiration. Mixed and applied in rdance with manufacturer's instructions. Apply to all broadleaf evergreen shrubs per manufacturer's recommendations.

#### RY SEWER SYSTEMS

sanitary sewer system shall be supplied and placed in accordance with all local, state and federal requirements

to pipe bedding detail for bedding and anti-seep collar requirements.

ess otherwise noted, sanitary pipe and fittings shall be Polyvinyl Chloride (PVC) per ASTM D 3034, SDR 35, with gaskets per ASTM D 3212, stomeric seal.

xemain pipe shall be Polyvinyl Chloride (PVC) per ASTM D 2241, SDR 21 (or lower if pressures are high in system) with gaskets per ASTM D , and elastomeric sed. The pipe shall be encased in a run of crush stone or arovel material with 100% passing the 1.0" sieve and 10% to 3% ing the 200 sieve. The mix shall be supplemented as needed to remove voids. Incorporate filter fabric around bedding or cradle stone if ground , silts, or sands are encountered.

pipe shall be placed in accordance with the manufacturer's recommendation and to the lines and grades shown on the drawings. Care shall be during backfill operations not to move or damage pipe or appurtenances while achieving the appropriate compaction requirements.

systems shall be visually inspected for alignment and workmanship. All debris, dirt or other foreign objects shall be removed from the system. taps to main lines shall be made with saddles when the tap is 1/2 the diameter or less of the existing pipe, but made with a sleeve when top is greater than 1/2 the diameter or equal to the existing pipe. If connections are required to equal size pipes of 8 inches or greater, a hole should be installed over the connection point and inverts formed. When connecting to an existing manhole, the connecting pipe hole shall cored and a press wedge installed. The connection shall be mortared up with waterproof/plug mortar. Inside the existing manhole, the existing t shall be broke out in a manner that protects from debris entering the live system, while a new invert is formed.

itary manholes shall be visually lamped after backfill to verify alignment, cleanliness and there is no damage to the system. After the system been backfilled for 30 days, the system shall be relamped and tested with a mandrel sized at 95% of the intended inside diameter.

vity systems shall be air tested between manholes to 3.5 psi for 5 minutes per ASTM F 1417 for plastic pipes. holes shall be tested separately for leakage or infiltration using ASTM C 969. The allowed leakage = 0.1 gallons/(feet of diameter)(feet of ad)(# of hours) and the test shall run for 24 hours.

sanitary sewer system shall be tested for infiltration and exfiltration using ASTM C 969. The system shall be broken up into sections when essary to consider groundwater depth, length and elevation differences. The allowable leakage shall be 100 gal/inch of pipe diameter/mile/day.

testing failure shall require the Contractor to repair or replace the failed section at no additional expense to the contract. all testing is complete, and before the system is turned over to the authority having jurisdiction, the system shall be checked to verify it is and free of dirt, debris and other foreign matter. The Contractor shall clean any sections required at no additional expense to the contract.

ase traps, if required, shall have cast iron lids and shall be installed per manufacturer's requirements.

pipe slope shall always meet the minimum, as listed in Section 33.4 of the Recommended Standards for Wastewater Facilities (Ten State

ease size of manhole if in the same horizontal plane there is two areas where the area between two pipes is less than 8 inches or ½ of the imference is supported by less than  $mu_2$  of the diameter of the manhole. Inverts shall be smooth cast in place concrete. Gaskets between risers all be rubber per ASTM C 443. Adjustment rings shall be precast concrete 4000 psi and 5 to 8% air entrainment. Inside and outside of ctures shall receive bitumastic coating.

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Taco Bell - Wa	tertown I	NΥ					
State St Watertown, NY 13601 Jefferson County, NY Project Name & Location:							
Specificat	ions						
Drawing Name:							
	Project	No.					
Date: 10/12/22	22-04	<u>408</u>					
Type: LG 50		тоо					

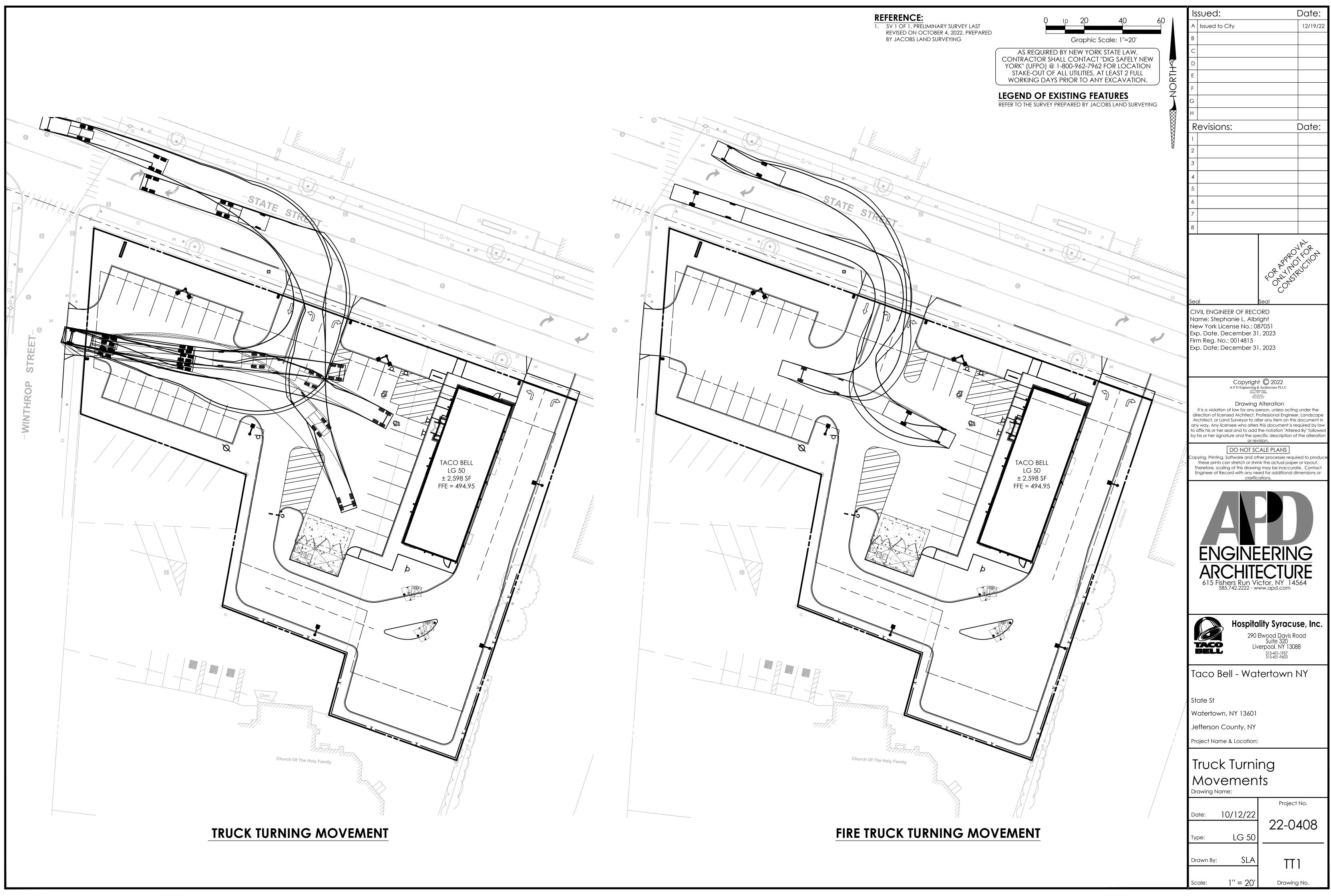
SLA

N.T.S.

Drawing No.

Drawn By:

cale



<sup>†</sup> 0.7	<sup>†</sup> .8	1.0	1.2	1.3	1.4	<sup>†</sup> 1.4	1.2	1.2	<sup>†</sup> 1.0	<sup>†</sup> .9	<b>b</b> .7	<sup>†</sup> 0.5	<sup>†</sup> 0.4	Ō.3	<sup>†</sup> .3	Ō.2	ō.2	<sup>†</sup> 0.2	<sup>†</sup> 0.2	ţ.2	<sup>0.2</sup>	ð.2	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<b>Ö</b> .1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<sup>†</sup> 0.0	Ō.0	ō.0	<sup>†</sup> 0.0	<sup>†</sup> 0.0	
1.0	1.2	<sup>†</sup> .6	1.9	÷2.2	<sup>+</sup> 2.4	<sup>+</sup> 2.4	<sup>+</sup> 2.2	<sup>+</sup> 2.2	1.8	1.5	1.1	<sup>†</sup> 0.9	<sup>†</sup> 0.7	<sup>†</sup> 0.6	<sup>†</sup> 0.4	<sup>†</sup> .3	<sup>†</sup> 0.3	<sup>†</sup> 0.3	<sup>†</sup> 0.3	<sup>†</sup> .3	<sup>†</sup> .2	<sup>†</sup> .2	<sup>†</sup> .2	<sup>†</sup> .2	<sup>†</sup> .2	<b>Ö</b> .1	<b>Ö</b> .1	<b>Ö</b> .1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<b>†</b> .0	<sup>†</sup> 0.0	<sup>†</sup> 0.0	<sup>†</sup> 0.0	
1.3	<b>1</b> .7	<sup>‡</sup> 2.3	*2.9	7 *3.4	<sup>+</sup> 3.7	4.0	<sup>+</sup> 4.3	<sup>+</sup> 3.9	<sup>+</sup> 3.2	+2,5	1.8	1.4	1.1	<sup>†</sup> 0.9	<sup>†</sup> 0.7	ō.5	<sup>†</sup> 0.5	<sup>†</sup> 0.5	<sup>†</sup> 0.4	<sup>†</sup> 0.4	<sup>†</sup> 0.4	<sup>†</sup> 0.3	<sup>†</sup> 0.3	<sup>†</sup> 0.3	<sup>†</sup> 0.2	ð.2	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<b>Ö</b> .1	<sup>†</sup> 0.0	<sup>†</sup> 0.0	Ō.0	
1.5	<sup>‡</sup> 2.1	<sup>+</sup> 2.8	1/ *3.7	<sup>+</sup> 4.7	5.7	2.ל	7.9	<sup>‡</sup> 6.5	5.8	4.1	<sup>+</sup> 3.1	*2.4	1.8	1.4	1.2	1.0	<sup>†</sup> 0.9	<sup>†</sup> 0.8	<sup>†</sup> 0.7	<sup>†</sup> 0.6	<sup>†</sup> 0.6	ō.5	°.4	<sup>*</sup> 0.4	ð.3	ð.3	<sup>†</sup> 0.2	<sup>†</sup> .2	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<b>Ö</b> .1	<sup>†</sup> 0.1	<sup>†</sup> 0.0	Ō.0	
1.5	<sup>‡</sup> 2.1	<sup>†</sup> 2.9	<sup>+</sup> 4.1	5.5	<sup>+</sup> 6.8	<sup>‡</sup> 8.7	10.4	<sup>†</sup> 9.4	*8.9	<sup>+</sup> 6.3	4.4	<sup>+</sup> 3.4	¤ *2.7	*a.1	1.8	1.6	1.5	1.4	1.2	<b>1</b> .1	1.0	ō.9	<sup>†</sup> 0.7	<sup>†</sup> 0.6	<sup>†</sup> 0.4	ð.3	<sup>†</sup> .3	<sup>†</sup> .2	<sup>†</sup> 0.2	<sup>†</sup> 0.1	<b>0</b> .1	<sup>†</sup> 0.1	<sup>†</sup> 0.0	<sup>†</sup> 0.0	
1.5	‡2.0	<sup>†</sup> 2.9	*4.1	5.7	/ ħ.1	<sup>\$9.3</sup>	AT.I.	11.1	10.1	7.8	<sup>‡</sup> 6.0	<sup>+</sup> 4.7	<sup>+</sup> 3.7	3.0	<sup>‡</sup> 2,5	+2.5	<sup>‡</sup> 2.4	<sup>+</sup> 2.3	*2.2	1.9	1.8	1.5	1.2	<sup>†</sup> 0.9	<sup>†</sup> 0.6	ð.5	<sup>†</sup> 0.4	ō.3	<sup>†</sup> 0.2	Ō.1	<b>0</b> .1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<sup>†</sup> 0.0	
1.3		2.7	/ *4.1	÷,0	ħ.5	<sup>\$</sup> 9.5	11.6	10.9	9.4	8.0	<sup>+</sup> 6.7	₅.6	<sup>+</sup> 4.7	*3.9	+36	+3.7	<sup>+</sup> 3.7	*3.6	<sup>†</sup> 3.7	<sup>+</sup> 3.5	<sup>+</sup> 3.3	<sup>‡</sup> 2.5	<sup>‡</sup> 2.1	<sup>†</sup> .4	1.0	ð.8	<sup>†</sup> 0.6	<sup>†</sup> 0.5	<sup>†</sup> 0.3	<sup>†</sup> .2	<b>Ö</b> .1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<sup>†</sup> 0.0	
1.1	1.7	<sup>+</sup> 2.6	<sup>+</sup> 3.9	5.8	7.6	, 9.2	10. <i>3</i>	<sup>*</sup> 9.8	*8.7	8.ל	6.8	5.9	ŧ.2	<sup>+</sup> 4.8	4.6	کر 4.9/	\$ 37	т <u>ь.</u> в	<sup>+</sup> 6.1	<sup>+</sup> 6.9	5.7	<sup>+</sup> 4.7	<sup>+</sup> 3.3	*2. <u>4</u>	1.7	1.2	<sup>†</sup> 0.8	<sup>†</sup> 0.8	<sup>†</sup> 0.5	<sup>†</sup> .3	°.2	<sup>†</sup> 0.1	<sup>†</sup> 0.1	<sup>†</sup> 0.1	
ō.9	1.5	<sup>‡</sup> 2.3	<sup>†</sup> 3.6	5.3	<b>†</b> .З	<sup>†</sup> 9.1	€9.5	/ •9.0	*8.7	י 5.ל	/		/	///	/	/		/																	
<u>t.</u> 9	1.4	<sup>‡</sup> 2.1	<sup>†</sup> 3.2	<sup>+</sup> 4.7	÷.5	5.9	₹8.3	*8.0	7.6	5.0	÷6.2	5.8	5.6	¥.5	5.7	<i>т</i> 6.3	7.3	*8.2	- <sup>t</sup> 9.8	11.1	10.4	€.9	7.4	<sup>+</sup> 4.7	<sup>+</sup> 3.4	<sup>‡</sup> 2.4	1.6	1.6	1.2	<sup>†</sup> .8	ō.5	Ъ.З	<sup>†</sup> .2	<b>Ö</b> .1	
ð.7	1.2	1.9	2.7	/*4.0	5.4	÷6.4	<b>†</b> 6.7	<sup>+</sup> 6.8	<sup>+</sup> 6.8	÷.6	<sup>+</sup> 6.4	<b>*</b> 6.1	5.9	₩ 5.8	5.9/	÷6.6	7.8	<sup>‡</sup> 8.6	10.0	11.9	Q1.1	9.2	7.1	5.5	4.1	7 2.9	1.9	1.2	1.6	1.2	<sup>†</sup> 0.8	<del>.5</del>	6.2	<b>Ö</b> .1	
ð.6	1.0	1.6	2.3		4.2	/	/		5.9	÷.3		÷.5	<sup>‡</sup> 6.5				\$.0		/	/		٦.9		5.3	+4.0	<sup>‡</sup> 2.9	+20	1.4	2.5			toz /	t.3	Ō.1	
ō.5	ō.8	1.3	1.9	2.5	*3.2	<sup>+</sup> 3.6	3.9	4.4	<sup>+</sup> 4,8	5.6 /	÷.2	<b>6</b> .1	<sup>+</sup> 6.2	<sup>+</sup> 6.6	<b>6</b> .7	5.0	7.8	*8.8	<sup>‡</sup> 9.7	9.74 N	8.5	٦.3	5.9	4.4				14	t31	~.5 *2.5	1.8	Z1.0	to.2	Ō.1	
<b>0</b> .4	ħ.7	1.0	1.5	1.9	±2.3	2.6	2.9	*3.4	4.0	₫.0	<sup>†</sup> 6.1	5.2 <sup>+</sup>	<sup>+</sup> 6.2	<sup>+</sup> 6.4	<sup>+</sup> 6.5	÷6.6	້າ.2	<sup>‡</sup> 8.0	<sup>‡</sup> 8.7	*8.5	7.3	<sup>+</sup> 6.1	<sup>+</sup> 4.6	*3.#					*3.1	*2.6	÷2.4	1.3	<sup>†</sup> 0.3	<b>Ö</b> .1	
<sup>†</sup> 0.4	ð.5	<sup>†</sup> 0.8	1.1	1.4	1.6	1.8	<sup>‡</sup> 2.0	2.3	<sup>*</sup> 3.0	4.1	5.6	6.3	<sup>+</sup> 6.2	5.5	5.3	5.6	<sup>+</sup> 6.1	<b>*</b> 6.6	5.0	<sup>+</sup> 6.7	922 75.7	4.5	BA						4.1	3.4	<sup>‡</sup> 2.3	ð.7	<b>.</b> 0	<b>0</b> .1	
<sup>†</sup> .З	<sup>†</sup> 0.4	ð.6	ð.8	1.0	<b>1</b> .1	1.2	1.3	1.7	<sup>‡</sup> 2.3	ţ.	/ 5.2	B 5.2	<sup>+</sup> 4.6	<sup>+</sup> 4.4	4.2	4.4	<sup>+</sup> 4.7	₽.5	₫.3	4.9	<sup>+</sup> 4.1	*3.2	2.5	7					<sup>+</sup> 4.3	3.6	†2,72	<sup>†</sup> 0.4	<sup>†</sup> 0.0	ō.o	
°.2	Ѣ.з	ō.5	<sup>†</sup> 0.6	<b>.</b> 7	<sup>†</sup> 0.8	Ъ.9	1.0	1.3	<sup>‡</sup> 2.0	<sup>†</sup> 3.5	<b>5</b> .0	<sup>+</sup> 4.4	<sup>+</sup> 3.4	‡2.\$	<sup>†</sup> 3.1	<sup>†</sup> 3.3	<sup>†</sup> 3.5	<sup>†</sup> 3.8	<sup>†</sup> 3.8	<sup>+</sup> 3:4	<sup>‡</sup> 2.8	<sup>‡</sup> 2.3	1.8					+4.4	<sup>+</sup> 4.3	<sup>†</sup> 3.9	1.9	<sup>†</sup> 0.1	<sup>†</sup> 0.0	Ō.0	
°.2	Ѣ.з	<sup>†</sup> 0.4	<sup>†</sup> 0.5	<sup>†</sup> 0.5	<sup>†</sup> 0.6	<sup>†</sup> 0.6	ō.8	<b>1</b> .1	1.6	<sup>‡</sup> 2.0	<sup>‡</sup> 2.6	<sup>†</sup> 32	<sup>‡</sup> 2.3	* <b>2</b> .1	<sup>‡</sup> 2.1	<sup>†</sup> 2.4	<sup>‡</sup> 2.6	<sup>‡</sup> 2.7	<sup>‡</sup> 2.6	tot	<sup>‡</sup> 2.0	1.p	1.0					<sup>+</sup> 4.3	4.2	<sup>†</sup> 3.3	1.0	<sup>†</sup> 0.1	<sup>†</sup> 0.0	<sup>†</sup> 0.0	
°.2	ð.2	Ъ.З	<sup>†</sup> 0.3	<sup>†</sup> 0.4	0.4	<sup>†</sup> 0.5	<sup>†</sup> 0.6	ō.8	1.0	1.3	1.6	2.0	1.8	1.6	1.7	1.8	1.9	<sup>‡</sup> 2.0	1.8 _	1.6	İ.4	1.0					4.5	<sup>+</sup> 4.3	4.5	12/7	ð.5	<sup>†</sup> 0.1	<sup>†</sup> 0.0	<sup>†</sup> 0.0	
<b>Ö</b> .1	ħ.2	<sup>†</sup> .2	ð.3	ð.3	ð.3	<sup>†</sup> 0.4	<sup>†</sup> 0.4	0.5	<b>b</b> .7	1.0	1.2	1.5	1.7	1.6	1.6	1.6	1.6	1.6	1.5	1.2	ð.9	ð.7	/				4.5	<sup>+</sup> 4.3	<sup>*</sup> 3.6	1.9	t.2	ō.o	<b>.</b> 0	<b>0</b> .0	
<sup>†</sup> 0.1	<sup>†</sup> 0.1	ð.2	ð.2	ð.2	<sup>†</sup> 0.2	ъ.з	ф.з	<sup>†</sup> 0.4	<sup>†</sup> 0.6	<sup>†</sup> 0.8	<sup>1</sup> .1	1.4	1.8	1.9	to 7 /	1.7	1.6	1.5	1.3	1.2	0.8	to #					<sup>+</sup> 4.0	3.7	<sup>‡</sup> 2.7	1.0	<b>Ö</b> .1	<sup>†</sup> 0.0	<sup>†</sup> 0.0	ō.o	
Ō.1	Ō.1	<b>Ö</b> .1	ð.1	ð.2	<sup>†</sup> 0.2	ð.2	ъ.з	ф.з	ō.5	ð.7	1.0	1.5	*2.0	<sup>†</sup> 0.4		0.6	0.6	¢.6	1.4	1.3	1.3					<sup>7</sup> 3.1	<sup>3.2</sup>	<sup>‡</sup> 2.9	† <b>7</b> .0	ð.5	ð.1	<b>.</b> 0	Ō.0	Ō.0	
<sup>†</sup> 0.1	Ō.1	<b>0</b> .1	Ō.1	Ō.1	<sup>†</sup> 0.1	ō.2	<sup>†</sup> 0.2	<sup>†</sup> .З	0.4	<sup>†</sup> 0.7	1.1	1.7	<sup>‡</sup> 2.4	2.9					2.0	1.8	1.8	1.8	±2,0			<sup>‡</sup> 2.9	<sup>†</sup> 3/0	*2.4	1.4	ð.2	<b>.</b> 00	<b>.</b> 0	Ō.0	Ō.0	
Ō.1	ð.1	<b>0</b> .1	<b>Ö</b> .1	<b>Ö</b> .1	Ō.1	<sup>†</sup> 0.1	Ō.1	<sup>†</sup> .2	0.4	<sup>†</sup> 0.7	1.1	1.9	<sup>‡</sup> 2.9	\$3.6	*3.9			<sup>†</sup> 3.2	<sup>‡</sup> 2.7	\$2.5	¥.4	<b>(3</b> .4	<sup>*</sup> 2.6	* <u>+</u> 8	tale	<sup>†</sup> 3.5	3.0	<sup>*</sup> 2.1	ъ.7 /	Ō.1	Ō.0	ō.o	<b>.</b> 0	Ō.0	
Ō.0	ð.1	<b>0</b> .1	<b>0</b> .1	<b>Ö</b> .1	Ō.1	<sup>†</sup> 0.1	Ō.1	<b>Ö</b> .1	ð.3	<sup>†</sup> 0.6	(1.1	<sup>‡</sup> 2.0	\$.0	<sup>‡</sup> 3.9	4.3	¥.4	4.6	<sup>+</sup> 4.5	<sup>*</sup> 4.0	⁺3.4	0.Ė	<sup>3.0</sup>	<sup>*3.2</sup>	4.0	4.4	4.1 <sup>7</sup>	<sup>+</sup> 3.3	*2.1	0.3	Ō.1	Ō.0	ð.0	<b>.</b> 0	<b>0</b> .0	
ō.0	ō.o	<b>0</b> .0	Ō.1	<b>Ö</b> .1	Ō.1	Ъ.1	ð.1	<b>Ö</b> .1	Ō.1	<sup>†</sup> 0.4	0.9	1.7	<sup>‡</sup> 2.6	*3.4	4.3	<sup>‡</sup> 4.3	<sup>+</sup> 4.1	<sup>+</sup> 4.5	+4.6	<sup>+</sup> 4.3	3.7	L 34	3.7	4.5	<sup>+</sup> 4.9	<sup>+</sup> 4.5	<sup>+</sup> 3.4	2.0	, р.з	Ō.1	ō.o	ō.o	Ō.0	Ō.0	
Ō.0	ō.o	Ō.0	<b>t</b> .0	ō.o	ð.1	Ъ.1	ð.1	ð.1	Ō.1	<sup>†</sup> 0.1	to.2	t0.5	1.6	<sup>\$2,3</sup>	<sup>‡</sup> 3.4	4.4	4.5	4.5	4.5	<sup>+</sup> 4.3	<sup>+</sup> 3.8	±3.6	4.0	5.0	<sup>+</sup> 4.8	<sup>+</sup> 4.6	<sup>+</sup> 3.3	1.4	<sup>†</sup> 0.1	Ō.0	Ō.0	ð.0	<b>.</b> 0	Ō.0	
Ō.0	ō.o	Ō.0	<b>t</b> .0	Ō.0	ō.o	<sup>†</sup> 0.0	ō.o	ð.1	Ō.1	<sup>†</sup> 0.1	ð.1	Ō.1	ð.2	0.3	<u>1</u> .3		<sup>+</sup> 3.2	<sup>+</sup> 3.6	*3.6	<sup>***</sup> <sup>*</sup> 3.7 C	3.6	<sup>†</sup> 3.6		<b>5</b> .1	<sup>+</sup> 4.5	<sup>+</sup> 4.6	*3. <b>2</b>	0.6	<sup>†</sup> 0.1	Ō.0	Ō.0	ō.o	<b>.</b> 0	Ō.0	
Ō.0	ð.o	Ō.0	<sup>†</sup> 0.0	<sup>†</sup> 0.0	ō.o	Ō.0	ō.o	<b>0</b> .0	<b>0</b> .0	<sup>†</sup> 0.1	<b>0</b> .1	Ō.1	<b>0</b> .1	<sup>†</sup> .2	<sup>†</sup> 0.3	0.5	to Z	1.7	2.4	<sup>‡</sup> 2.8	<sup>‡</sup> 3.1	<sup>+</sup> 3.4	4.5	<sup>+</sup> 4.8	<sup>+</sup> 4.4	<sup>‡</sup> 4.5		0.4	<sup>†</sup> 0.1	<sup>†</sup> 0.0	Ō.0	ð.o	<sup>†</sup> 0.0	<sup>†</sup> 0.0	
<sup>†</sup> 0.0	ð.o	Ō.0	<sup>†</sup> 0.0	<sup>†</sup> 0.0	ō.o	ō.0	ō.o	<b>0</b> .0	<b>0</b> .0	<sup>†</sup> 0.0	<b>0</b> .1	Ō.1	<b>0</b> .1	<b>0</b> .1	<sup>†</sup> 0.1	<sup>†</sup> .2	ъ.з	0.5	1.0	1.5	<sup>‡</sup> 2.4	<sup>+</sup> 3.2	<sup>+</sup> 4.3	<sup>+</sup> 4.6	<sup>‡</sup> 4.5	<sup>†</sup> 3.7	1.4	ð.2	<sup>†</sup> 0.0	<sup>†</sup> 0.0	<b>0</b> .0	°.0	<sup>†</sup> 0.0	<sup>†</sup> 0.0	
ō.o	ð.o	ō.o	Ō.0	ō.o	ō.o	ō.0	ō.o	<b>b</b> .0	ō.o	<sup>†</sup> 0.0	ō.o	Ъ.1	<b>0</b> .1	<b>0</b> .1	<sup>†</sup> 0.1	ţ.5	ъ.з	ð.5	<sup>†</sup> .8	1.3	1.9	<sup>‡</sup> 2.7	<sup>‡</sup> 3.8	<sup>+</sup> 4.2	<sup>‡</sup> 3.7	<sup>†</sup> 2.¢	0.4	<b>Ö</b> .1	ō.o	<sup>†</sup> 0.0	ō.o	ð.o	Ō.0	ō.o	
ō.o	ð.o	ō.o	Ō.0	<sup>†</sup> 0.0	ō.o	Ō.0	ō.o	ō.o	ō.o	<sup>†</sup> 0.0	ō.o	Ъ.1	<b>0</b> .1	Ō.1	<sup>†</sup> 0.1	ţ.5	ъ.з/	t <u>0</u> .5	ţ.8	1.2	1.8	<sup>‡</sup> 2.4	<sup>‡</sup> 3.0	<sup>+</sup> 3.2	<sup>‡</sup> 2.8	1.8	<sup>†</sup> 0.1	Ъ.о	ō.o	<sup>†</sup> 0.0	ō.o	ð.o	<sup>†</sup> 0.0	<b>.</b> 0	
ō.o	ō.o	ō.o	<sup>†</sup> 0.0	Ō.0	ō.o	<sup>†</sup> 0.0	ō.o	ō.o	ō.o	<sup>†</sup> 0.0	ō.o	ð.o	<b>0</b> .1	<b>D</b> .1	<sup>†</sup> 0.1	ţ.5	ЪЗ	\$.4	ð.7	1.0	1.3	1.7	<sup>‡</sup> 2.0	<sup>‡</sup> 2.2	1.9	ð.8	<sup>†</sup> 0.0	Ъ.о	ō.o	<sup>†</sup> 0.0	Ō.0	ō.o	<sup>†</sup> 0.0	<sup>†</sup> 0.0	
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Label	CalcType
CALCULATION POINTS @ GRADE	Illuminance
PARKING & DRIVING SUMMARY	Illuminance

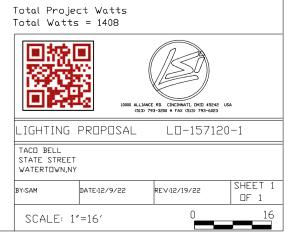
Luminaire Schedule									
Symbol	Qty	Label	Arrangement	Description	LLD	LDD	LLF	Arr. Lum. Lumens	Arr. Watts
\$	2	А	2 @ 90°	MRM-LED-24L-SIL-FT-50-70CRI-D90-25' MH	1.000	1.000	1.000	51928	352
	1	В	SINGLE	MRM-LED-24L-SIL-FT-50-70CRI-SINGLE-25' MH	1.000	1.000	1.000	25964	176
	3	С	SINGLE	MRM-LED-24L-SIL-FT-50-70CRI-IL-SINGLE-25' MH	1.000	1.000	1.000	16436	176

### PHOTOMETRIC EVALUATION NOT FOR CONSTRUCTION

Based on the information provided, all dimensions and luminaire locations shown represent recommended positions. The engineer and/or architect must determine the applicability of the layout to existing or future field conditions.

This lighting plan represents illumination levels calculated from laboratory data taken under controlled conditions in accordance with The Illuminating Engineering Society (IES) approved methods. Actual performance of any manufacturer's luminaires may vary due to changes in electrical voltage, tolerance in lamps/LED's and other variable field conditions. Calculations do not include obstructions such as buildings, curbs, landscaping, or any other architectural elements unless noted. Fixture nomenclature noted does not include mounting hardware or poles. This drawing is for photometric evaluation purposes only and should not be used as a construction document or as a final document for ordering product.







SYMBOL	ITEM/MATERIAL	MANUFACTURER	COLOR
1	FIELD BRICK VENEER	BELDEN	MODULAR BRICK '8530'
2	TOWER BRICK VENEER	BELDEN	MODULAR BRICK 'REGAL BLEND'
3	MANUFACTURED STONE VENEER	BORAL CULTURED STONE	COBBLE FIELD CUT, 'ECHO RIDGE'
4	PARAPET CAP	DUROLAST	IRON ORE (SW7069), SEMI-GLOSS, 24 GA GALVANIZED
5	E.I.F.S.	STO. CORP.	NATURAL CHOICE (SW7011), SEMI-GLOSS
6	CAST STONE WATERTABLE SILL	THE CAST STONE COMPANY	STANDARD WHITE
7	STOREFRONT WINDOWS	OLD CASTLE	CLEAR ANODIZED
8	PIPE BOLLARDS	STREET SMART	YELLOW - 1/4" THICK PLASTIC COVER (US.POSTMAN.COM OR EQUAL)
9	PARAPET BACK ROOFING	DUROLAST	FACTORY COLOR 'TAN', EQUAL ALTERNATE ALLOWED
10	GUTTERS & DOWNSPOUTS		ROCK BOTTOM (SW7062)
11	HOLLOW METAL DOOR	BY OWNER	NATURAL CHOICE (SW7011), SEMI-GLOSS
12	AWNINGS	SIGNAGE VENDOR	BLACK
13	BACK OF TOWER BRICK	BELDEN	MODULAR THIN BRICK 'REGAL BLEND'
13	BACK OF TOWER BRICK	BELDEN	MODULAR THIN BRICK 'REGAL BLEND'



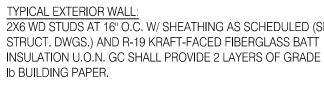


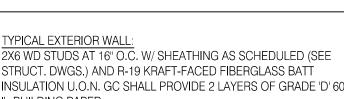


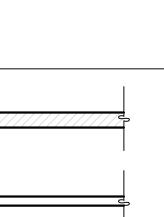


WALL SUBSTRATES: - DINING ROOM: 6" HIGH CEMENT BOARD AT BASE OF WALL, 1/2" GYPSUM WALLBOARD FROM 6" A.F.F. TO 6" ABOVE CEILING HEIGHT U.O.N. SEE 6 & 8/A6.3 (NOTE: THE CEM FLEXIBILITY.) WALL BEHIND THE POS COUNTER AND AROUND DINING ROOM SODA MACHINE TO HAVE 1/2" CEMENT BOARD FULL HEIGHT - KITCHEN WALLS AND DINING ROOM CLOSET: 1/2" CEMENT WALLBOARD FROM T.O. SLAB T.O. 12" A.F.F. AT 12" A.F.F., USE 1/2" CDX PLYWOOD W/FRP SURFACE FINISH TO 6" ABOVE CEILING HEIGHT U.O.I SPECIFIED THE PLYWOOD SHALL BE CONTINUOUS FROM SILL PLATE TO TOP PLATE. SEE 4/A6.3. - RESTROOM WALLS: 1/2" CEMENT WALLBOARD FROM T.O. SLAB OR T.O CONCRETE CURB TO 6" ABOVE CEILING HEIGHT. FINISH AS SCHEDULED - ALL OTHER FRAME WALL CONDITIONS:

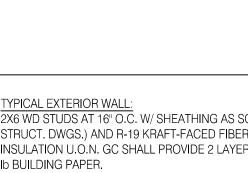
-FACED FIBERGLASS BATT

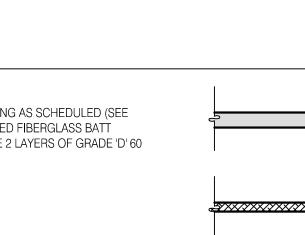


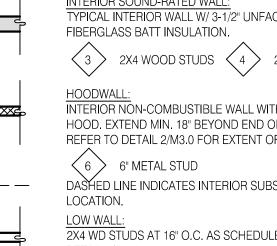




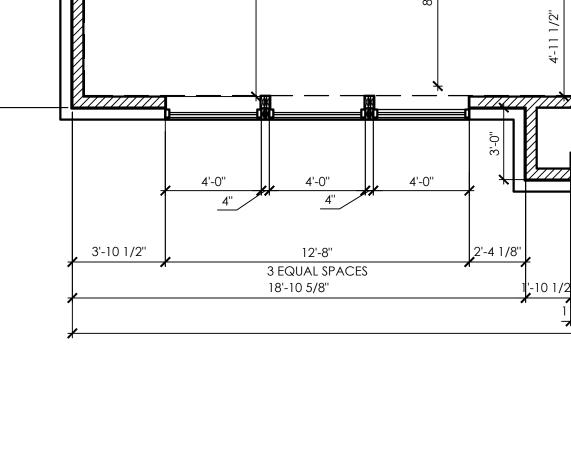
TYPICAL EXTERIOR WALL:
2X6 WD STUDS AT 16" O.C. W/ SHEA STRUCT. DWGS.) AND R-19 KRAFT-F
INSULATION U.O.N. GC SHALL PRO'

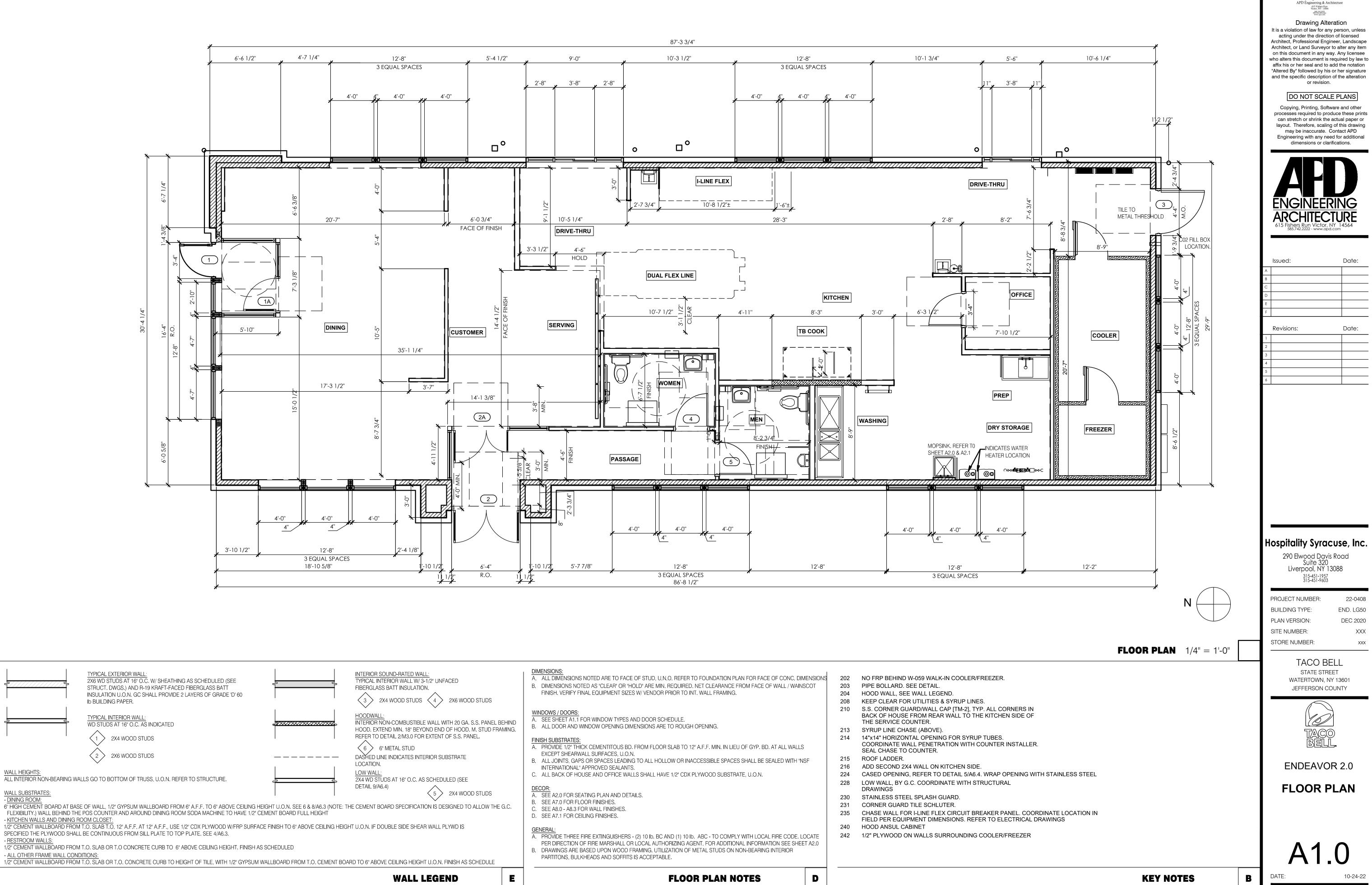




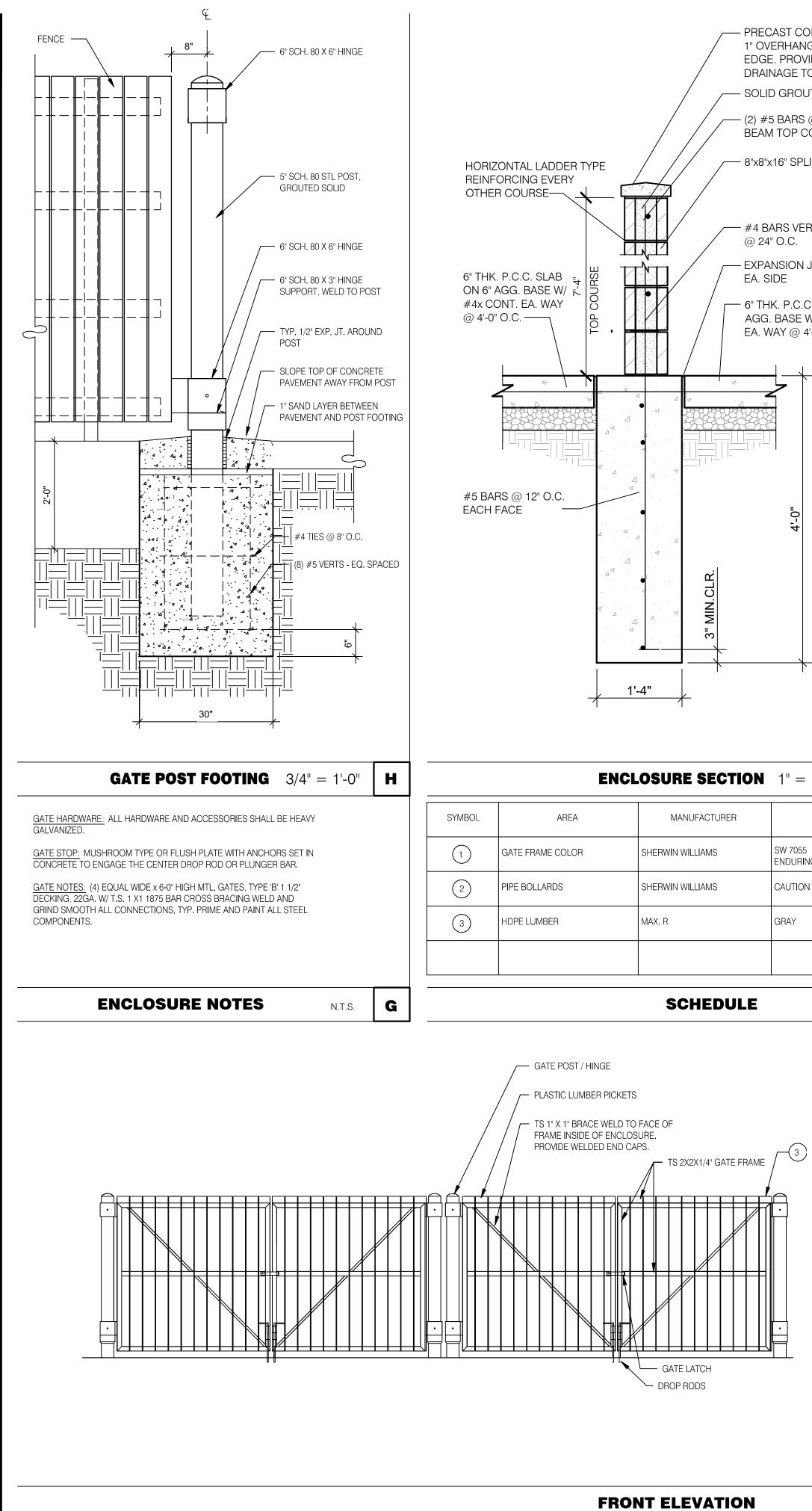


WALL LEGEND E	FLOOR PLAN NOTES D		
) 6" ABOVE CEILING HEIGHT U.O.N. FINISH AS SCHEDULE	<ul> <li>B. DRAWINGS ARE BASED UPON WOOD FRAMING. UTILIZATION OF METAL STUDS ON NON-BEARING INTERIOR PARTITONS, BULKHEADS AND SOFFITS IS ACCEPTABLE.</li> </ul>		
	A. PROVIDE THREE FIRE EXTINGUISHERS - (2) 10 lb. BC AND (1) 10 lb. ABC - TO COMPLY WITH LOCAL FIRE CODE. LOCATE PER DIRECTION OF FIRE MARSHALL OR LOCAL AUTHORIZING AGENT. FOR ADDITIONAL INFORMATION SEE SHEET A2.0	242	1/2" PLYWOOD ON WALLS SURROUNDING COOLER
).N. IF DOUBLE SIDE SHEAR WALL PLYWD IS	GENERAL:	240	FIELD PER EQUIPMENT DIMENSIONS. REFER TO EL HOOD ANSUL CABINET
	C. SEE A8.0 - A8.3 FOR WALL FINISHES. D. SEE A7.1 FOR CEILING FINISHES.	235	CHASE WALL FOR I-LINE FLEX CIRCUIT BREAKER F
MENT BOARD SPECIFICATION IS DESIGNED TO ALLOW THE G.C.	B. SEE A7.0 FOR FLOOR FINISHES.	231	CORNER GUARD TILE SCHLUTER.
$\langle 5 \rangle$ 2X4 WOOD STUDS	A. SEE A2.0 FOR SEATING PLAN AND DETAILS.	230	STAINLESS STEEL SPLASH GUARD.
TAIL 9/A6.4)	DECOR:	228	LOW WALL, BY G.C. COORDINATE WITH STRUCTUF DRAWINGS
<u>N WALL:</u> I WD STUDS AT 16" O.C. AS SCHEDULED (SEE	C. ALL BACK OF HOUSE AND OFFICE WALLS SHALL HAVE 1/2" CDX PLYWOOD SUBSTRATE, U.O.N.	224	CASED OPENING, REFER TO DETAIL 5/A6.4. WRAP
CATION.	INTERNATIONAL" APPROVED SEALANTS.	216	ADD SECOND 2X4 WALL ON KITCHEN SIDE.
SHED LINE INDICATES INTERIOR SUBSTRATE	B. ALL JOINTS, GAPS OR SPACES LEADING TO ALL HOLLOW OR INACCESSIBLE SPACES SHALL BE SEALED WITH "NSF	215	ROOF LADDER.
6 6" METAL STUD	A. PROVIDE 1/2" THICK CEMENTITOUS BD. FROM FLOOR SLAB TO 12" A.F.F. MIN. IN LIEU OF GYP. BD. AT ALL WALLS EXCEPT SHEARWALL SURFACES, U.O.N.		COORDINATE WALL PENETRATION WITH COUNTER SEAL CHASE TO COUNTER.
FER TO DETAIL 2/M3.0 FOR EXTENT OF S.S. PANEL.	FINISH SUBSTRATES:	214	14"x14" HORIZONTAL OPENING FOR SYRUP TUBES.
ERIOR NON-COMBUSTIBLE WALL WITH 20 GA. S.S. PANEL BEHIND OD, EXTEND MIN, 18" BEYOND END OF HOOD, M, STUD FRAMING,	B. ALL DOOR AND WINDOW OPENING DIMENSIONS ARE TO ROUGH OPENING.	213	SYRUP LINE CHASE (ABOVE).
	WINDOWS / DOORS: A. SEE SHEET A1.1 FOR WINDOW TYPES AND DOOR SCHEDULE.	210	S.S. CORNER GUARD/WALL CAP [TM-2], TYP. ALL C BACK OF HOUSE FROM REAR WALL TO THE KITCH THE SERVICE COUNTER.
$3$ 2X4 WOOD STUDS $\langle 4 \rangle$ 2X6 WOOD STUDS		208	KEEP CLEAR FOR UTILITIES & SYRUP LINES.
	FINISH. VERIFY FINAL EQUIPMENT SIZES W/ VENDOR PRIOR TO INT. WALL FRAMING.	204	HOOD WALL, SEE WALL LEGEND.
ERGLASS BATT INSULATION.	B. DIMENSIONS NOTED AS "CLEAR" OR "HOLD" ARE MIN. REQUIRED. NET CLEARANCE FROM FACE OF WALL / WAINSCOT	203	PIPE BOLLARD. SEE DETAIL.
ERIOR SOUND-RATED WALL: PICAL INTERIOR WALL W/ 3-1/2" UNFACED	DIMENSIONS: A. ALL DIMENSIONS NOTED ARE TO FACE OF STUD, U.N.O. REFER TO FOUNDATION PLAN FOR FACE OF CONC. DIMENSIONS	202	NO FRP BEHIND W-059 WALK-IN COOLER/FREEZER



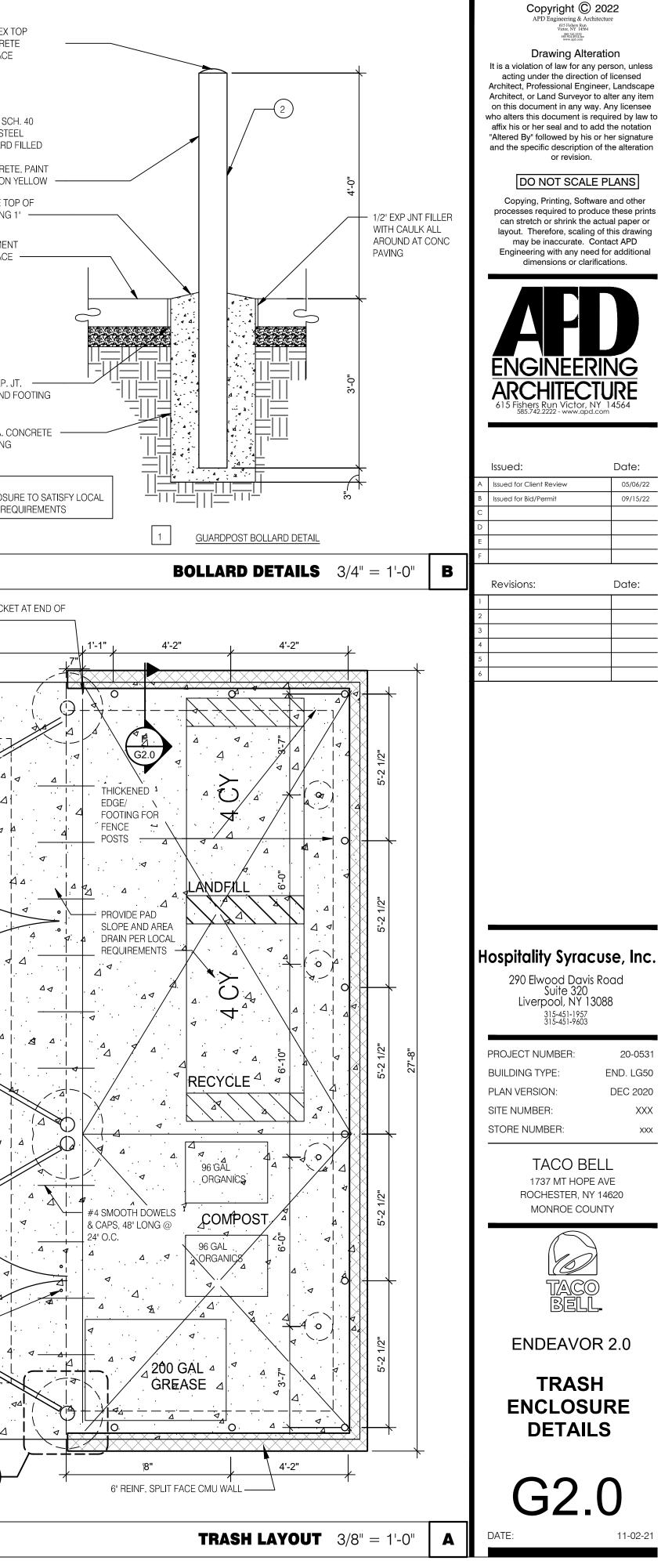


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DNC. CAP W/ G W/DRIP IDE POSITIVE O EDGE		CONVEX
JT ALL CORES		SURFAC
@ BOND OURSE		6" DIA. S
IT FACE C.M.U.		TUBE ST BOLLAR WITH CONCRE
RTICALLY		SLOPE T FOOTING
JOINT, TYP.		PAVEME SURFAC
C. SLAB ON 6" W/ #4x CONT. I'-0" O.C.		
-		1/2" EXP AROUNI
		18" DIA. FOOTING
		NOTE: ENCLOS
		CODE R
		INSTALL PICK RAIL
<del>.</del>		SEE SITE PLAN
1'-0" <b>F</b>		
COLOR		
NG BRONZE		A A REPORT, WHICHEVER IS GREATER) CONCRETE PAVEMENT W/24" WIDE X A 10" THICKENED EDGE ABOLIND ENTIDE
N YELLOW		<ul> <li>∠ △ PERIMETER, 28 DAY MIN.</li> <li>∠ △ COMPRESSION STRENGTH OF 4,000 PSI</li> <li>✓ △ WITH #4 CONT. E.W. @ 24" O.C.</li> </ul>
		SUB-GRADE PER GEOTECHNICAL
		$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$
N.T.S.		
		GATES 7 GATES 7 A SEE DETAIL C A A A A A A A A A A A A A A A A A A A
		PROVIDE 24" WIDE X 12" V I THICKENED EDGE ON 7" CONCRETE SLAB ENTIRE
		A A A A A A A A A A A A A A A A A A A
		$\begin{array}{ c c c c c c } \hline & & & & & & & & & & & & & & & & & & $
		CONCRETE NOTES: 1. INSTALL 1/2" EXP. JT. MATERIAL AND SEALANT BETWEEN CONCRETE SLABS
		AND SEALANT BETWEEN CONCRETE SLABS 2. INSTALL CONTROL JOINTS AT MAX. 10' O.C. IN SLABS. EDGE OF CONC PAVING ON GRADE
N.T.S. D	J	



ENGINEER'S REPORT

### **TACO BELL – STATE ST**

STATE ST CITY OF WATERTOWN JEFFERSON COUNTY, NY

> Hospitality Syracuse 290 Elwood Davis Road Suite 320 Liverpool, NY 13088

> > Prepared by:

## **APD ENGINEERING & ARCHITECTURE**

BRINGING YOUR DESIGN TO LIFE

615 Fishers Run, Victor, New York 14564 Phone: 585.742.2222 | Fax: 585.924.4914 | Website: <u>www.apd.com</u> | eMail: <u>info@apd.com</u>

Original Date: December 19, 2022

Project No.: 22-0408



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#### **INTRODUCTION/SCOPE**

The proposed Taco Bell will be located at the existing parcels of 514, 528, and 540 State St. The total property area is  $\pm 0.84$  acres. These parcels are located on the south side of State St and on the east side of Winthrop St. The general area surrounding the site consists of commercial properties along State St with the church and residential properties approximately south of the site. The existing property consists of two vacant parking lots and the vacant Trailways building/parking lot. There are minimal pervious areas on the existing site. The proposed project will demolish the existing building and site features to construct the Taco Bell and associated parking lot, utilities, lighting, and landscaping.

#### **STORE OPERATIONS**

The proposed Taco Bell is  $\pm 2,700$  SF. The restaurant will be open from 7am until 10pm with the drive thru being open until 1am on weekdays and 2am on Friday and Saturday nights. Alcohol will not be serviced at this location. There will be approximately 25 employees at this location with a maximum of 7 per shift. A WB-62 tractor trailer will provide as-grade deliveries to the restaurant a couple of times per week. Trash generated by the restaurant will be taken out to the enclosed dumpster area located on-site. A local trash company will empty the dumpsters a couple of times per week. The dumpsters will be located to the southwest of the building, behind the rear building line. They will be enclosed with a split face CMU wall (painted to match the building) and plastic lumber gates.

#### SITE INFORMATION

The site ranges a total of 6' across the site with the southeast corner being the highest at  $\pm 498$  and the northwest corner about  $\pm 492$ . However, most of this grade is made up of steep asphalt right along State St. A majority of the remainder of the parking lot is generally 1-2% slopes. The proposed finish floor elevation is 494.95 and the proposed parking lot slopes will generally be 1-3% slopes.

There will be a sidewalk connection from the State St sidewalk to the Taco Bell sidewalk at the northwest corner of the building. A bike rack is also proposed on-site in this area.

According to the Web Soil Survey of Jefferson County, the soils encountered at the project site are listed as Ur (Urban land). Urban land consists of land so altered by earth moving or so obscured by buildings or other structures that the original soils cannot be identified. The Hydrologic Soil Group rating is assumed to be a D based on adjacent soil types and urban land use. The soil map is provided as an appendix within the full Post-Construction Stormwater Management Plan, provided under separate cover.

#### **DOMESTIC WATER AND FIRE SERVICE**

Domestic water will be provided to the site via a 1 <sup>1</sup>/<sub>2</sub>" copper service connecting to the 8" cast iron main in the Winthrop St ROW. The Taco Bell demand will be 30 gpm. The building will not

be sprinklered. A backflow report and application will be prepared per DOH standards and submitted to the City/DOH for review and approval.

Fire services are provided via a hydrant on the east side of Winthrop St at the intersection with State St (corner of the proposed Taco Bell parcel).

### SANITARY SEWER

The sanitary service to the existing Trailways building is provided via a 6" PVC sewer lateral with a cleanout near the northeast corner of the property. Taco Bell is proposing to connect their 6" sewer lateral into this existing cleanout.

There will be a proposed 1,000-gallon grease interceptor on the south side of the building and a separate sanitary lateral exiting the building. The two laterals will combine and connect to the existing Trailways cleanout on the east side of the property. The peak sanitary demand is 864 gpd.

### STORMWATER MANAGEMENT

The existing parcels are comprised of mostly impervious surfaces. Runoff from the site sheet flow to either the State St or Winthrop St storm sewer systems. The proposed project will increase green space and will therefore improve water quality and reduce the peak rate runoff from the site. The impervious area will be reduced by approximately 20%. Runoff will be collected on-site in catch basins and discharge to the State St storm sewer system.

The table below summarizes the peak flow discharge rates from the site. Refer to the Post-Construction Stormwater Management Plan (provided to the City as a separate report) for full stormwater calculations.

i car Discharge Nates at the Analysis i onit										
	Drainage	e Area 1	Drainage	e Area 2	Analysis Point					
Design	ExistingProposedConditionConditionPeakPeakDischarge(cfs)		Existing	Proposed	Existing	Proposed				
Storm			Condition	Condition	Condition	Condition				
			Peak	Peak	Peak	Peak				
			Discharge	Discharge	Discharge	Discharge				
			(cfs) (cfs)		(cfs)	(cfs)				
1-year	2.44	2.41	0.27	0.0	2.71	2.41				
Storm										
10-	4.32	4.27	0.48	0.0	4.80	4.27				
year										
Storm										
100-	6.50	6.42	0.72	0.0	7.22	6.42				
year										
Storm										

#### Peak Discharge Rates at the Analysis Point

#### TRAFFIC/SITE ACCESS

The site is located along State St which is a busy 3-lane road (one in each direction and a shared center turn lane). There are four existing curb cuts on State St and one existing curb cut on Winthrop St (all full access enter/exit). The Taco Bell project is proposing to reduce the total curb cuts and provide one full access on Winthrop St, one full access on State St (one lane entering, two lanes exiting).

The site has been designed to maximize drive thru queuing on-site. Cars will enter the drive thru lane at a single-entry point in the middle of the site. As the vehicles make their way south and then east the lane will widen and split to allow two rows of stacking. There will be two order points, a pre-pay window, and a pick-up window. The main exit lane on the east side of the building is 24' wide to allow for bypass vehicles as well as right and left turn stacking at the State St exit. There is sufficient space for a total of 17 cars to be stacked in the drive thru lanes before traffic would back up into the parking lot. This drive thru configuration also prevents vehicles from stacking along parking spaces. An additional 4 vehicles could stack toward State St before backing up in the ROW. An additional 5 vehicles could stack toward Winthrop St before backing up in the ROW (these vehicles would stack along parking spaces if this scenario occurred).

Delivery trucks (WB-62) will access the site via the main entrance on State St (west of the proposed building). The truck turn plan is provided to the City as part of the Site Plan submittal package to the Planning Board demonstrating delivery access as well as fire department access.

A traffic impact assessment has been completed by our subconsultant, GTS Consulting and the full report is available for review (provided to the City as a separate report). A Site visit was conducted in November and data collected to complete the report. This data included turning movement counts for the morning, mid-day, and evening peaks, pedestrian observations, gap analysis, traffic queuing, etc.

For a quick service restaurant, approximately 50% of the customers will come from pass-by traffic (vehicles that are already in this corridor). The remaining customers will create a minor increase in the traffic generated on State St. For this project, it is assumed there will be an increase of 20-40 cars during the peak hour. The capacity analysis indicates this will have a negligible impact to the traffic operations on State St. All traffic movements on State St are projected to operate at a Level of Service A, with the site driveways all operating at a Level of Service C or better during all three peak hours.

Based on the projected turning movements on State St at the site driveways, there are more than sufficient gaps available to accommodate the proposed development.

#### **LIGHTING**

The project is proposing to remove all existing site lighting and replace with new LED fixtures at a 25' mounting height, on 3' concrete bases. There is existing street lighting along the ROW for

both State St and Winthrop St. One of the State St lights will need to be relocated as it conflicts with the proposed site access. We will work with the City as required for this relocation.

There is an existing utility pole with a flood light attached that is located on the 514 State St parcel and directs light toward the church parking lot. This light pole will be removed as part of this project. If agreeable to the City, the applicant is willing to provide a new light pole in this area and direct the light toward the church parking lot. We will work with the City staff on a final photometric plan as necessary to address any lighting concerns.

The photometric plan is provided in the Site Plan submittal package for review. Note that this plan shows some additional spillage to the south. The full site plan set shows the anticipated light pole locations to minimize this spillage. We are working with the lighting vendor on an updated photometric plan and will provide this to the City upon completion. When the updated analysis comes back, we will provide to the City for review.

#### **LANDSCAPING**

The existing site is mostly lawn with a few trees and shrubs. All existing landscaping will be removed to facilitate construction of the proposed Taco Bell. The removal of 20% of the existing impervious area will create significant green space for this project.

Perimeter trees are provided on the southern and eastern property lines to the maximum extent practical. The proposed waterline will run in the island on the southwestern portion of the property line and therefore no trees have been shown in this area.

Interior curbed islands and trees are minimized due to the site configuration and required truck access to the site.

A total of 12 trees are proposed for the site in four different species. Landscaping along the State St ROW has been minimized due to utilities, existing street tree locations, proposed light poles, and site visibility.

The landscape plan is provided in the Site Plan submittal package for review.

### POST CONSTRUCTION STORMWATER MANAGEMENT PLAN for Taco Bell – Watertown, NY

State St City of Watertown, NY (Jefferson County)

> Hospitality Syracuse 290 Elwood Davis Road Suite 320 Liverpool, NY 13088

Prepared by:



APD ENGINEERING & ARCHITECTURE PLLC 615 Fishers Run, Victor, New York 14564 Phone: 585.742.2222 | Fax: 585.924.4914 | Website: <u>www.apd.com</u> | eMail: <u>info@apd.com</u>

Original Date: December 14, 2022



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#### **Site Description**

The Stormwater Management Plan for the proposed Taco Bell in the City of Watertown, NY (Jefferson County) is outlined in this report. The total property area is approximately  $\pm 0.84$  acres. The project consists of three parcels located at 514, 528, 540 State St. These parcels are located on the south side of State St and on the east side of Winthrop St. The general area surrounding the site consists of commercial properties along State St with the church and residential properties approximately south of the site. The existing property consists of two vacant parking lots and the vacant Trailways building/parking lot. There are minimal pervious areas on the existing site. The proposed project will demolish the existing building and site features to construct the Taco Bell and associated parking lot, utilities, lighting, and landscaping.

For purposes of this study, only the three parcels under Taco Bell control are included in this analysis. The drainage paths for the church parcel to the south will remain unchanged and a portion of their parcel will flow onto the Taco Bell parcel and eventually into the State St storm sewer system. The impervious limits on the church parcel will not change and therefore have been excluded from the calculations.

#### **Methodology**

Stormwater runoff rates discharged from the site under existing conditions provide the basis on which to compare the impacts of the proposed site improvements. Points of analysis are established where runoff exits the site to provide a fixed location at which existing and proposed stormwater quantities can be compared. The areas draining to each analysis point are delineated using topographic survey maps, USGS maps, field verifications, and grading plans.

Peak runoff rates for the design storms are based on the Rational Method of modeling runoff. Hydraflow Hydrographs Extension for AutoCAD Civil 3D 2020 computer model was used to analyze discharges from drainage areas and to the analysis point.

#### **Soils Information**

According to the Web Soil Survey of Jefferson County, the soils encountered at the project site are listed as Ur (Urban land). Urban land consists of land so altered by earth moving or so obscured by buildings or other structures that the original soils cannot be identified. The Hydrologic Soil Group rating is assumed to be a D based on adjacent soil types and urban land use. Refer to Appendix III for the site soil map.

#### **Hydrology**

#### **Existing Conditions**

The overall drainage area evaluated under existing conditions consists of  $\pm 0.84$  acres. The existing conditions can be found on the Existing Drainage Map located in Appendix I.

Drainage Area 1 consists of a majority of the vacant parking lots and the Trailways building. This area sheet flows across the parking lot in a northerly direction where it flows into the State St storm sewer system.

Drainage Area 2 consists of a small part of the western lot that sheet flows to the west into the Winthrop St storm sewer, which ultimately discharges into the State St system.

Table I summarizes the hydrologic characteristics of the existing drainage areas described above.

Drainage Area	Description	Size (ac)	Composite CN	Composite c	Tc (min.)
Area 1	Sheet flow to the State St storm sewer	±0.76	97	0.85	5*
Area 2	Sheet flow to the Winthrop St storm sewer	$\pm 0.08$	98	0.90	5*

Table I	– Existing	Conditions
I able I	LAISting	Conditions

\* The actual computed Tc was less than 5 minutes, therefore a direct user input of 5 minutes was used in Hydraflow

Table III in the Summary of Results provides the existing and proposed peak discharge rates from the site at the analysis points.

#### **Proposed Conditions**

The overall drainage area for the proposed conditions consists of  $\pm 0.84$  acres. The proposed conditions can be found on the Proposed Drainage Map located in Appendix I.

Drainage Area 1 is comprised of the entire Taco Bell parcel including the building, parking lot, and new landscape/lawn areas. A majority of the site will be collected via new on-site storm sewer and connected to the State St storm sewer system.

Table II summarizes the hydrologic characteristics of the proposed drainage areas described above.

Drainage Area	Description	Size (ac)	Composite CN	Composite c	Tc (min.)
Area 1	Entire parcel to the State St storm sewer	±0.84	94	0.76	5*

**Table II – Proposed Conditions** 

\* The actual computed Tc was less than 5 minutes, therefore a direct user input of 5 minutes was used in Hydraflow

Table III in the Summary of Results provides the existing and proposed peak discharge rates from the site at the analysis points.

#### **SPDES Phase II Requirements**

The overall project disturbance will be under 1-acre, thus a NYSDEC SPDES General Permit for Construction Activities is not needed. This project was designed to prevent overbank flooding and help control extreme floods.

#### **Peak Rate Control**

The drainage patterns for this project have generally been maintained. Under existing conditions, a portion of the site discharged to the Winthrop St storm sewer system and will not be directed to the State St storm sewer system. Both of these storm sewers combine at the Analysis Point as labeled on the Drainage Maps. Due to the increase in green space for the site, the peak flow rate for the storm events up to an including the 100-year, 24-hour storm have been decreased for both storm sewer systems. Refer to Table III and Appendix II for additional information.

#### **Summary of Results**

Table III depicts the existing and proposed peak discharge rates from the site at the analysis points. The rainfall rates used in Hydraflow were provided from NOAA's Precipitation Frequency Data Servers.

The table shows the comparison of the peak discharge for both Drainage Area 1 and Drainage Area 2, as well as the comparison for the site as a whole at the Analysis Point.

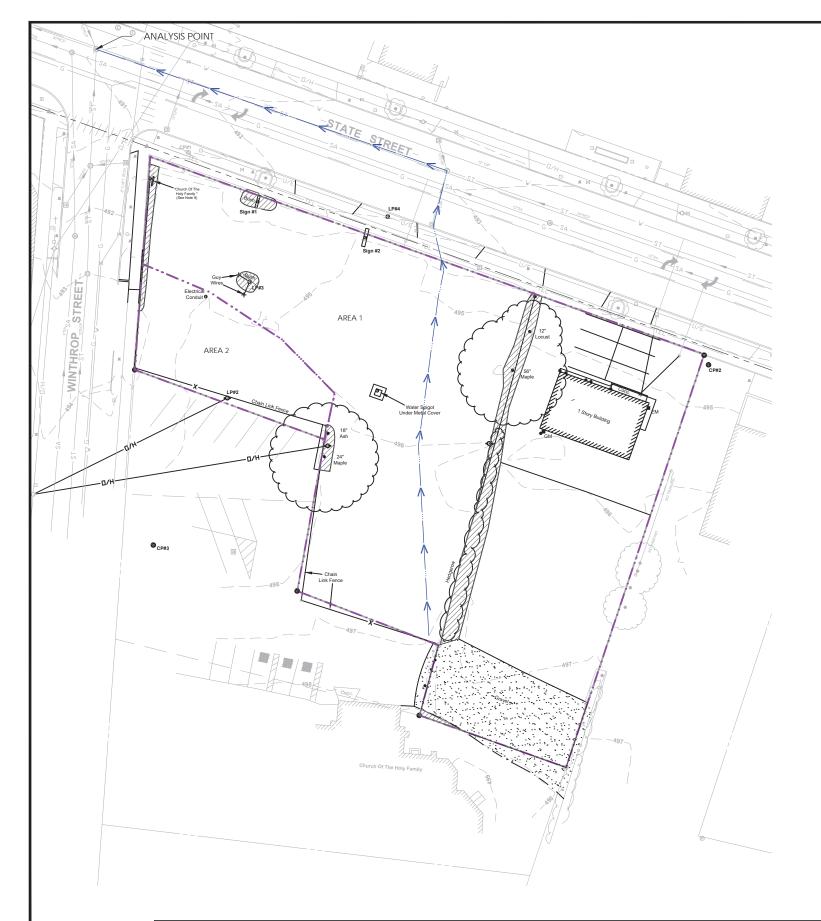
	Drainage	e Area 1	Drainage	e Area 2	Analysis Point		
Design Storm	ExistingProposedConditionCondition		Existing Condition	Proposed Condition	Existing Condition	Proposed Condition	
	Peak Discharge (cfs)	Peak Discharge (cfs)	Peak Discharge	Peak Discharge	•	Peak Discharge (cfs)	
1-year Storm	2.44	2.41	(cfs) 0.27	(cfs) 0.0	(cfs) 2.71	2.41	
10- year Storm	4.32	4.27	0.48	0.0	4.80	4.27	
100- year Storm	6.50	6.42	0.72	0.0	7.22	6.42	

Table III - Peak Discharge Rates at the Analysis Point

As can be seen from the tables above, the peak discharge from the site for each of the design storms will be reduced by 11% at the Analysis Point.

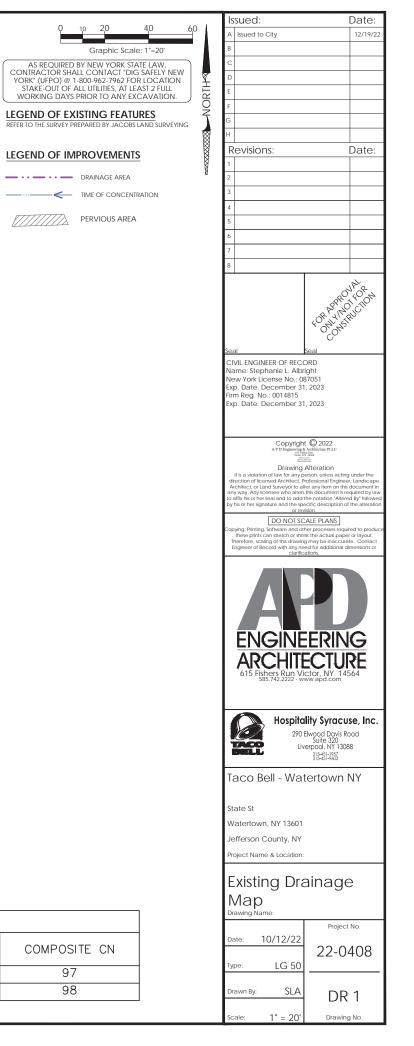
### **APPENDIX I**

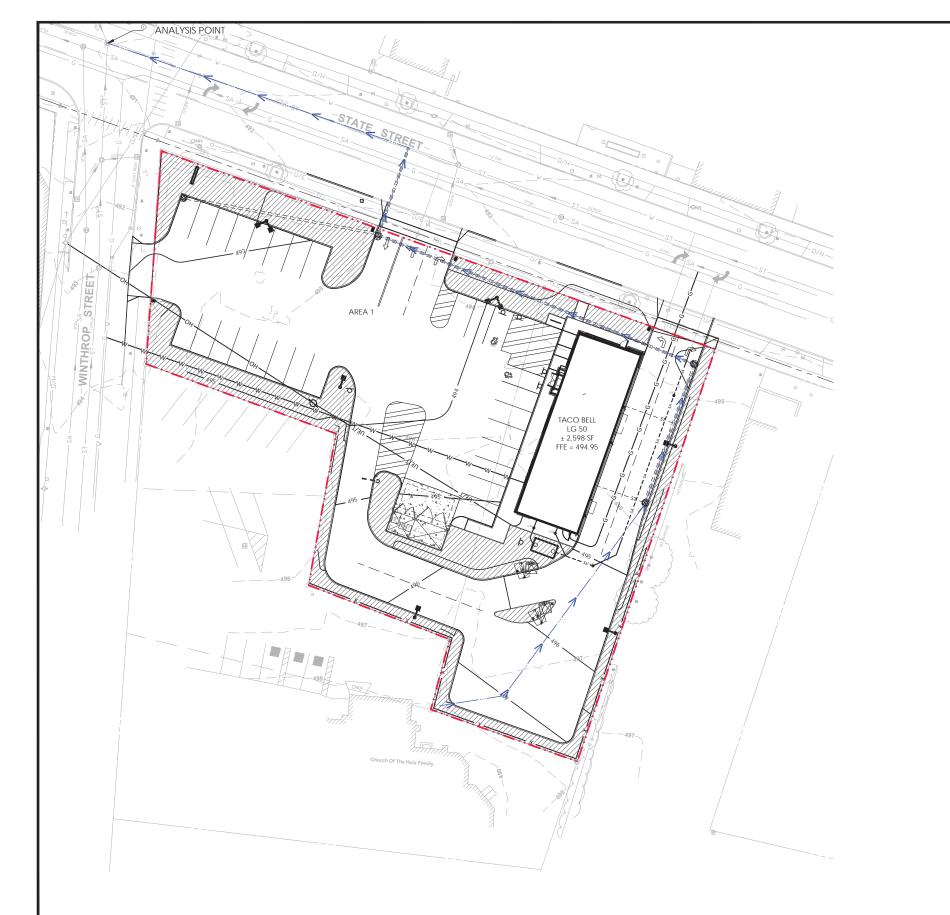
### Existing Drainage Conditions Plan (DR 1) and Proposed Drainage Conditions Plan (DR 2)



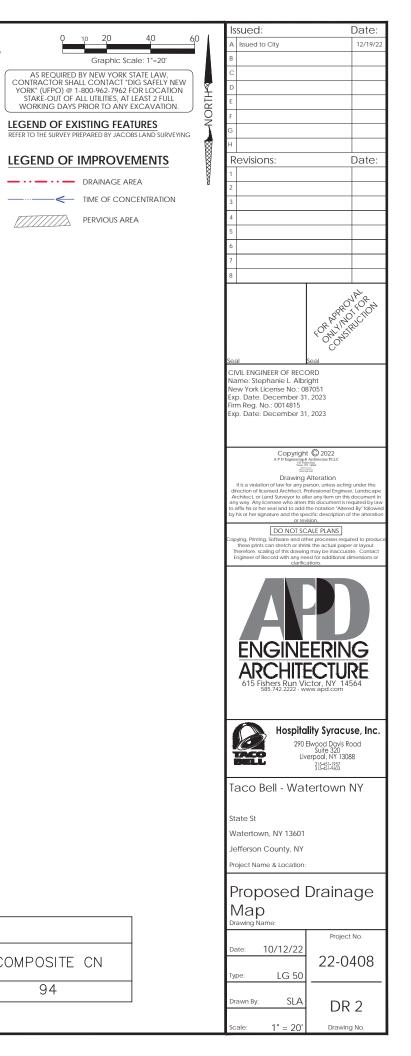
EXISTING CONDITIONS									
	IMPERVIOUS AREA (PAVED) (SF)	CN	IMPERVIOUS (GRAVEL) AREA (SF)	CN	PERVIOUS AREA (SF)	CN	TOTAL AREA (SF)	TOTAL AREA (AC)	
AREA 1	29034	98	2459	91	1531	80	33023	0.76	
AREA 2	3613	98	0	91	47	80	3660	0.08	
					·				

REFERENCE: 1. SV 1 OF 1, PRELIMINARY SURVEY LAST REVISED ON OCTOBER 4, 2022, PREPARED BY JACOBS LAND SURVEYING





PROPOSED CONDITIONS								
	IMPERVIOUS AREA (SF)	CN	PERVIOUS AREA (SF)	CN	TOTAL AREA (SF)	TOTAL AREA (AC)	CC	
AREA 1	28081	98	8545	80	36626	0.84		



REFERENCE: 1. SV 1 OF 1, PRELIMINARY SURVEY LAST REVISED ON OCTOBER 4, 2022, PREPARED BY JACOBS LAND SURVEYING

### **APPENDIX II**

### Hydraflow Hydrographs Output for Existing and Proposed Conditions

### Hydraflow Table Oph Contentsell - Watertown - Winthrop and State St\Reports\Stormwater\Hydraflow Analysis.gpw

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4	Wednesday, 12 / 14 / 2022

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# Hydrograph Return Period Recap Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

		h Inflow Peak Outflow (cfs) hyd(s)						Peak Outflow (cfs) Hydrograph		Hydrograph Description	
	(origin)		1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
	Rational		2.439				4.320			6.500	Existing Area 1
2	Rational		0.272				0.481			0.724	Existing Area 2
3	Combine	1, 2	2.711				4.801			7.224	Total Existing Dischage
5	Rational		2.411				4.269			6.424	Proposed Area 1

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## Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	2.439	1	5	732				Existing Area 1
2	Rational	0.272	1	5	82				Existing Area 2
3	Combine	2.711	1	5	813	1, 2			Total Existing Dischage
5	Rational	2.411	1	5	723				Proposed Area 1

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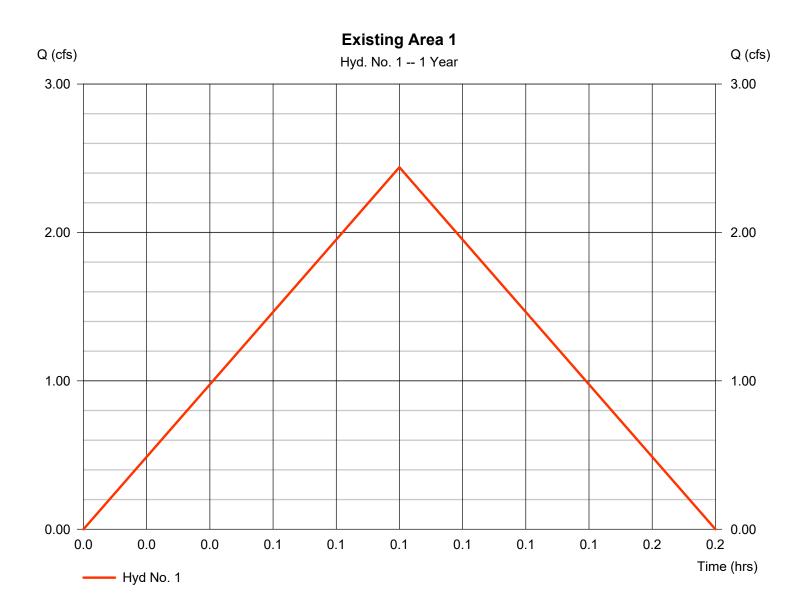
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 1

**Existing Area 1** 

Hydrograph type	= Rational	Peak discharge	= 2.439 cfs
Storm frequency	= 1 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 732 cuft
Drainage area	= 0.760 ac	Runoff coeff.	= 0.85*
Intensity	= 3.776 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.060 x 0.55) + (0.670 x 0.90) + (0.030 x 0.30)] / 0.760



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Wednesday, 12 / 14 / 2022

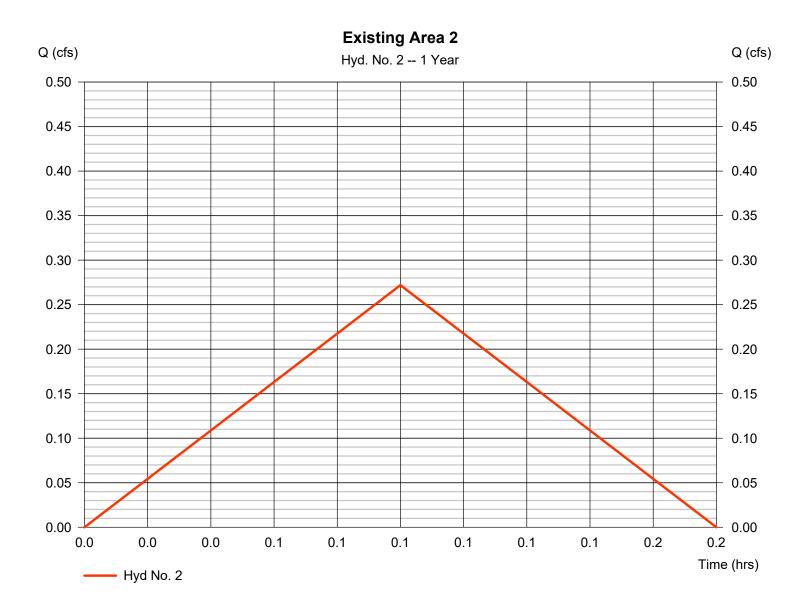
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 2

**Existing Area 2** 

Hydrograph type	= Rational	Peak discharge	= 0.272 cfs
Storm frequency	= 1 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 82 cuft
Drainage area	= 0.080 ac	Runoff coeff.	= 0.9*
Intensity	= 3.776 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.080 x 0.90)] / 0.080

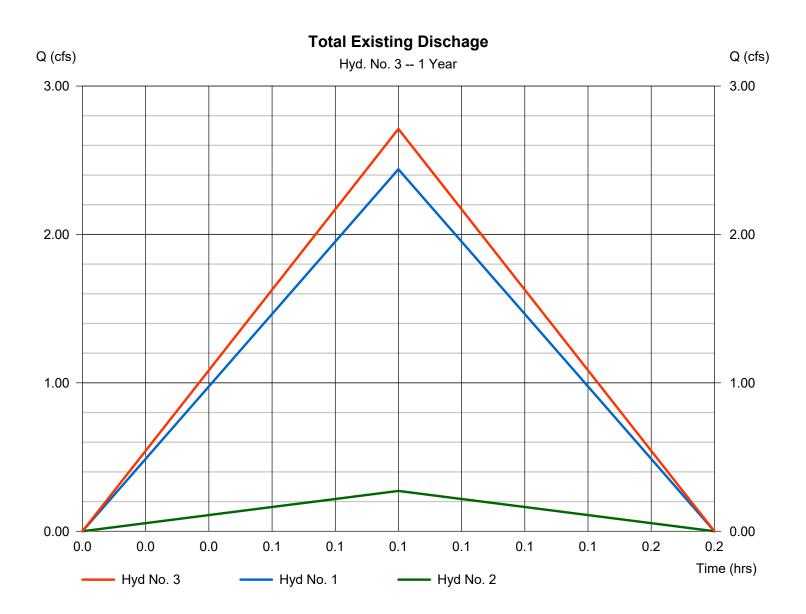


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 3

**Total Existing Dischage** 

Hydrograph type	= Combine	Peak discharge	= 2.711 cfs
Storm frequency	= 1 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 813 cuft
Inflow hyds.	= 1.2	Contrib. drain. area	= 0.840 ac
millow myus.	- 1, 2	Contrib. drain. area	- 0.040 ac



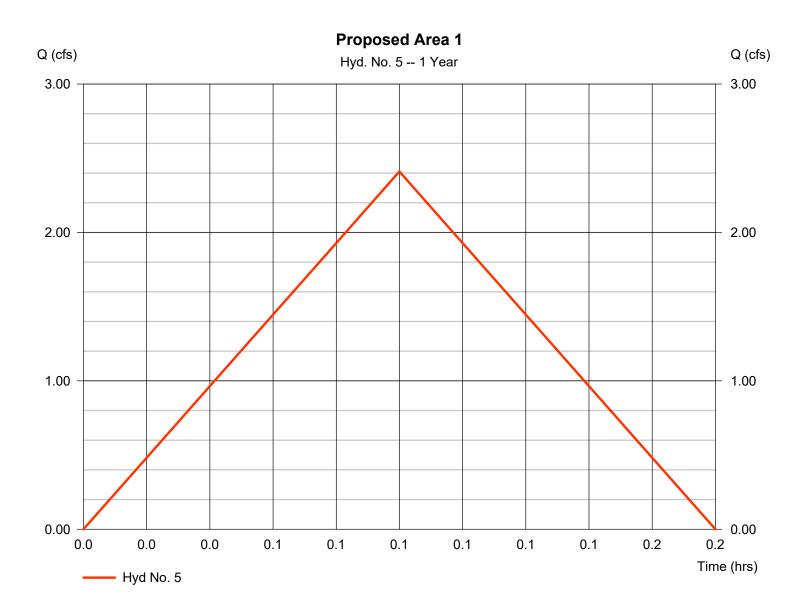
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 5

Proposed Area 1

Hydrograph type	= Rational	Peak discharge	= 2.411 cfs
Storm frequency	= 1 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 723 cuft
Drainage area	= 0.840 ac	Runoff coeff.	= 0.76*
Intensity	= 3.776 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.640 x 0.90) + (0.200 x 0.30)] / 0.840



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## Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	4.320	1	5	1,296				Existing Area 1
2	Rational	0.481	1	5	144				Existing Area 2
3	Combine	4.801	1	5	1,440	1, 2			Total Existing Dischage
5	Rational	4.269	1	5	1,281				Proposed Area 1

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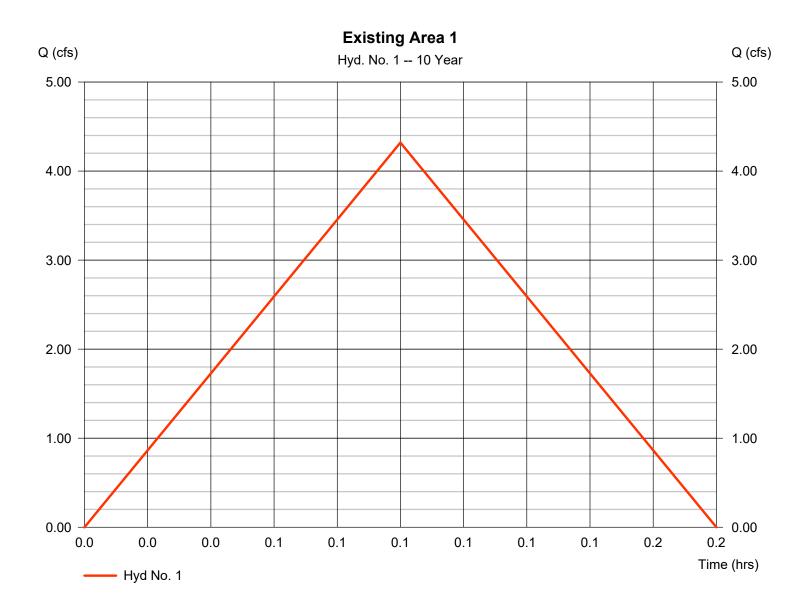
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 1

**Existing Area 1** 

Hydrograph type	= Rational	Peak discharge	= 4.320 cfs
Storm frequency	= 10 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 1,296 cuft
Drainage area	= 0.760 ac	Runoff coeff.	= 0.85*
Intensity	= 6.687 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.060 x 0.55) + (0.670 x 0.90) + (0.030 x 0.30)] / 0.760



8

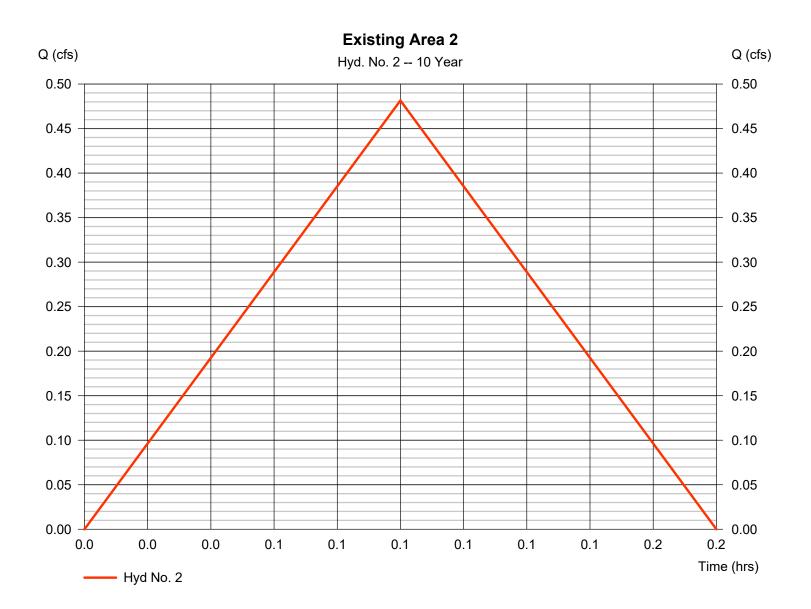
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 2

**Existing Area 2** 

Hydrograph type	= Rational	Peak discharge	= 0.481 cfs
Storm frequency	= 10 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 144 cuft
Drainage area	= 0.080 ac	Runoff coeff.	= 0.9*
Intensity	= 6.687 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.080 x 0.90)] / 0.080

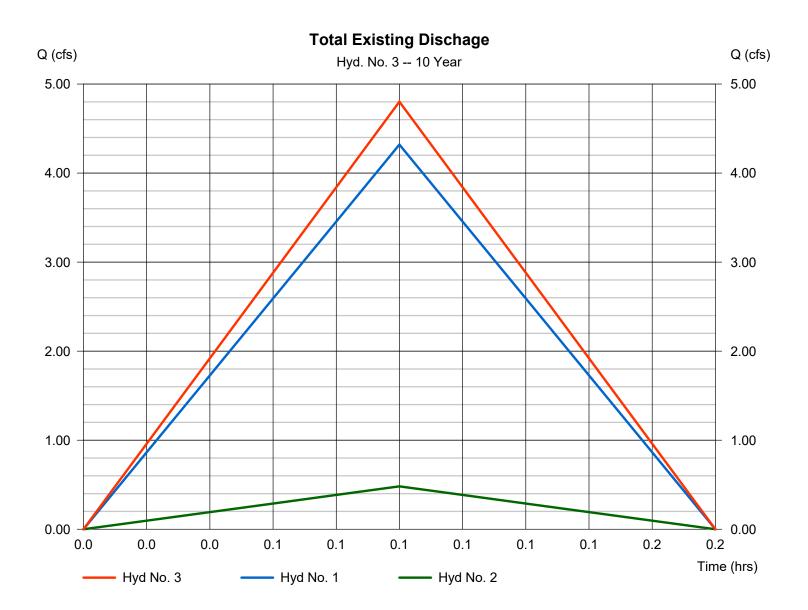


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 3

**Total Existing Dischage** 

Hydrograph type	<ul> <li>= Combine</li> <li>= 10 yrs</li> <li>= 1 min</li> <li>= 1, 2</li> </ul>	Peak discharge	= 4.801 cfs
Storm frequency		Time to peak	= 0.08 hrs
Time interval		Hyd. volume	= 1,440 cuft
Inflow hyds.		Contrib. drain. area	= 0.840 ac
innow nyda:	- 1, <b>Z</b>		- 0.040 ac



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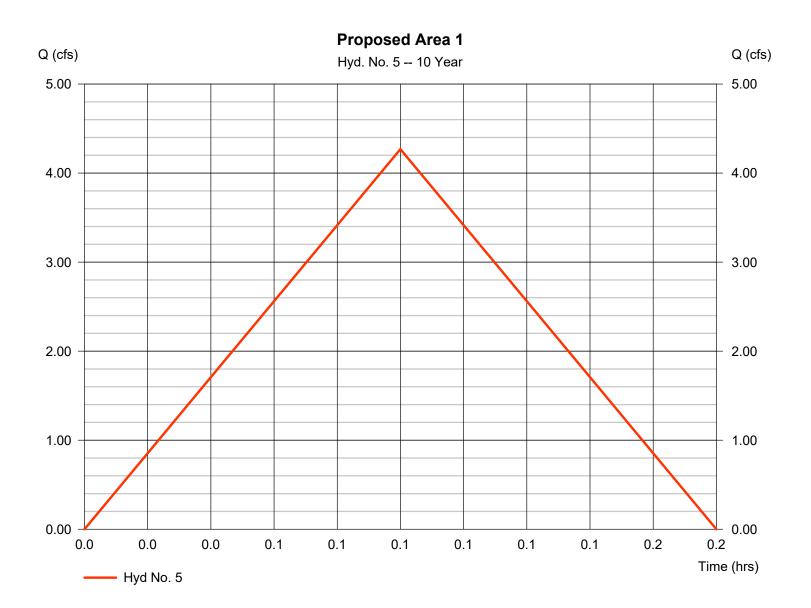
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 5

Proposed Area 1

Hydrograph type	= Rational	Peak discharge	= 4.269 cfs
Storm frequency	= 10 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 1,281 cuft
Drainage area	= 0.840 ac	Runoff coeff.	= 0.76*
Intensity	= 6.687 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.640 x 0.90) + (0.200 x 0.30)] / 0.840



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## Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Rational Rational Combine Rational	6.500 0.724 7.224 6.424	1 1 1 1	5 5 5	1,950 217 2,167 1,927	1, 2		 Existing Area 1 Existing Area 2 Total Existing Dischage Proposed Area 1
ombine	7.224	1	5	2,167	1, 2		 Total Existing Dischage
lational	6.424	1	5	1,927			 Proposed Area 1
						1	

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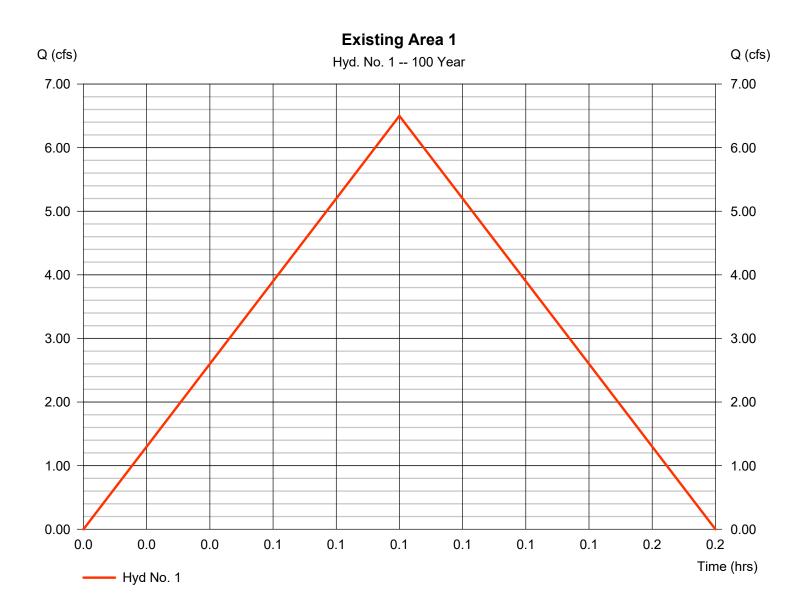
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 1

**Existing Area 1** 

Hydrograph type Storm frequency	= Rational = 100 yrs = 1 min	Peak discharge Time to peak	= 6.500 cfs = 0.08 hrs = 1.050 cuft
Time interval	= 1 min	Hyd. volume	= 1,950 cuft
Drainage area	= 0.760 ac	Runoff coeff.	= 0.85*
Intensity	= 10.062 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.060 x 0.55) + (0.670 x 0.90) + (0.030 x 0.30)] / 0.760



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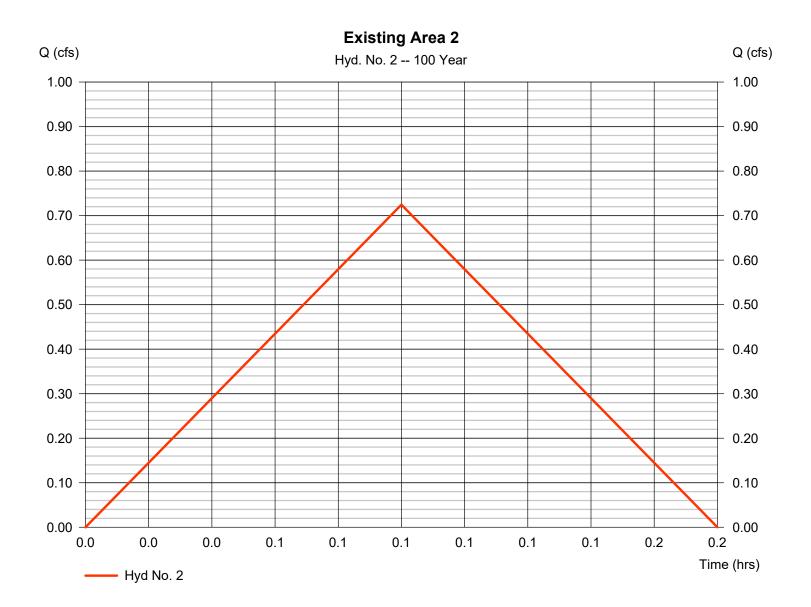
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 2

**Existing Area 2** 

Hydrograph type	= Rational	Peak discharge	= 0.724 cfs
Storm frequency	= 100 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 217 cuft
Drainage area	= 0.080 ac	Runoff coeff.	= 0.9*
Intensity	= 10.062 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.080 x 0.90)] / 0.080



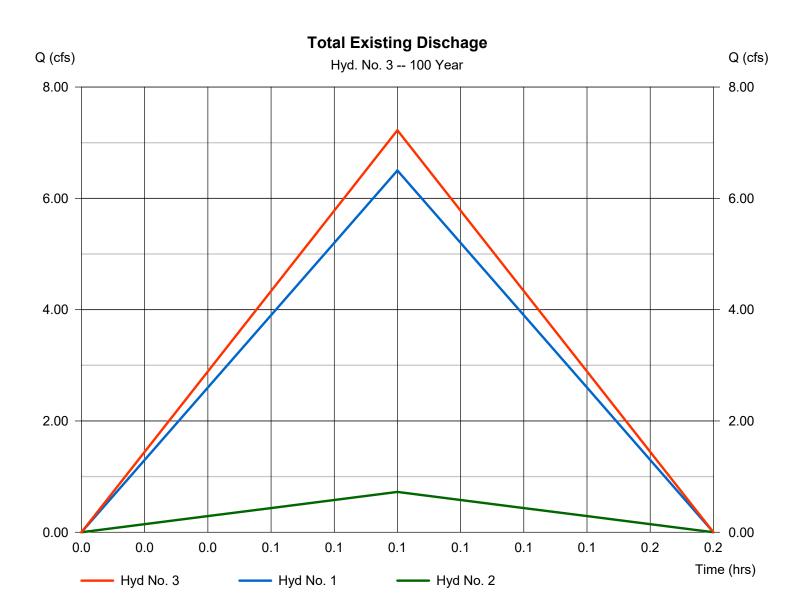
14

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 3

**Total Existing Dischage** 

Hydrograph type= CombinStorm frequency= 100 yrsTime interval= 1 minInflow hyds.= 1, 2	5	= 7.224 cfs = 0.08 hrs = 2,167 cuft = 0.840 ac
11110W Hyds. = 1, 2		- 0.040 ac



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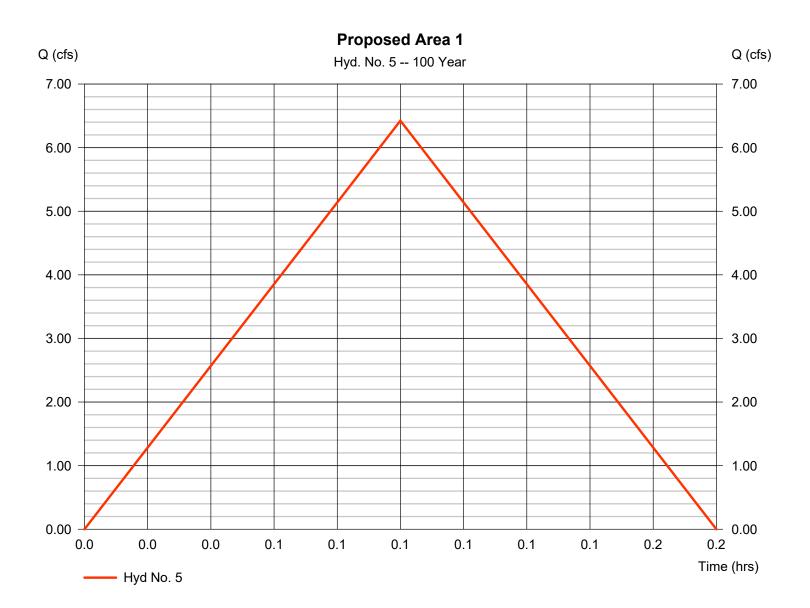
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 5

Proposed Area 1

Hydrograph type	= Rational	Peak discharge	= 6.424 cfs
Storm frequency	= 100 yrs	Time to peak	= 0.08 hrs
Time interval	= 1 min	Hyd. volume	= 1,927 cuft
Drainage area	= 0.840 ac	Runoff coeff.	= 0.76*
Intensity	= 10.062 in/hr	Tc by User	= 5.00 min
IDF Curve	= 22-0408 - IDF Curve.IDF	Asc/Rec limb fact	= 1/1

\* Composite (Area/C) = [(0.640 x 0.90) + (0.200 x 0.30)] / 0.840



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# **Hydraflow Rainfall Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	20.5650	4.1000	0.7675						
2	25.2512	4.2000	0.7758						
3	0.0000	0.0000	0.0000						
5	0.0000	0.0000	0.0000						
10	36.8237	4.1000	0.7725						
25	0.0000	0.0000	0.0000						
50	0.0000	0.0000	0.0000						
100	56.9052	4.3000	0.7770						

File name: 22-0408 - IDF Curve.IDF

#### Intensity = B / (Tc + D)^E

Return	<b>3</b> ( )											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	3.78	2.70	2.14	1.79	1.55	1.37	1.23	1.12	1.04	0.96	0.90	0.84
2	4.51	3.22	2.55	2.13	1.84	1.63	1.47	1.34	1.23	1.14	1.06	1.00
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
10	6.69	4.77	3.77	3.15	2.72	2.41	2.17	1.98	1.82	1.69	1.58	1.48
25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
100	10.06	7.20	5.71	4.77	4.13	3.65	3.28	2.99	2.75	2.55	2.39	2.24
						[						

Tc = time in minutes. Values may exceed 60.

cip. file nar	ne: M:\2022 J	lobs\408 - HF	G - Taco Bell	- Watertown ·	Winthrop an	d State St\Re	ports\Stormwater	\blank.pcp
-								

	Rainfall Precipitation Table (in)								
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
SCS 24-hour	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	

## **APPENDIX III**

**Soils Data** 



United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Jefferson County, New York



# Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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# **How Soil Surveys Are Made**

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

### Custom Soil Resource Report Soil Map



	MAP L	EGEND		MAP INFORMATION
Area of Int	terest (AOI) Area of Interest (AOI)	8	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:15,800.
Soils	Soil Map Unit Polygons	00 V	Very Stony Spot Wet Spot	Warning: Soil Map may not be valid at this scale.
~	Soil Map Unit Lines Soil Map Unit Points	v ∆	Other	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil
•	Point Features Blowout	 Water Fea	Special Line Features	line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.
0 2	Borrow Pit	~~ Transport	Streams and Canals	Please rely on the bar scale on each map sheet for map
<b>≍</b>	Clay Spot Closed Depression	+++	Rails Interstate Highways	measurements.
X	Gravel Pit	~	US Routes	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
.: ©	Gravelly Spot Landfill	~	Major Roads Local Roads	Maps from the Web Soil Survey are based on the Web Mercator
۸ بینه ج	Lava Flow Marsh or swamp Mine or Quarry	Backgrou	nd Aerial Photography	projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
0	Miscellaneous Water Perennial Water			This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
× +	Rock Outcrop Saline Spot			Soil Survey Area: Jefferson County, New York Survey Area Data: Version 22, Sep 10, 2022
:: =	Sandy Spot Severely Eroded Spot			Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.
♦	Sinkhole Slide or Slip			Date(s) aerial images were photographed: Jul 19, 2020—Nov 5, 2020
ø	Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
CnB	Collamer silt loam, 3 to 8 percent slopes	2.6	12.6%
NoA	Niagara silt loam, 0 to 3 percent slopes	3.0	14.6%
Ur	Urban land	14.9	72.9%
Totals for Area of Interest		20.5	100.0%

# Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The

delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

# Jefferson County, New York

### CnB—Collamer silt loam, 3 to 8 percent slopes

### **Map Unit Setting**

National map unit symbol: 9smx Elevation: 250 to 1,080 feet Mean annual precipitation: 33 to 50 inches Mean annual air temperature: 45 to 46 degrees F Frost-free period: 110 to 170 days Farmland classification: All areas are prime farmland

### **Map Unit Composition**

Collamer and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

### **Description of Collamer**

### Setting

Landform: Lake plains Landform position (two-dimensional): Summit Landform position (three-dimensional): Tread Down-slope shape: Concave Across-slope shape: Convex Parent material: Silty and clayey glaciolacustrine deposits

### **Typical profile**

H1 - 0 to 8 inches: silt loam
H2 - 8 to 18 inches: silt loam
H3 - 18 to 32 inches: silty clay loam
H4 - 32 to 60 inches: stratified silt loam to very fine sand to clay

### **Properties and qualities**

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.57 in/hr)
Depth to water table: About 18 to 24 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 15 percent
Available water supply, 0 to 60 inches: High (about 10.1 inches)

### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: C/D Ecological site: F101XY009NY - Moist Lake Plain Hydric soil rating: No

### **Minor Components**

Unnamed soils, clayey surface texture and sandy areas Percent of map unit: 10 percent Hydric soil rating: No

#### Niagara

Percent of map unit: 8 percent Hydric soil rating: No

### Canandaigua

Percent of map unit: 2 percent Landform: Depressions Hydric soil rating: Yes

### NoA—Niagara silt loam, 0 to 3 percent slopes

#### Map Unit Setting

National map unit symbol: 9sqx Elevation: 250 to 930 feet Mean annual precipitation: 33 to 50 inches Mean annual air temperature: 45 to 46 degrees F Frost-free period: 110 to 170 days Farmland classification: Prime farmland if drained

### Map Unit Composition

Niagara and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

### **Description of Niagara**

### Setting

Landform: Lake plains Landform position (two-dimensional): Footslope Landform position (three-dimensional): Tread Down-slope shape: Concave Across-slope shape: Linear Parent material: Silty and clayey glaciolacustrine deposits

### **Typical profile**

*H1 - 0 to 13 inches:* silt loam *H2 - 13 to 35 inches:* silt loam *H3 - 35 to 75 inches:* silt loam

### **Properties and qualities**

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.57 in/hr)
Depth to water table: About 6 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 15 percent

Available water supply, 0 to 60 inches: High (about 10.5 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3w Hydrologic Soil Group: C/D Ecological site: F142XB018VT - Moist Lake Plain Hydric soil rating: No

#### **Minor Components**

### Guffin

Percent of map unit: 5 percent Landform: Depressions Hydric soil rating: Yes

### Collamer

Percent of map unit: 5 percent Hydric soil rating: No

### Canandaigua

Percent of map unit: 5 percent Landform: Depressions Hydric soil rating: Yes

# Ur—Urban land

### Map Unit Setting

National map unit symbol: 9srz Mean annual precipitation: 33 to 50 inches Mean annual air temperature: 45 to 46 degrees F Frost-free period: 110 to 170 days Farmland classification: Not prime farmland

### Map Unit Composition

*Urban land:* 90 percent *Minor components:* 10 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

### **Minor Components**

### Udorthents, smoothed

Percent of map unit: 10 percent Landform: Depressions Hydric soil rating: No

# Soil Information for All Uses

# **Soil Properties and Qualities**

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

# **Soil Qualities and Features**

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

# Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

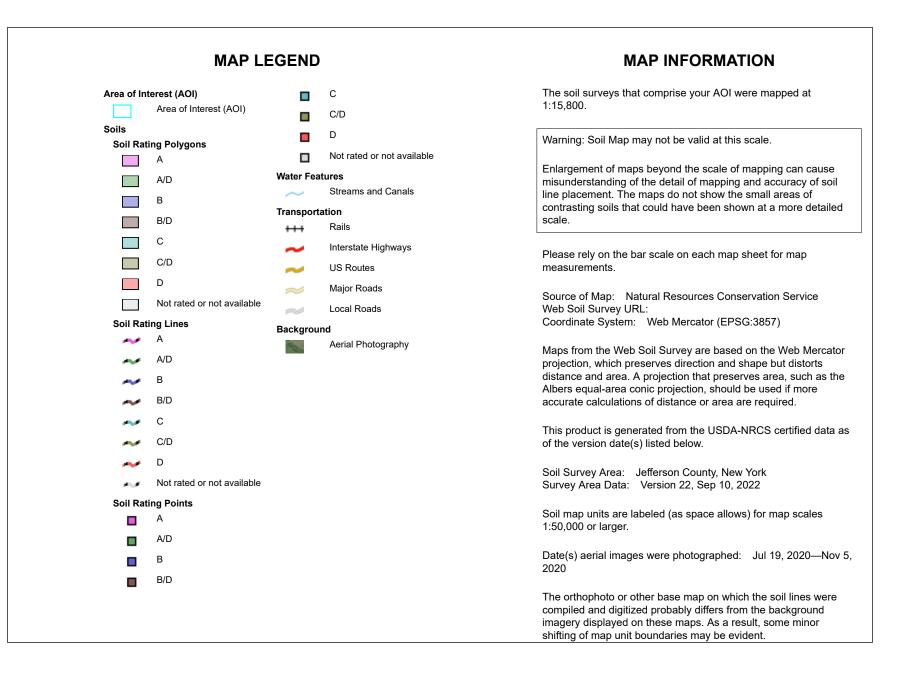
Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.





# Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
CnB	Collamer silt loam, 3 to 8 percent slopes	C/D	2.6	12.6%
NoA	Niagara silt loam, 0 to 3 percent slopes	C/D	3.0	14.6%
Ur	Urban land		14.9	72.9%
Totals for Area of Interes	st	20.5	100.0%	

# Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

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November 16, 2022

APD Engineering & Architecture, PLLC 615 Fishers Run Victor, NY 14564

Attn: Ms. Stephanie Albright, P.E.

# Re: Traffic Impact Assessment – Proposed Taco Bell Development State Street – Watertown, NY

Dear Ms. Albright:

I have completed my review of traffic operations associated with the proposed Taco Bell development located on the southeast corner of the intersection of State Street with Winthrop Street in Watertown, NY. This letter summarizes the work completed in this review as well as my findings.

### Project Understanding

The proposed development is located on the southeast corner of the intersection of State Street with Winthrop Street in the City of Watertown. The site is currently vacant with a small vacant Trailways building on the northeast corner which will be removed. The proposed development includes a 2,598 SF Taco Bell restaurant with drive through operations. Access to the development is proposed via one full access driveway to State Street located 150 feet to the east of Winthrop Street, a drive through exit only access driveway to State Street located approximately 280 feet to the east of Winthrop Street and a full access driveway to Winthrop Street located approximately 90 feet south of State Street.

A site plan developed by APD Engineering & Architecture, dated October 12<sup>th</sup>, 2022 has been attached.

### Data Collection

Site visits were conducted on Wednesday – November 9<sup>th</sup>, 2022 to collect the following:

- <u>Existing Traffic Volume Counts</u> Traffic turning movement counts were collected at the intersection of State Street with Winthrop Street during the weekday morning (7-9am), midday (11am-1pm), and evening (4-6pm) peak travel periods to ensure that actual peak hours of the adjacent streets were captured. Separate heavy vehicles counts were collected by approach. There were minor pedestrian volumes observed during the traffic count periods and all area schools were in session.
- <u>State Street Gap Data</u> Gap data was collected to assess the ability for vehicles to turn in and out of the proposed site driveways on State Street. In order for a vehicle to turn right out of the site, or left into the site, the vehicle only requires a gap in the eastbound direction on State Street. A vehicle requires a gap in traffic in both directions at the same time to turn left out of



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# Re: Traffic Impact Assessment – Proposed Taco Bell Development State Street – Watertown, NY

the site onto State Street. These gaps in traffic were observed and timed on State Street at the locations of the proposed full access driveway during the weekday morning, weekday midday, and weekday evening traffic count periods. The gaps were then converted to a number of vehicles that could turn left or right out of the proposed site during each gap and then totaled for the peak hour. For example, one vehicle can turn from the driveway with a 6-9 second gap in traffic, two can turn with a 10-13 second gap, 3 with a 14-17 second gap, 4 with an 18-19 second gap, etc.

- <u>Traffic Queue Data</u> Traffic queues in the westbound direction on State Street at the Mechanic Street signalized intersection, and in the eastbound direction on State Street at the High Street/William Street signalized intersection, were observed and recorded at the beginning of each green phase throughout the traffic count periods in order to identify average and maximum traffic queues, and any impacts they may have on access to the site.
- <u>Spot Speed Measurements</u> 50 speed measurements were collected in each direction on State Street to identify average and 85<sup>th</sup> percentile operating speeds in the area passing the site driveways. The data was collected for free flow traffic during off-peak times. The weather was clear and the roadway was dry.
- <u>Sight Distance Measurements</u> Sight lines looking east/west along State Street from the proposed site driveways were collected for comparison to design standards in order to confirm that adequate sight lines are available for safe ingress and egress from the site.
- <u>Operational Data</u> Other data needed to evaluate traffic operations, such as intersection geometry, control, and speeds limits were also collected.

# Existing Operations

State Street has one through lane in each direction with a two way center left turn lane passing the site driveways. The center lane provides stopping space for westbound vehicles turning left on Winthrop Street or into the proposed site driveway, as well as stopping space for eastbound vehicles turning left onto High Street. Winthrop Street is a one lane southbound only roadway traveling away from State Street.

Based on the traffic counts collected, the peak hours were identified as follows:

Morning Peak Hour – 7:15am to 8:15am Midday Peak Hour – 12:00pm-1:00pm Evening Peak Hour – 4:30pm to 5:30pm



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## Re: Traffic Impact Assessment – Proposed Taco Bell Development State Street – Watertown, NY

The 2022 existing traffic volumes collected in November are shown in the attached Figure 1 for the morning, midday, and evening peak hours.

The existing traffic counts were reviewed and compared to historical traffic volume data on State Street to identify any necessary adjustments to account for seasonal adjustments or lingering impacts from the Covid pandemic. Specifically, the 2019 AADT traffic count for State Street between Washington Street and Park Drive was taken from the NYSDOT Traffic Data Viewer website was reviewed. Based on the 2019 data, State Street has a design hour volume of 1,036 vehicles through the study area with a directional design hour volume of 557 vehicles. The 2022 traffic volumes collected in November have evening peak hour volume of 1,086 vehicles passing the site with a directional peak volume of 566 vehicles. Since the 2022 traffic counts are 5% higher than the overall historical traffic count and 2% higher in the peak direction, and since the seasonal adjustment factor for November (Factor Group 30 - 1.018) would actually decrease the volumes slightly, there were no adjustments made.

State Street carries approximately 297 vehicles eastbound/512 vehicles westbound passing the site during the morning peak hour, 493 vehicles eastbound/493 vehicles westbound passing the site during the midday peak hour and 566 vehicles eastbound/520 vehicles westbound passing the site during the evening peak hour.

Based on the gap data collected, there were sufficient gaps in traffic observed to accommodate approximately 471 vehicles turning right onto State Street from the site driveways during the morning peak hour, 368 vehicles turning right during the midday peak hour, and 294 vehicles turning right during the evening peak hour. These gaps would also be available for vehicles turning left into the site from State Street. There were sufficient gaps observed to accommodate approximately 272 vehicles turning left out of the site onto State Street during the morning peak hour, 241 vehicles turning left out during the midday peak hour, and 163 vehicles turning left out during the evening peak hour. There are no concerns with available gaps in traffic to accommodate traffic movements entering and exiting the site. The gap data is attached.

The average traffic queues in the eastbound left turn lane on State Street at High Street were less than 1 vehicle during all three peak hours, with maximum queues of 2-3 vehicles observed during each peak hour. These queues will not interfere with access at the site driveway. The eastbound through/right lane at High Street had average traffic queues of 2.6 vehicles during the morning peak hour, 4.5 vehicles during the midday peak hour, and 6.9 vehicles during the evening peak hour. The maximum queues observed in the eastbound through/right lanes were 9 vehicles during the morning peak hour, 14 vehicles during the midday peak hour and 17 vehicles during the evening peak hour. With 150 feet of storage space between the High Street signal and the exit driveway from the drive through, there is sufficient storage for up to 6-7 vehicles without impacting access from the site. The maximum queues observed temporarily blocked the driveway for 6 out 53 signal cycles during the morning peak hour and 15 out of 55 signal cycles during the midday peak hour, and 24 out of 52 cycle during the evening



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peak hour. Any blockages quickly clear during the green phase of the signal. Any delays exiting the site would be internal to the site and will not impact traffic operations along State Street.

The average traffic queues on State Street westbound at Mechanic Street were 2-3 vehicles during each peak hour with maximum queues observed of 6-8 vehicles. The maximum queues did not reach Winthrop Street during any of the three peak hours.

The posted speed limit passing the site is 30 mph on State Street. The speed data collected indicates that the average speeds passing the site on State Street are 32 mph eastbound and 34 mph westbound. The 85<sup>th</sup> percentile speeds based on the data collected are 34.5 mph eastbound and 37.5 mph westbound on State Street. The speed data has been attached.

The following table provides a summary of the recommended sight distances along State Street from the AASHTO <u>A Policy on Design of Highways and Streets</u> as well as the available sight distances based on field measurements. The speed limit in the area is 30 mph on State Street, however the speed data collected indicates that the operating speed is closer to 35-40 mph in both directions. Therefore 40 mph was used for the sight distance review. The recommended sight distance for left turning vehicles was adjusted to account for the two center lane to be crossed when exiting the site

# **Sight Distance Summary**

			AASHTO	
	Operating		Recommended	Available
Location	Speed	Direction	Sight Distance	Sight Distance
West Full Access Site Driveway @	40 mph	Looking Left	470 feet	700+ feet
State Street – Turning Left	40 mpn	Looking Right	470 feet	1,000+ feet
West Full Access Site Driveway @	40 mph	Looking Left	385 feet	700+ feet
State Street – Turning Right	40 mpn	Looking Leit	363 1001	700+ Ieei
East Exit Only Site Driveway @	40 mph	Looking Left	470 feet	800+ feet
State Street – Turning Left	40 mph	Looking Right	470 feet	1,000+ feet
East Exit Only Site Driveway @	40 mph	Looking Loft	385 feet	800+ feet
State Street – Turning Right	40 mph	Looking Left	385 Teet	ouu+ leet

There are more than adequate sight distances available looking in both directions along State Street from the proposed access locations based on the observed operating speeds of 35-40 mph. There are no concerns with sight distances and safety for ingress and egress from the proposed site driveways.

Capacity analysis of the existing traffic operations was completed using Synchro11, an industry accepted standard for the analysis of both signalized and unsignalized intersections that is based on methodologies developed in the Highway Capacity Manual. Intersection and individual movement operations are graded in terms of Level of Service ranging from A to F, as described in the HCM. For



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example, an unsignalized intersection movement with an average delay of 5 seconds per vehicle is considered a Level of Service A while an average delay per vehicle of 20 seconds is considered a C. A Level of Service D or better is generally considered acceptable for a signalized intersection while a Level of Service E or better is generally considered acceptable for an unsignalized intersection.

It is noted that with Winthrop Street being a southbound only roadway away from State Street, Synchro does not provide a printable delay report since there is no side street traffic approaching the intersection. The program does however show projected delays for the westbound left turning traffic onto Winthrop Street under each scenario. These values were taken from the programs and included in the attached Level of Service summary.

The results of the Synchro capacity analysis indicates that the left turn movement from State Street onto Winthrop Street is operating at Level of Service A during all three peak hours with no delays on the free flow east-west through movements. The detailed Level of Service summary has been attached.

There are no noted concerns with existing traffic operations on State Street in the vicinity of the project site as there are ample gaps in traffic and adequate sight lines in both directions. There are acceptable delays at the adjacent Winthrop Street intersection with acceptable Level of Service A for all traffic movements and minimal queuing concerns with relation to the proposed driveway locations and the adjacent traffic signals to the east and west. These findings are consistent with observations made during the data collection.

# Accident Analysis

An accident analysis was completed for State Street between Mechanic Street and High Street/William Street using history reports obtained for a three year period from June 2019 through May 2022. Over the three year period, there were 51 total accidents in the study area with 17 accidents at the signalized High Street/William Street intersection, 13 accidents at the signalized Mechanic Street intersection, 3 accidents at the Winthrop Street intersection, 12 accidents along State Street (not at the intersections), and 6 accidents in area parking lots.

Of the 17 accidents at the State Street / High Street / William Street intersection, 8 were rearend accidents, 7 were right angle accidents, 1 was an overtaking accident, and 1 was a pedestrian accident.

Of the 13 accidents at the State Street / Mechanic Street intersection, 10 were rearend accidents, 1 was a right angle accident, 1 was an overtaking accident, and 1 was a right turn accident.

All three accidents at the Winthrop Street intersection were rearend accidents on State Street.

Of the 12 midblock accents on State Street, 10 were rearend accidents, 1 was a right turn accident, and 1 was a pedestrian accident. The midblock accident rate is 5.01 accidents per million vehicle miles,



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# Re: Traffic Impact Assessment – Proposed Taco Bell Development State Street – Watertown, NY

which is above the statewide average of 2.71 accidents per million vehicle miles for similar facilities. It is likely that a significant number of the rearend accidents report on the midblock are related to the traffic signals to the east and west of the site.

33 (73%) of the 45 accidents along State Street were either rearend or overtaking accidents, which are the most common type of accident at a signalized intersections. There are no other distinct patterns noted.

71% of the accidents were property damage only accidents with 29% including injuries. There was one fatality accident on September 25<sup>th</sup>, 2020 which involved a westbound motorist on State Street striking a pedestrian.

The detailed accident summary has been attached.

# 2023 Background Operations

The proposed Taco Bell development is assumed to be completed by 2023, therefore 2023 was used as the design year for this study. In order to fully understand the impacts of the development on the adjacent roadway system, analysis of the operations immediately before the project opening must first be completed. The existing traffic volumes were adjusted by a growth rate to account for any unknown development that may occur prior to completion of the project.

Historical traffic volumes along State Street between Washington Street and Park Drive were taken from the NYSDOT Traffic Data Viewer website and reviewed in order to identify an appropriate background growth rate. Long term growth rates in the area have been negative at -0.2% per year between 2003 and 2019. In order to maintain a conservative analysis, a positive +1.0% per year growth was chosen and used to grow the 2022 existing traffic volumes to the 2023 background condition. The 2023 background peak hour volumes for the study area with 1.0% growth are shown in Figure 1. The detailed growth calculations have been attached.

The Synchro capacity analysis of the 2023 background condition shows no increases in delay at the study area intersection during the peak hours. All movements continue to operate at Level of Service A during both peak hours.

The detailed Level of Service summary and capacity analysis printouts have been attached.

# Trip Generation Estimate and Distribution

The proposed development includes a 2,598 SF Taco Bell restaurant with drive through operations. Trips generated by the proposed development were estimated using the ITE <u>Trip Generation</u>, 11<sup>th</sup> Edition, which is the industry accepted standard for estimating traffic generated by new developments.



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# Re: Traffic Impact Assessment – Proposed Taco Bell Development State Street – Watertown, NY

Land Use 934 – Fast Food Restaurant with Drive-Through Window was used. The 11<sup>th</sup> Edition <u>Trip</u> <u>Generation</u> does not provide direct data for the weekday midday peak, however the 10<sup>th</sup> Edition does provide data on the percentage of total daily traffic generated for each peak hour. The 10<sup>th</sup> Edition, indicates that the evening peak hour is approximately 6.1% of the total daily traffic while the midday peak hour is approximately 11.8% of the total daily traffic generated. Therefore the evening peak hour trip generation rate was multiplied by a factor of 1.934 (11.8/6.1) to develop the trip generation estimate for the midday peak hour.

Additionally, the ITE <u>Trip Generation</u>, was used to estimate the percentage of trips for the proposed development that would be pass-by trips. Pass-by trips are vehicles that stop at the development on their way to another location, such as stopping on their way to work in the morning or on their way home in the evening. These vehicles are already traveling on the roadway and are diverted to the site. Based on data reviewed, the average pass-by percentage for a fast food restaurant is 49% during the morning peak hour and 50% during the evening peak hour. A 50% pass-by trip generation rate was assumed for all three peak hours.

The following table summarizes the trip generation estimate for the proposed Taco Bell development on State Street in Watertown, NY.

# **Trip Generation Summary**

	Mornii	<b>Morning Peak</b>		y Peak	<b>Evening Peak</b>		
	Entering	Exiting	Entering	Entering	Entering	Entering	
Taco Bell–2,598 SF	59	57	83	83	45	41	
Pass-by Trips - 50%	<u>-29</u>	-29	<u>-41</u>	<u>-41</u>	<u>-21</u>	<u>-21</u>	
New Trips Generated	30	28	42	42	24	20	

The detailed trip generation calculations have been attached.

Overall the proposed development is minor traffic generator with less than 60 total trips entering and exiting during the morning peak hour, approximately 83 total trips entering and exiting during the midday peak hour, and less than 50 total trips entering and exiting during the evening peak hour. With half of the trips generated being drawn from traffic already in the area, the site is only expected to generate 20-40 new trips entering and exiting the area during peak hours.

Based on existing traffic patterns, 50% of the new trips generated are expected to arrive from the east on State Street and 50% are expected to arrive from the west on State Street. Exiting traffic is expected to primarily use State Street with 48% expected to exit to each direction, and 4% is expected to travel to the south on Winthrop Street. Traffic was distributed to the driveways assuming that approximately 70% of the traffic generated uses the drive through. Separate pass-by trip distributions were developed for each peak hour based on specific traffic patterns passing the site on State Street.



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# Re: Traffic Impact Assessment – Proposed Taco Bell Development State Street – Watertown, NY

The anticipated arrival/departure distribution and resulting trips generated are shown in Figure 2 for the morning peak hour, Figure 3 for the midday peak hour, and Figure 4 for the evening peak hour. The resultant full build traffic volumes expected when the development is complete are shown in Figure 5.

# **Build Operations**

Based on the projected turning movements on State Street at the site driveways, there are more than sufficient gaps available to accommodate the proposed development. The center left turn lane on State Street provides refuge for both vehicles turning left into the site as well as vehicles turning left out of the site.

Capacity analysis of the build condition with the proposed Taco Bell development indicates that the development will have negligible impacts on traffic operations on State Street. All traffic movements on State Street are projected to operate at Level of Service A with the site driveways all operating at Levels of Service C or better during all three peak hours.

The detailed Level of Service summary and capacity analysis printouts have been attached.

# Conclusions

The additional traffic generated by the proposed Taco Bell development will have no notable or significant impact on traffic operations on State Street or at the adjacent Winthrop Street intersection. There are adequate gaps in traffic to accommodate turning movements into and out of the development, adequate sight lines in each direction, no significant queuing concerns from the adjacent signals, and no capacity concerns. There are no mitigation measures recommended.

If you have any questions or need additional information, please call.

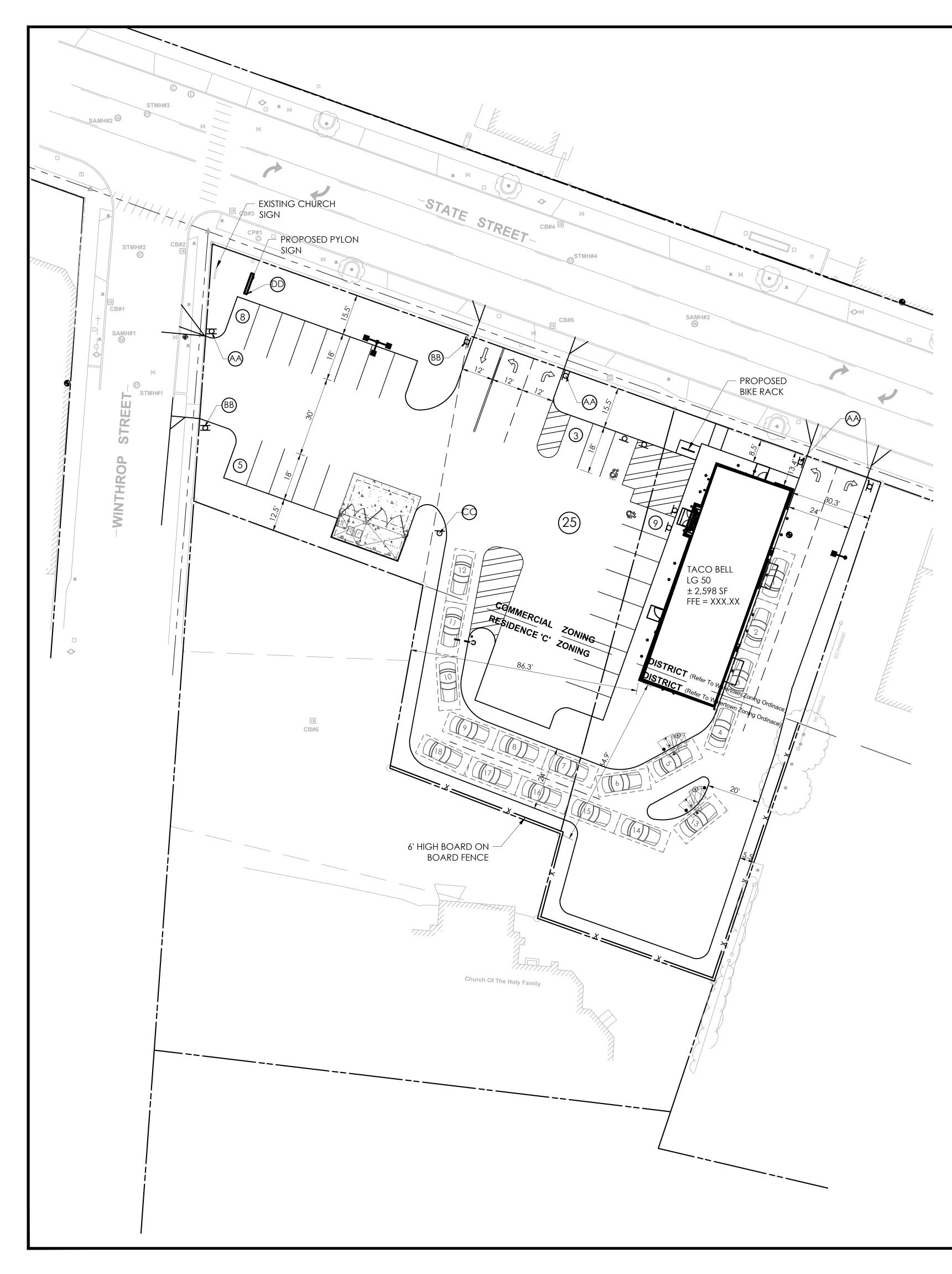
Sincerely,

Gordon T. Stansbury, P.E., ₽.T.O.E

Gordon T. Stansbury, P.E., GTS Consulting

Attachments -

Site Plan Traffic Volume Figures 1-5 Spot Speed Data Trip Generation Estimate Count Data Level of Service Summary Gap Calculations Growth Rate Calculations Accident Data Synchro Capacity Printouts



# SITE LEGEND:

- ACCESSIBLE PARKING SIGN, POST & BOLLARD (REFER TO DETAIL)
- "NO PARKING ANY TIME" SIGN, POST & BOLLARD (REFER B "NO PARKING TO DETAIL)
- C "VAN ACCESSIBLE" SIGN (REFER TO DETAIL)
- (D) SYSL/4" PARKING STALL STRIPING
- E CONCRETE CURB (REFER TO DETAIL)
- (F) TRANSITION CURB (REFER TO DETAIL)
- G ADA RAMP AND DETECTABLE WARNING (REFER TO DETAIL)
- H BOLLARDS (REFER TO ARCH. PLAN DETAIL)
- LAWN/MULCH AREA (REFER TO PLANTING PLAN FOR DELINEATION)
- J LIGHT POLE (REFER TO DETAIL)
- MENU BOARD, CANOPY & SPEAKER BOX (REFER TO (K) MENU BC DETAIL)
- (REFER TO DETAIL)
- (AA) TACO BELL EXIT SIGN (PROVIDED BY SIGN VENDOR)
- TACO BELL ENTRANCE SIGN (PROVIDED BY SIGN (BB) TACO BELL VENDOR)
- CO TACO BELL VENDOR) TACO BELL DRIVE THRU SIGN (PROVIDED BY SIGN
- DD TACO BELL PYLON SIGN (PROVIDED BY SIGN VENDOR)
- (EE) CLEARANCE BAR (REFER TO DETAIL)
- (FF) CONCRETE DUMPSTER PAD (REFER TO ARCH. PLANS)

# **GENERAL NOTES:**

- 1. ALL IMPROVEMENTS SHALL BE IN ACCORDANCE WITH THE MOST RECENT STANDARDS AND SPECIFICATIONS OF THE CITY OF WATERTOWN AND/OR THE APPROPRIATE WATER, SEWER AND/OR DRAINAGE DISTRICTS, AND/OR OTHER AUTHORITIES HAVING JURISDICTION.
- 2. ALL EXISTING BUILDING(S), SITE, ROADWAY, UTILITY, BOUNDARY, AND TOPOGRAPHY INFORMATION SHOWN ON THIS PLAN IS REPRESENTED BASED ON USE OF THE LISTED REFERENCES. CONTRACTOR TO VERIFY LOCATION AND LIMITS OF WORK PRIOR TO STARTING. ANY CHANGES OR CONFLICTS DISCOVERED SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER AND ENGINEER. IF CONTRACTOR DOES NOT ACCEPT EXISTING TOPOGRAPHY AS SHOWN ON THE PLANS, WITHOUT EXCEPTION, HE SHALL HAVE MADE, AT HIS EXPENSE, A TOPOGRAPHIC SURVEY BY A REGISTERED LAND SURVEYOR AND SUBMIT IT TO THE OWNER FOR REVIEW.
- 3. CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR EXACT LOCATIONS AND DIMENSIONS OF VESTIBULE, SLOPED PAVING, EXIT PORCHES, RAMPS, PRECISE BUILDING DIMENSIONS, AND EXACT BUILDING UTILITY ENTRANCE LOCATIONS. ALL PAVING, CURBING, FLATWORK, SIDEWALKS, FENCING, BOLLARDS, ETC., WHICH CONFLICT WITH NEW CONSTRUCTION ARE TO BE DEMOLISHED AND DISPOSED OF IN ACCORDANCE WITH ANY LOCAL, STATE, OR FEDERAL REGULATIONS.
- 4. CONTRACTOR MUST PROTECT THE PUBLIC AT ALL TIMES WITH FENCING, BARRICADES, ENCLOSURES, ETC. CONTRACTOR SHALL MAINTAIN ALL EXISTING PARKING, SIDEWALKS, DRIVES, ETC. OUTSIDE OF WORK LIMITS CLEAR AND FREE FROM ANY CONSTRUCTION ACTIVITY AND/OR MATERIAL TO ENSURE EASY AND SAFE PEDESTRIAN AND VEHICULAR TRAFFIC TO AND FROM THE SITE.
- 5. REFER TO THE SURVEY FOR THE PROPERTY BOUNDARY INFORMATION (E.G. LOT AREA, BEARINGS, DISTANCES, ETC).
- 6. ANY DAMAGE TO EXISTING SIDEWALKS ON WINTHROP STREET AS A RESULT OF GENERAL CONSTRUCTION MUST BE REPAIRED TO THE SATISFACTION OF THE CITY ENGINEER.

# **REFERENCE:** 1. SV 1 OF 1, PRELIMINARY SU REVISED ON OCTOBER 4, 20

BY JACOBS LAND SURVEYI

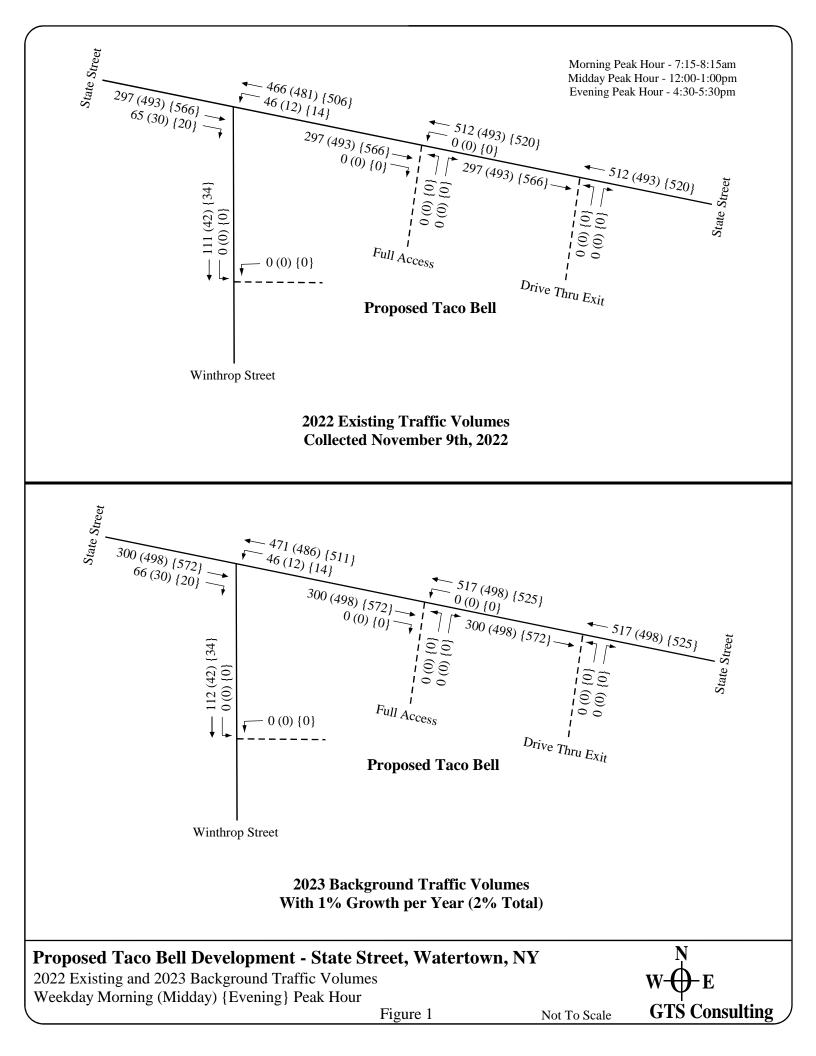
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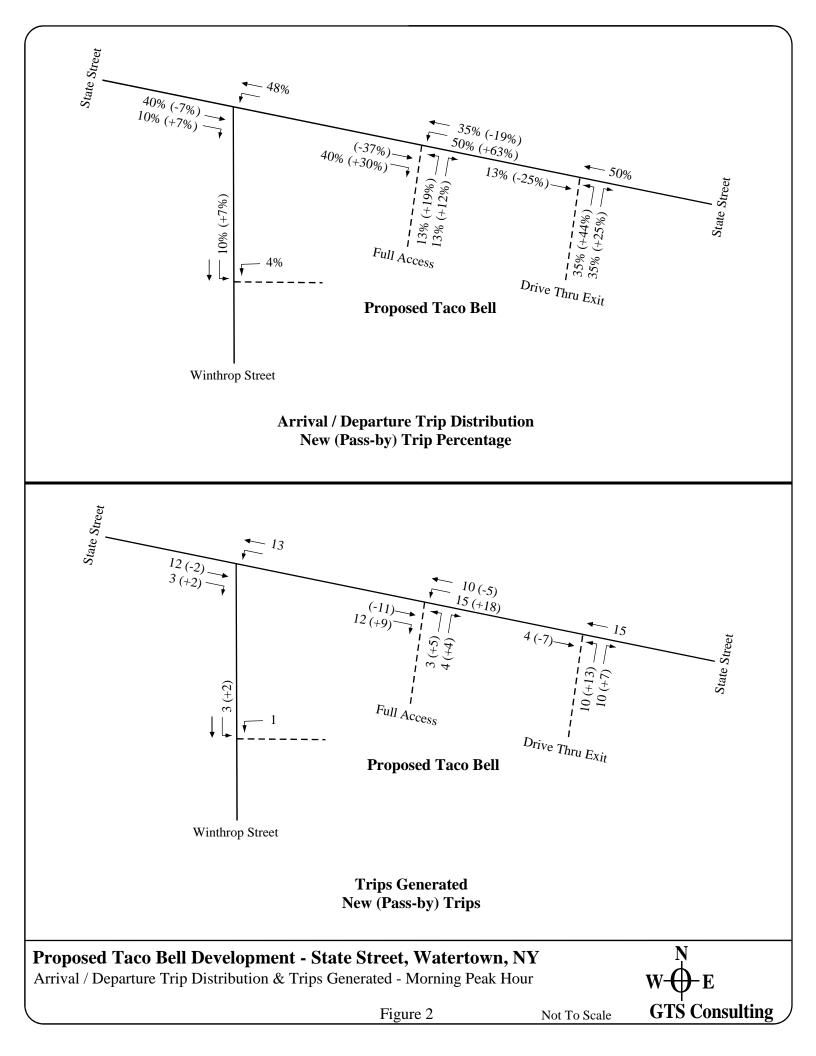
# Proposed TacoBell Development 1997 / 2003 / 2007 Ridge Road, Town of West Seneca, NY

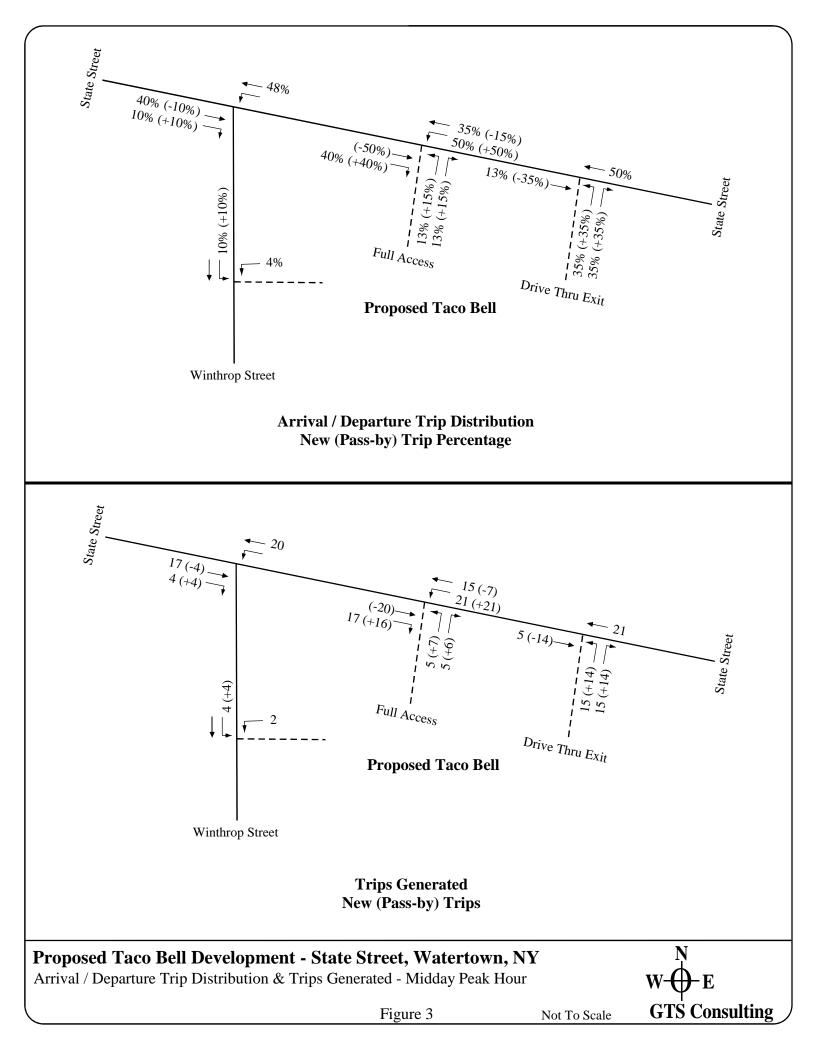
# Intersection Level of Service Summary

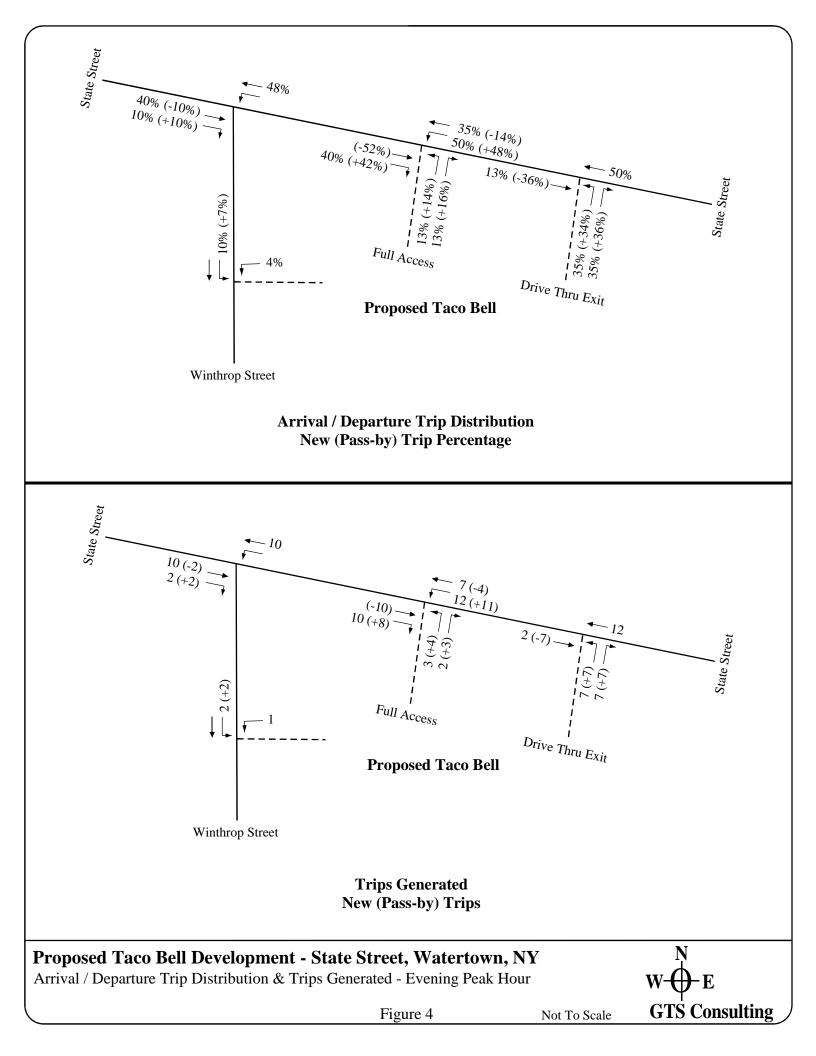
	Μ	orning Peak Ho	ur	Ν	fidday Peak Ho	ur	Evening Peak Hour		
	2022	2023	2023	2022	2023	2023	2022	2023	2023
	Existing	Background	Build	Existing	Background	Build	Existing	Background	Build
Intersection	Condition	Condition	Condition	Condition	Condition	Condition	Condition	Condition	Condition
State Street @									
Winthrop Street									
EB Through/Right	a(0)	a(0)	a(0)	a(0)	a(0)	a(0)	a(0)	a(0)	a(0)
WB Left	a(8)	a(8)	a(9)	a(9)	a(9)	a(9)	a(9)	a(9)	a(9)
WB Through	a(0)	a(0)	a(0)	a(0)	a(0)	a(0)	a(0)	a(0)	a(0)
State Street @									
Full Access Driveway									
EB Through/Right	-	-	a(0)	-	-	a(0)	-	-	a(0)
WB Left	-	-	a(8)	-	-	a(9)	-	-	a(9)
WB Through	-	-	a(0)	-	-	a(0)	-	-	a(0)
NB Left	-	-	c(16)	-	-	c(16)	-	-	c(17)
NB Right	-	-	b(11)	-	-	b(12)	-	-	b(13)
State Street @									
Drive Thru Exit Driveway									
EB Through	-	-	a(0)	-	-	a(0)	-	-	a(0)
WB Through	-	-	a(0)	-	-	a(0)	-	-	a(0)
NB Left	-	-	c(15)	-	-	c(15)	-	-	c(16)
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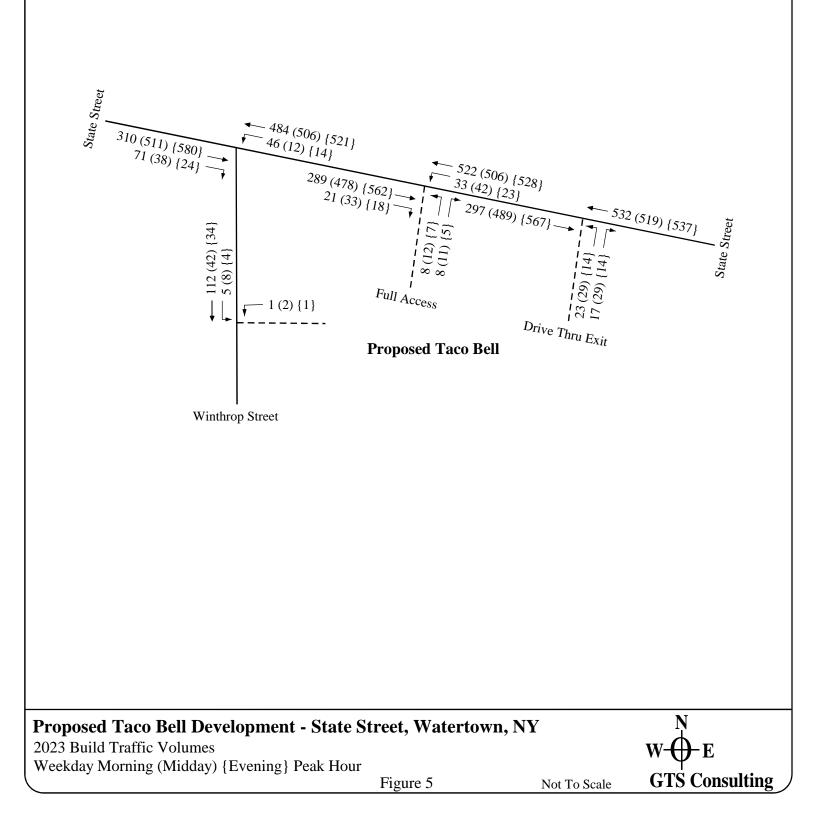
a(9) – Unsignalized Level of Service (Average Delay per Vehicle in Seconds) – Synchro











### Intersection Gap Study

Project: Date:	Proposed Taco Bel 11/9/2022	l Developme	ent - State St	reet - Water	rtown, NY				T	Consult	ing
Intersection:	Proposed Site Acce				_						
Movement:	Right Turns Exiting	/ Left Turns	Entering								
		6-9 sec	10-13 sec	14-17 sec	18-19 sec	20-23 sec	24-25 sec	26-29 sec	>29 sec	Interval	Hour
Time Interval		x 1	x 2	х З	x 4	x 5	x 6	х7	x 8	Total	Total
Morning Peak Hou	ır										
7:15-7:30am	# of Gaps	4	5	1	2	1	2	3	7		
	# of Vehicles	4	10	3	8	5	12	21	56	119	
7:30-7:45am	# of Gaps	9	7	8	3	2	2	1	4		
	# of Vehicles	9	14	24	12	10	12	7	32	120	
7:45-8:00am	# of Gaps	3	5	3	2	3	4	2	5		
	# of Vehicles	3	10	9	8	15	24	14	40	123	
8:00-8:15am	# of Gaps	9	6	8	1	1	0	1	6		
	# of Vehicles	9	12	24	4	5	0	7	48	109	471
Midday Peak Hour 12:00-12:15pm	# of Gaps	14	7	4	2	4	1	2	1		
12.00 12.100	# of Vehicles	14	14	12	8	20	6	14	8	96	
12:15-12:30pm	# of Gaps	11	2	7	1	3	2	2	1		
	# of Vehicles	11	4	21	4	15	12	14	8	89	
12:30-12:45pm	# of Gaps	9	6	2	5	2	2	1	2		
	# of Vehicles	9	12	6	20	10	12	7	16	92	
12:45-1:00pm	# of Gaps	12	6	6	1	2	2	1	2		
	# of Vehicles	12	12	18	4	10	12	7	16	91	368
Evening Peak Hou						-	-				
4:30-4:45pm	# of Gaps	12	3	4	0	5	0	1	3		
	# of Vehicles	12	6	12	0	25	0	7	24	86	
4:45-5:00pm	# of Gaps	20	9	6	1	1	0	2	1		
	# of Vehicles	20	18	18	4	5	0	14	8	87	
5:00-5:15pm	# of Gaps	8	2	3	0	3	0	2	1		
	# of Vehicles	8	4	9	0	15	0	14	8	58	
5:15-5:30pm	# of Gaps	17	8	3	0	0	1	1	1		
	# of Vehicles	17	16	9	0	0	6	7	8	63	294

G

### Intersection Gap Study



Intersection: Pr Movement: Le Time Interval Morning Peak Hour 7:15-7:30am 7:30-7:45am 7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	1/9/2022 roposed Site Acce eft Turns Exiting # of Gaps # of Vehicles # of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles	6-9 sec x 1 11 13 13 16 16 9	10-13 sec x 2 9 18 11 22 10 20	14-17 sec x 3 5 15 6 18 2 6	18-19 sec x 4 1 4 1 4 2	20-23 sec x 5 5 25 2 10	24-25 sec x 6 1 6 1 6	x 7 0 0 0	>29 sec x 8	Interval Total <b>79</b>	Hour Total
Movement: Le	# of Gaps # of Vehicles # of Vehicles # of Vehicles # of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps	6-9 sec x 1 11 13 13 16 16 9	10-13 sec x 2 9 18 11 22 10 20	x 3 5 15 6 18 2	x 4	x 5 5 <b>25</b> 2	x 6 1 6 1	x 7 0 0 0	x 8 0 0	Total	
Time Interval Morning Peak Hour 7:15-7:30am 7:30-7:45am 7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps # of Gaps # of Gaps	x 1 11 13 13 16 16 16 9	x 2 9 18 11 22 10 20	x 3 5 15 6 18 2	x 4	x 5 5 <b>25</b> 2	x 6 1 6 1	x 7 0 0 0	x 8 0 0	Total	
Morning Peak Hour 7:15-7:30am 7:30-7:45am 7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps	x 1 11 13 13 16 16 16 9	x 2 9 18 11 22 10 20	x 3 5 15 6 18 2	x 4	x 5 5 <b>25</b> 2	x 6 1 6 1	x 7 0 0 0	x 8 0 0	Total	
Morning Peak Hour 7:15-7:30am 7:30-7:45am 7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps	11 11 13 13 16 16 16 9	9 18 11 22 10 20	5 15 6 18 2	1 4 1 4	5 <b>25</b> 2	1 6 1	0 0 0	0 <b>0</b>		Total
7:15-7:30am 7:30-7:45am 7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps	11 13 13 16 16 9	<b>18</b> 11 <b>22</b> 10 <b>20</b>	<b>15</b> 6 <b>18</b> 2	<b>4</b> 1 <b>4</b>	<b>25</b> 2	<b>6</b> 1	<b>0</b> 0	0	79	
7:15-7:30am 7:30-7:45am 7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Vehicles # of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps	11 13 13 16 16 9	<b>18</b> 11 <b>22</b> 10 <b>20</b>	<b>15</b> 6 <b>18</b> 2	<b>4</b> 1 <b>4</b>	<b>25</b> 2	<b>6</b> 1	<b>0</b> 0	0	79	
7:30-7:45am 7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Gaps # of Vehicles # of Gaps # of Vehicles # of Gaps	13 <b>13</b> 16 <b>16</b> 9	11 22 10 20	6 <b>18</b> 2	1 <b>4</b>	2	1	0	-	79	
7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Vehicles # of Gaps # of Vehicles # of Gaps	<b>13</b> 16 <b>16</b> 9	<b>22</b> 10 <b>20</b>	<b>18</b> 2	4		-	-	0		
7:45-8:00am 8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Gaps # of Vehicles # of Gaps	16 <b>16</b> 9	10 <b>20</b>	2		10	6	•			
8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Vehicles # of Gaps	<b>16</b> 9	20		2		U	0	0	73	
8:00-8:15am Midday Peak Hour 12:00-12:15pm	# of Gaps	9		6	<u> </u>	1	2	0	0		
Midday Peak Hour 12:00-12:15pm			10	0	8	5	12	0	0	67	
Midday Peak Hour 12:00-12:15pm	# of Vehicles		10	1	1	1	2	0	0		
12:00-12:15pm		9	20	3	4	5	12	0	0	53	272
	# of Gaps	17	11	1	0	0	1	1	0		
10.1E 10.00mm	# of Vehicles	17	22	3	0	0	6	7	0	55	
12:15-12:30pm	# of Gaps	11	4	4	0	3	1	1	0		
	# of Vehicles	11	8	12	0	15	6	7	0	59	
12:30-12:45pm	# of Gaps	13	5	5	1	2	2	0	0		
	# of Vehicles	13	10	15	4	10	12	0	0	64	
12:45-1:00pm	# of Gaps	11	5	3	1	2	2	1	0		
	# of Vehicles	11	10	9	4	10	12	7	0	63	241
Evening Peak Hour											
4:30-4:45pm	# of Gaps	12	3	2	2	0	0	0	2		
	# of Vehicles	12	6	6	8	Ŭ Ŭ	ů 0	Ŭ Û	16	48	
4:45-5:00pm	# of Gaps	17	3	3	0	0	1	0	1	-	
	# of Vehicles	17	6	9	Ő	Ŭ Ŭ	6	Ŭ Û	8	46	
5:00-5:15pm	# of Gaps	10	0	4	0	1	1	0	0	-	
	# of Vehicles	10	0	12	Ő	5	6	0	0	33	
5:15-5:30pm		15	5	1	0	0	0	0	1		
	# of Gaps		10	3	0	0	0	0	8	36	163

#### Proposed Taco Bell Development - State Street - Watertown NY

Speed Study Measurements - State Street Passing Site

11//9/2022

Distance Tra	velled (ft) = 145	50 Speed Me	easurements per Direction		Speed Limit	30 mph		
EB Time	Calculated	EB Time	Calculated	WB Time	Calculated		WB Time	Calculated
Seconds	Speed	Seconds	Speed	Seconds	Speed		Seconds	Speed
3.63	27	3.10	32	3.38	29		2.9	34
3.6	27	3.09	32	3.35	30		2.9	34
3.58	28	3.09	32	3.31	30		2.88	34
3.46	29	3.06	32	3.28	30		2.87	34
3.42	29	3.05	32	3.28	30		2.87	34
3.4	29	3.04	33	3.27	30		2.87	34
3.39	29	3.04	33	3.12	32		2.85	35
3.37	29	3.03	33	3.1	32		2.81	35
3.32	30	3.03	33	3.1	32		2.78	36
3.28	30	3.03	33	3.1	32		2.78	36
3.28	30	3.03	33	3.04	33		2.75	36
3.28	30	3.01	33	3.02	33		2.72	36
3.28	30	3	33	3.01	33		2.72	36
3.27	30	2.97	33	3	33		2.72	36
3.23	31	2.97	33	3	33		2.66	37
3.22	31	2.95	34	3	33		2.66	37
3.22	31	2.87	34	3	33		2.65	37
3.22	31	2.85	35	2.87	34		2.62	38
3.22	31	2.84	35	2.94	34		2.61	38
3.15	31	2.75	36	2.94	34		2.57	38
3.15	31	2.74	36	2.94	34		2.54	39
3.13	32	2.72	36	2.94	34		2.52	39
3.12	32	2.72	36	2.93	34		2.51	39
3.12	32	2.59	38	2.91	34		2.5	40
3.1	32	2.44	41	2.9	34		2.5	40
Eastbound				Westbound				
Average Spe	ed =	32 mph		Average Spe	ed =	34	mph	

85th Percentile Speed =

37.5 mph

85th Percentile Speed = 34.5 mph

# **Background Traffic Growth Calculations**

## Proposed Taco Bell Development, State Street, Watertown, NY

NYSDOT Traffic Data Viewer

### State Street (NYS Route 3) - Between Washington Street and Park Drive

2019	)	2015	5	2009	)	200	3
13,6	77 veh	13,3	64 veh	12,5	90 veh	14,1	135 veh
	+0.6% per year		+1.0% per year		-1.8% per year		
	+0.9	9% pe	r year				
			-0.2% per year				

### Use +1.0% annual growth for conservative traffic projections

### Proposed Taco Bell Development State Street - Watertown, NY

### **Trip Generation Estimate**

Proposed	Devel	opment
----------	-------	--------

2,598 SF - Taco Bell with Drive Through

#### ITE Trip Generation - 11th Edition

Land Use 934 - Fast-Food Restaura	nt with Drive-Through Window		
AM Peak Hour	44.61 Trips/1,000 SF	51% Enter	49% Exit
Midday Peak Hour	63.89 Trips/1,000 SF	50% Enter	50% Exit
PM Peak Hour	33.03 Trips/1,000 SF	52% Enter	48% Exit
* - No Trip Generation Rates Availalble	, Trip Generaiton 10th Edition - 4:30-	5:30 = 6.1% of daily tra	afffic, 12-1pm = 11.8% of daily traffic, midday rate = 63.89 trip/1,000 SF

#### Pass-by Trip Percentages

Fast Food Restaurant - AM - 49%, PM - 50% - Use 50% for all three peak hours

#### **Trip Generation Summary**

		Morn	ing Peak Hoι	ır	Eve	ning Peak Ho	ur	Satu	rday Peak Ho	ur
Development	Size	Total Trips	Entering	Exiting	Total Trips	Entering	Exiting	Total Trips	Entering	Exiting
Taco Bell	2,598 SF	116	59	57	166	83	83	86	45	41
Pass-by Tri	ips - 50%	<u>-58</u>	<u>-29</u>	<u>-29</u>	<u>-82</u>	<u>-41</u>	<u>-41</u>	<u>-42</u>	<u>-21</u>	<u>-21</u>
New Trips O	Generated	58	30	28	84	42	42	44	24	20

### Proposed Taco Bell Development - State Street, Watertown, NY Accident History Summaries - June 1, 2019 Through May 31, 2022

Accident #	Date	Location	Туре	# Cars	Severity	Direction	Conditions	<b>Contributing Factors</b>
1	6/17/2019	Winrthrop @ State	Rearend	2	INJ	WB / WB	Dry	Following Too Closely
2	8/5/2019	Midblock State	Rearend	2	PDO	WB / WB	Dry	Following Too Closely
3	8/16/2019	Midblock State	Rearend	2	INJ	WB/WB	Dry	Following Too Closely
4	8/16/2019	Midblock State	Rearend	2	PDO	WB/WB	Dry	Following Too Closely
5	10/19/2019	Parking Lot	Backing	2	PDO	WB / Backing	Dry	Backing Unsafely
6	10/21/2019	Mechanic @ State	Right Turn	2	PDO	WB Right / SB Stopped	Dry	Turning Improper
7	10/21/2019	High/William @ State	Rearend	2	PDO	EB / EB	Dry	Following Too Closely
8	10/23/2019	Mechanic @ State	Rearend	2	PDO	EB / EB	Dry	Following Too Closely
9	10/23/2019	High/William @ State	Rearend	3	INJ	EB / EB / EB	Dry	Following Too Closely
10	11/8/2019	Mechanic @ State	Rearend	2	PDO	WB/WB	Dry	Following Too Closely
11	11/12/2019	Winrthrop @ State	Rearend	2	PDO	WB/WB	lcy	Pavement Slippery
12	11/19/2019	Parking Lot	Backing	2	PDO	Unknown / Backing	Dry	Backing Unsafely
13	12/18/2019	Mechanic @ State	Rearend	2	PDO	WB/WB	lcy	Driver Inattention
14	1/2/120	High/William @ State	Overtaking	2	PDO	WB/WB	Dry	Failure to Yield ROW
15	6/1/2020	Mechanic @ State	Rearend	2	PDO	WB/WB	Dry	Following Too Closely
16	6/12/2020	Mechanic @ State	Rearend	2	PDO	WB/WB	Dry	Following Too Closely
17	6/16/2020	High/William @ State	Right Angle	2	PDO	EB / Unknown	Dry	Failure to Yield ROW
18	7/1/2020	Mechanic @ State	Rearend	2	INJ	WB/WB	Dry	Following Too Closely
19	8/7/2020	Mechanic @ State	Rearend	2	INJ	WB/WB	Dry	Following Too Closely
20	9/11/2020	Midblock State	Rearend	2	PDO	EB / EB	Dry	Following Too Closely
21	9/18/2020	Midblock State	Rearend	3	INJ	WB/WB/WB	Dry	Following Too Closely
22	9/25/2020	Midblock State	Pedestrian	1	FAT	WB / Pedestrian	Dry	View Obstructed
23	10/1/2020	High/William @ State	Rearend	2	PDO	EB / EB	Dry	Driver Inattention
24	10/18/2020	High/William @ State	Rearend	3	INJ	EB / EB / EB	Dry	Following Too Closely
25	2/6/2021	High/William @ State	Right Angle	2	PDO	EB / Unknown	lcy	Speed UnSafe
26	4/1/2021	High/William @ State	Right Angle	2	PDO	EB / Unknown	Dry	Runaway Vehicle
27	4/27/2021	Midblock State	Rearend	2	INJ	WB/WB	Dry	Following Too Closely
28	4/30/2021	Parking Lot	Backing	2	PDO	EB / Backing	lcy	Backing Unsafely
29	5/1/2021	Parking Lot	Backing	2	PDO	EB / Backing	Dry	Backing Unsafely
30	5/22/2021	High/William @ State	Rearend	2	PDO	WB/WB	Dry	Alcohol Involvment
31	5/22/2021	High/William @ State	Right Angle	2	PDO	WB / Unknown	Dry	Alcohol Involvment
32	5/26/2021	Winrthrop @ State	Rearend	2	PDO	WB/WB	Dry	Following Too Closely
33	6/5/2021	High/William @ State	Rearend	2	PDO	EB / EB	Dry	Following Too Closely
34	6/25/2021	High/William @ State	Rearend	2	INJ	WB/WB	Dry	Reaction to Another Vehicle
35	6/25/2021	Midblock State	Rearend	3	PDO	WB/WB/WB	Dry	Not Entered
36	8/17/2021	Mechanic @ State	Rearend	2	PDO	EB / EB	Wet	Following Too Closely

Accident #	Date	Location	Туре	# Cars	Severity	Direction	Conditions	<b>Contributing Factors</b>
37	9/5/2021	High/William @ State	Right Angle	2	PDO	WB / Unknown	Dry	Traffic Control Disregared
38	9/21/2021	Parking Lot	Fixed Object	1	PDO	Backing / Utility Pole	Dry	Backing Unsafely
39	9/12/2021	High/William @ State	Right Angle	2	INJ	EB / Unknown	Dry	Failure to Yield ROW
40	9/26/2021	Midblock State	Rearend	2	INJ	EB / EB	Dry	Following Too Closely
41	11/9/2021	High/William @ State	Pedestrian	1	INJ	EB . Pedestrain	Wet	Driver Inattention
42	11/12/2021	High/William @ State	Rearend	2	PDO	NB / NB	Wet	Following Too Closely
43	11/21/2021	Mechanic @ State	Right Angle	2	PDO	WB / SB	Dry	Driver Inattention
44	12/8/2021	Mechanic @ State	Rearend	2	PDO	WB/WB	lcy	Pavement Slippery
45	12/10/2021	Mechanic @ State	Overtaking	2	PDO	WB/WB	Dry	Speed UnSafe
46	2/11/2022	Mechanic @ State	Rearend	2	INJ	WB/WB	Dry	Following Too Closely
47	2/21/2021	Parking Lot	Sideswipe	2	PDO	Unknown / Backing	lcy	Backing Unsafely
48	2/26/2022	High/William @ State	Right Angle	2	PDO	EB / Unknown	lcy	Failure to Yield ROW
49	2/27/2022	Midblock State	Rearend	2	INJ	WB/WB	Wet	Speed UnSafe
50	3/7/2022	Midblock State	Rearend	2	PDO	WB/WB	Wet	Following Too Closely
51	4/7/2022	Midblock State	Right Turn	2	PDO	WB / WB	Dry	Turning Improper
State Street	@ Mechanic St	reet - 13 Accidents	State Street @	High/Willi	am Street - 1	7 Accidents	State Street @ V	Vinthrop Street - 3 Accidents
10 - Rearend	Accidents		8 - Rearend Ac	cidents			3 - Rearend Acc	idents
1 - Right Ang	le Accident		7 - Right Angle	Accidents				
1 - Right Turi	n Accident		1 - Overtaking /	Accident				
1 - Overtakin	g Accident		1 - Pedestrian A	Accident				
State Street I 10 - Rearend 1- Right Turn 1 - Pedestria	Accident	Accidents	Parking Lots - 6	S Accident	S			
State Street -	AADT = 13,67	77 Vehicles						
	_							

Link Accident Rates

<u># Accidents X 1,000,000</u> Link Length X AADT X # Years X 365 Days Time Period = 3 years State Street Link Length = 0.16 Miles

#### State Street Midblock - 12 Accidents

Accident Rate = 5.01 accidents per million vehicle miles

Statewide average for similar facilities = 2.71 accidents per million vehicle miles Urban - Free Access - 3 Lanes - Undivided - Mainline Accidents Only) Link Accident History is Above the Statewide Average

File Name : State Street @ Winthrop Street Site Code : 00000001 Start Date : 11/9/2022

Page No : 1

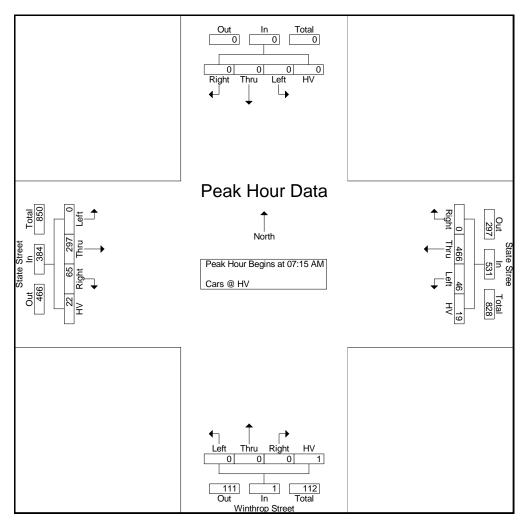
						Gr	oups P	rinted-	Cars @	₽ HV							_
						State S			٧		Street			State S			
		Southb				Westb				Northb				Eastbo			
Start Time	Right	Thru	Left	ΗV	Right	Thru	Left	ΗV	Right	Thru	Left	ΗV	Right	Thru	Left	ΗV	Int. Total
07:00 AM	0	0	0	0	0	70	2	3	0	0	0	0	4	52	0	6	137
07:15 AM	0	0	0	0	0	114	13	3	0	0	0	0	12	65	0	3	210
07:30 AM	0	0	0	0	0	124	11	5	0	0	0	1	17	73	0	6	237
07:45 AM	0	0	0	0	0	122	17	6	0	0	0	0	32	78	0	7	262
Total	0	0	0	0	0	430	43	17	0	0	0	1	65	268	0	22	846
08:00 AM	0	0	0	0	0	106	5	5	0	0	0	0	4	81	0	6	207
08:15 AM	0	0	0	0	0	124	1	4	0	0	0	0	3	69	0	2	203
08:30 AM	0	0	0	0	0	100	1	4	0	0	0	0	4	70	0	8	187
08:45 AM	0	0	0	0	0	107	4	2	0	0	0	0	2	78	0	9	202
Total	0	0	0	0	0	437	11	15	0	0	0	0	13	298	0	25	799
11:00 AM	0	0	0	0	0	127	2	9	0	0	0	0	5	103	0	4	250
11:15 AM	0	0	0	0	0	126	2	4	0	0	0	0	3	112	0	3	250
11:30 AM	0	0	0	0	0	109	3	6	0	0	0	0	3	113	0	5	239
11:45 AM	0	0	0	0	0	125	2	7	0	0	0	0	6	95	0	5	240
Total	0	0	0	0	0	487	9	26	0	0	0	0	17	423	0	17	979
12:00 PM	0	0	0	0	0	130	7	6	0	0	0	0	6	122	0	6	277
12:15 PM	0	0	0	0	0	129	1	3	0	0	0	0	5	126	0	1	265
12:30 PM	0	0	0	0	0	127	2	1	0	0	0	0	13	119	0	4	266
12:45 PM	0	0	0	0	0	95	2	6	0	0	0	0	6	126	0	6	241
Total	0	0	0	0	0	481	12	16	0	0	0	0	30	493	0	17	1049
04:00 PM	0	0	0	0	0	127	4	6	0	0	0	0	9	125	0	1	272
04:15 PM	0	0	0	0	0	117	2	2	0	0	0	0	12	137	0	3	273
04:30 PM	0	0	0	0	0	131	5	1	0	0	0	0	5	128	0	1	271
04:45 PM	0	0	0	0	0	136	4	4	0	0	1	0	2	128	0	2	277
Total	0	0	0	0	0	511	15	13	0	0	1	0	28	518	0	7	1093
05:00 PM	0	0	0	0	0	123	3	3	0	0	1	0	8	162	0	1	301
05:15 PM	0	0	0	0	0	116	2	2	0	0	0	0	5	148	0	1	274
05:30 PM	0	0	0	0	0	134	2	2	0	0	0	0	2	125	0	3	268
05:45 PM	0	0	0	0	0	103	1	0	0	0	1	0	0	134	0	2	241
Total	0	0	0	0	0	476	8	7	0	0	2	0	15	569	0	7	1084
Grand Total	0	0	0	0	0	2822	98	94	0	0	3	1	168	2569	0	95	5850
Apprch % Total %	0	0	0	0	0	93.6	3.3	3.1	0	0	75	25	5.9	90.7	0	3.4	
	0	0	0	0	0	48.2	1.7	1.6	0	0	0.1	0	2.9	43.9	0	1.6	1

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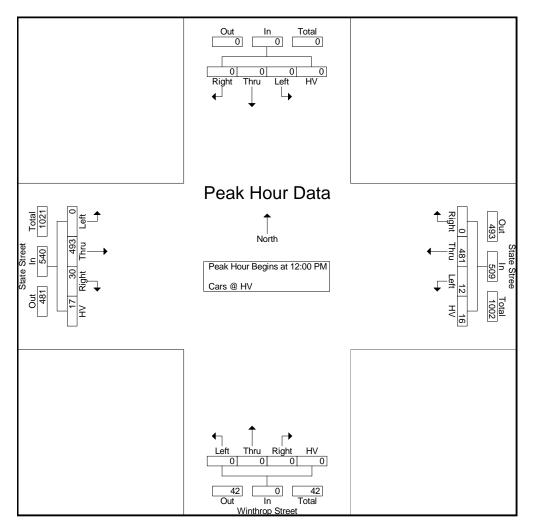
File Name : State Street @ Winthrop Street Site Code : 00000001 Start Date : 11/9/2022 Page No : 2

		So	uthbo	ound				ate Stestbo					hrop rthbo	Street und	d Eastbound						
Start Time	Right	Thru	Left	HV	App. Total	Right	Thru	Left	HV	App. Total	Right	Thru	Left	HV	App. Total	Right	Thru	Left	HV	App. Total	Int. Total
Peak Hour A	nalysi	s From	n 07:0	0 AM t	o 09:45	AM -	Peak 1	1 of 1													
Peak Hour fo	or Enti	re Inte	rsectio	on Beg	jins at 0	7:15 A	M														
07:15 AM	0	0	0	0	0	0	114	13	3	130	0	0	0	0	0	12	65	0	3	80	210
07:30 AM	0	0	0	0	0	0	124	11	5	140	0	0	0	1	1	17	73	0	6	96	237
07:45 AM	0	0	0	0	0	0	122	17	6	145	0	0	0	0	0	32	78	0	7	117	262
08:00 AM	0	0	0	0	0	0	106	5	5	116	0	0	0	0	0	4	81	0	6	91	207
Total Volume	0	0	0	0	0	0	466	46	19	531	0	0	0	1	1	65	297	0	22	384	916
% App. Total	0	0	0	0		0	87.8	8.7	3.6		0	0	0	100		16.9	77.3	0	5.7		
PHF	.000	.000	.000	.000	.000	.000	.940	.676	.792	.916	.000	.000	.000	.250	.250	.508	.917	.000	.786	.821	.874



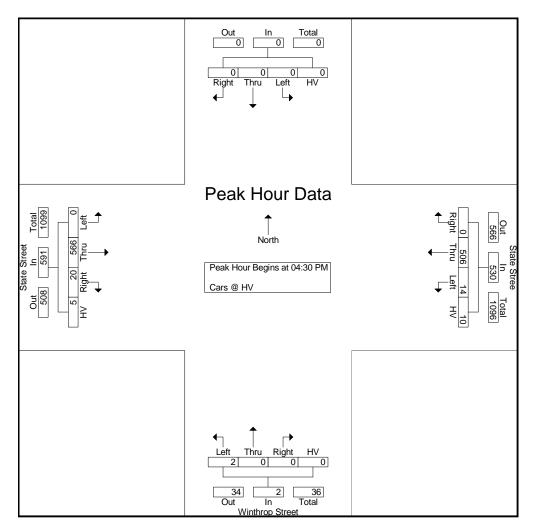
File Name : State Street @ Winthrop Street Site Code : 00000001 Start Date : 11/9/2022 Page No : 3

		So	uthbo	und				ate St estbo					hrop : rthbo					ate St astbou			
Start Time	Right	Thru	Left	ΗV	App. Total	Right	Thru	Left	ΗV	App. Total	Right	Thru	Left	ΗV	App. Total	Right	Thru	Left	ΗV	App. Total	Int. Total
Peak Hour A	nalysi	s From	n 10:00	0 AM t	to 01:45	PM -	Peak '	l of 1													
Peak Hour for	or Enti	re Inte	rsectio	on Beg	gins at 1	2:00 P	M														
12:00 PM	0	0	0	0	0	0	130	7	6	143	0	0	0	0	0	6	122	0	6	134	277
12:15 PM	0	0	0	0	0	0	129	1	3	133	0	0	0	0	0	5	126	0	1	132	265
12:30 PM	0	0	0	0	0	0	127	2	1	130	0	0	0	0	0	13	119	0	4	136	266
12:45 PM	0	0	0	0	0	0	95	2	6	103	0	0	0	0	0	6	126	0	6	138	241
Total Volume	0	0	0	0	0	0	481	12	16	509	0	0	0	0	0	30	493	0	17	540	1049
% App. Total	0	0	0	0		0	94.5	2.4	3.1		0	0	0	0		5.6	91.3	0	3.1		
PHF	.000	.000	.000	.000	.000	.000	.925	.429	.667	.890	.000	.000	.000	.000	.000	.577	.978	.000	.708	.978	.947



File Name : State Street @ Winthrop Street Site Code : 00000001 Start Date : 11/9/2022 Page No : 4

		So	uthbo	ound				ate St estbo					hrop : rthbo		:			ate St astbou			
Start Time	Right	Thru	Left	ΗV	App. Total	Right	Thru	Left	ΗV	App. Total	Right	Thru	Left	ΗV	App. Total	Right	Thru	Left	ΗV	App. Total	Int. Total
Peak Hour A	nalysi	s Fron	n 02:0	0 PM t	o 05:45	PM -	Peak '	1 of 1													-
Peak Hour for	or Enti	re Inte	rsectio	on Beg	jins at 0	4:30 P	M														
04:30 PM	0	0	0	0	0	0	131	5	1	137	0	0	0	0	0	5	128	0	1	134	271
04:45 PM	0	0	0	0	0	0	136	4	4	144	0	0	1	0	1	2	128	0	2	132	277
05:00 PM	0	0	0	0	0	0	123	3	3	129	0	0	1	0	1	8	162	0	1	171	301
05:15 PM	0	0	0	0	0	0	116	2	2	120	0	0	0	0	0	5	148	0	1	154	274
Total Volume	0	0	0	0	0	0	506	14	10	530	0	0	2	0	2	20	566	0	5	591	1123
% App. Total	0	0	0	0		0	95.5	2.6	1.9		0	0	100	0		3.4	95.8	0	0.8		
PHF	.000	.000	.000	.000	.000	.000	.930	.700	.625	.920	.000	.000	.500	.000	.500	.625	.873	.000	.625	.864	.933



File Name : State Street @ Winthrop Street Site Code : 00000001 Start Date : 11/9/2022

Page No : 1

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		Southb	ound			State S Westb	Stree			ls Vinthrop Northb				State S Eastbo			
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	3
Total	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	4
08:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1
Total	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2
11:15 AM 11:30 AM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	1 0	0 0	0 0	0 0	3 1	4 1
Total	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	6	7
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2 2 2
12:30 PM 12:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	0	0 0	0 0	0 1	0 0	0 0	0 0	2 4	25
Total	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	10	<u> </u>
04:00 PM	0	0	0		0	0	0	0		0	0		0	0	0	2	-
04:00 PM 04:15 PM	0 0	0 0	0 0	0	0	0 0	0 0	0 0	0	0 0	0 0	2 1	0 0	0 0	0 0	3 1	5 2
04:30 PM	0	0	0	0	0 0	0	0	0	0	0	0	ò	0	0	0	2	2
04:45 PM	Õ	Õ	Ő	Ő	0	Õ	Õ	Ő	0	Õ	Õ	1	Õ	Õ	Ő	0	1
Total	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	6	10
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	3
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	2
Total	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	1	5
Grand Total	0	0	0	0	0	0	0	1	0	0	0	14	0	0	0	23	38
Apprch %	0	0	0	0	0	0	0	100	0	0	0	100	0	0	0	100	
Total %	0	0	0	0	0	0	0	2.6	0	0	0	36.8	0	0	0	60.5	

Movement         EBT         EBR         WBL         WBT         NBR           Lane Configurations         Image: Minor 289         21         33         522         8         8           Conflicting Peds, #/m         0         4         4         0         0         0           Sign Control         Free         Free         Free         Stop         8         8           Conflicting Peds, #/m         0         4         4         0         0         0           Sign Control         Free         Free         None         None         None         None           Sign Control         Free         Free         Free         None         None         None           Sign Control         Free         Free         None         None         None         None           Sitage Length         -         -         0         0         -         Grade,%         0         -         -         0         0         -           Heavy Vehicles, %         4         1         4         1         1         Heavy Vehicles,%         -         -         -         745         -           Conflicting Flow All         0         0	Intersection											
Lane Configurations 1 1 1 1 29 2 8 8 Traffic Vol, veh/h 289 21 33 522 8 8 Conflicting Peds, #hr 0 4 4 0 0 0 Sign Control Free Free Free Free Stop Stop RT Channelized - None - None - None Storage Length 50 - 0 0 - Grade, % 0 0 0 - Freak Hour Factor 82 62 79 79 90 90 Peak Hour Factor 82 62 79 79 90 90 Peak Hour Factor 82 62 79 79 90 90 Heavy Vehicles, % 4 1 1 4 1 1 Mvmt Flow 352 26 42 661 9 9 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 382 0 1114 369 Stage 1 369 - Stage 2 745 - Critical Hdwy 51 541 - Follow-up Hdwy - 2209 - 3509 3.309 Pd Cap - 1 Maneuver - 1182 - 231 679 Stage 1 745 - Follow-up Hdwy - 2209 - 3509 3.309 Pd Cap - 1 Maneuver - 1177 - Platon blocked, % 4471 - Platon blocked, % 4454 - Stage 2 4471 - Platon blocked, % 4454 - Stage 2 454 - Stage 2 Stage 2 Stage 2 Stage 2 Stage 2 Stage 2 Stage 2	Int Delay, s/veh	0.5										
Traffic Vol, veh/h       289       21       33       522       8       8         Future Vol, veh/h       289       21       33       522       8       8         Conflicting Peaks, #/hr       0       4       4       0       0       0         Sign Control       Free       Free       Free       Free       Stop       Stop         R1 Channelized       - None       - 0       0       -       -       O       0         Veh in Median Storage, #       0       -       0       0       -       -       Paek Hour Factor       82       82       79       90       90         Heavy Vehicles, %       4       1       1       4       1       1         Myrm Flow       352       26       42       661       9       9         Major/Minor       Major1       Major2       Minor1       14       1       1         Myrm Flow       352       26       42       661       9       9         Stage 1       -       -       745       -       541       -         Conflicting Flow All       0       0       3309       309       9       9	Movement	EBT	EBR	WBL	WBT	NBL	NBR					
Future Vol, veh/h       289       21       33       522       8       8         Conflicting Peds, #/hr       0       4       4       0       0       0         Sign Control       Free       Free       Free       Free       Stop       Stop         RT Channelized       -       None       -       None       -       None         Storage Length       -       -       0       0       -       -         Grade, %       0       -       -       0       0       -         Peak Hour Factor       82       82       79       90       90         Heavy Vehinles, %       4       1       1       1         Mvmt Flow       352       26       42       661       9       9         Major/Minor       Major1       Major2       Minor1       Conflicting Flow All       0       0       382       0       1114       369         Stage 1       -       -       -       369       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       - <td>Lane Configurations</td> <td>et 👘</td> <td></td> <td>۳</td> <td>•</td> <td>۳</td> <td>1</td> <td></td> <td></td> <td></td> <td></td> <td></td>	Lane Configurations	et 👘		۳	•	۳	1					
Conflicting Peds, #/hr         0         4         4         0         0         0           Sign Control         Free         Free         Free         Free         Free         Stop           Storage Length         -         500         -         0         0           Vah in Median Storage, #         0         -         0         0           Grade, %         0         -         0         0           Peak Hour Factor         82         82         79         90         90           Heavy Vehicles, %         4         1         1         4         1           Mym Flow         352         26         42         661         9         9           Major/Minor         Major         Major         Minor1         26         20         1114         369           Stage 1         -         -         -         369         -         Stage 1         -         -         541           Conticital Howy Stg 1         -         -         -         541         -         Criticial Howy Stg 2         -         -         541         -           Contical Howy Stg 2         -         -         541         - </td <td>Traffic Vol, veh/h</td> <td>289</td> <td>21</td> <td>33</td> <td>522</td> <td>8</td> <td>8</td> <td></td> <td></td> <td></td> <td></td> <td></td>	Traffic Vol, veh/h	289	21	33	522	8	8					
Sign Control         Free         Free         Free         Stop         Stop           RT Channelized         - None         - None         - None         - None           Storage Length         -         -         50         -         0         0           Veh in Median Storage, #         0         -         -         0         0         -           Pack Hour Factor         82         82         79         90         90         -           Peak Hour Factor         82         26         42         661         9         9           Major/Minor         Major         Major2         Minor1         -         -         -           Conflicting Flow All         0         0         382         0         1114         369         -           Stage 1         -         -         -         745         -         -         -           Critical Hdwy Stg 1         -         -         -         5.41         -         -           Critical Hdwy Stg 2         -         -         5.41         -         -         -           Stage 1         -         -         -         702         -         -	Future Vol, veh/h	289	21	33	522	8	8					
RT Channelized       -       None       -       None         Storage Length       -       -       0       0         Grade, %       0       -       -       0       0         Grade, %       0       -       -       0       0       -         Peak Hour Factor       82       82       79       79       90       90         Heavy Vehicles, %       4       1       1       4       1       1         Myrmt Flow       352       26       42       661       9       9         Major/Minor       Major1       Major2       Minor1       -       -       -         Conflicting Flow All       0       0       382       0       -       -         Stage 1       -       -       -       745       -       -       -         Critical Hdwy Stg 1       -       -       -       5.41       -       -       -         Follow-up Hdwy       -       2.209       -       3.509       3.309       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -	Conflicting Peds, #/hr	0	4	4	0	0	0					
Storage Length       -       -       50       -       0       0         Veh in Median Storage, #       0       -       -       0       0       -         Grade, %       0       -       -       0       0       -         Peak Hour Factor       82       82       79       79       90       90         Heavy Vehicles, %       4       1       1       4       1       1         Myrmt Flow       352       26       42       661       9       9         Major/Minor       Major1       Major2       Minor1       1       4       1       1         Conflicting Flow All       0       0       382       0       1114       369       -       -       -       745       -       -       Critical Hdwy       4.11       -       6.41       6.21       -       -       Tritical Hdwy Stg 1       -       -       -       5.41       -       -       Follow-up Hdwy       -       2.209       -       3.509       3.309       Pol Cap-1 Maneuver       -       1172       -       222       676       Mov Cap-2 Maneuver       -       -       471       -       Pago ach       S	Sign Control	Free	Free	Free	Free	Stop	Stop					
Veh in Median Storage, #       0       -       -       0       0       -         Grade, %       0       -       -       0       0       -         Peak Hour Factor       82       279       79       90       90         Heavy Vehicles, %       4       1       1       4       1       1         Minort       Major/       Major/       Minort       1       4       1         Conflicting Flow All       0       0       382       0       1114       369         Stage 1       -       -       -       369       -       -         Critical Hdwy       -       -       4.11       -       -       -         Critical Hdwy Stg 1       -       -       -       5.41       -       -         Critical Hdwy Stg 2       -       -       -       5.41       -       -         Follow-up Hdw       -       -       2.209       -       3.309       -         Pot Cap-1 Maneuver       -       1177       -       -       -       -         Mov Cap-1 Maneuver       -       1177       -       222       676         Mov Cap-2 Man	RT Channelized	-	None	-	None	-	None					
Grade, %       0       -       -       0       0       -         Peak Hour Factor       82       82       79       79       90       90         Heavy Vehicles, %       4       1       1       4       1       1         Mymt Flow       352       26       42       661       9       9         Major/Minor       Major1       Major2       Minor1       -       -         Conflicting Flow All       0       0       382       0       114       369         Stage 1       -       -       -       745       -       -         Critical Hdwy       -       4.11       -       6.41       6.21         Critical Hdwy Stg 2       -       -       -       5.541       -         Follow-up Hdwy       -       2.209       -       3.509       3.309         Pot Cap-1 Maneuver       -       1182       -       221       676         Mov Cap-1 Maneuver       -       1177       -       222       676         Mov Cap-2 Maneuver       -       -       699       -       Stage 1       -       -       -       699       -	Storage Length	-	-	50	-	0	0					
Peak Hour Factor         82         82         79         90         90           Heavy Vehicles, %         4         1         1         4         1         1           Mymt Flow         352         26         42         661         9         9           Major/Minor         Major1         Major2         Minor1         Employed         1114         369           Stage 1         -         -         -         369         -         Stage 2         -         -         745         -           Critical Hdwy         -         4         1         -         -         5.41         -           Critical Hdwy Stg 1         -         -         -         5.41         -         -           Follow-up Hdwy         -         2.209         -         5.509         3.309         -           Polow-up Hdwy         -         2.209         -         5.41         -         -           Follow-up Hdwy         -         2.209         -         3.509         3.309         -           Stage 1         -         -         -         702         -         -         -           Stage 1         -	Veh in Median Storage	,# 0	-	-	0	0	-					
Heavy Vehicles, %       4       1       1       4       1       1         Mvmt Flow       352       26       42       661       9       9         Major/Minor       Major1       Major2       Minor1       Conflicting Flow All       0       0       382       0       1114       369         Stage 1       -       -       -       369       -       -       -       745       -       111       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -	Grade, %	0	-	-	0	0	-					
Mvmt Flow         352         26         42         661         9         9           Major/Minor         Major1         Major2         Minor1         -	Peak Hour Factor	82	82	79	79	90	90					
Major/Minor         Major1         Major2         Minor1           Conflicting Flow All         0         0         382         0         1114         369           Stage 1         -         -         -         369         -           Stage 2         -         -         -         369         -           Critical Hdwy         -         4.11         -         6.41         6.21           Critical Hdwy Stg 1         -         -         5.41         -           Critical Hdwy Stg 2         -         -         5.41         -           Critical Hdwy Stg 2         -         -         5.41         -           Follow-up Hdwy         -         2.209         -         3.509         3.309           Pot Cap-1 Maneuver         -         1182         -         231         679           Stage 1         -         -         702         -         Stage 2         -         -           Mov Cap-1 Maneuver         -         1177         -         222         676           Mov Cap-2 Maneuver         -         -         345         -         -           Stage 2         -         -         -	Heavy Vehicles, %	4	1	1	4	1	1					
Conflicting Flow All         0         0         382         0         1114         369           Stage 1         -         -         -         369         -         Stage 2         -         -         -         369         -         Stage 2         -         -         -         369         -         Stage 2         -         -         -         745         -         -         Critical Hdwy Stg 1         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         7.02         -         -         -         7.02         -         Stage 2         -         -         -         -         -         3.65         -         -         Stage 1         -         -         -         3.45         -         -	Mvmt Flow	352	26	42	661	9	9					
Conflicting Flow All         0         0         382         0         1114         369           Stage 1         -         -         -         369         -         Stage 2         -         -         -         369         -         Stage 2         -         -         -         369         -         Stage 2         -         -         -         745         -         -         Critical Hdwy Stg 1         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         5.41         -         -         -         7.02         -         -         -         7.02         -         Stage 2         -         -         -         -         -         3.65         -         -         Stage 1         -         -         -         3.45         -         -												
Conflicting Flow All         0         0         382         0         1114         369           Stage 1         -         -         -         369         -           Stage 2         -         -         -         745         -           Critical Hdwy Stg 1         -         -         -         5.41         -           Critical Hdwy Stg 2         -         -         -         5.41         -           Critical Hdwy Stg 2         -         -         -         5.41         -           Follow-up Hdwy         -         2.209         -         3.509         3.309           Pot Cap-1 Maneuver         -         1182         -         231         679           Stage 1         -         -         -         702         -           Stage 2         -         -         -         471         -           Platoon blocked, %         -         -         -         -         345         -           Stage 1         -         -         -         699         -         -         -           Stage 2         -         -         -         454         -         -	Major/Minor I	Major1	1	Major2		Minor1						
Stage 1       -       -       -       369       -         Stage 2       -       -       -       745       -         Critical Hdwy       -       -       4.11       -       6.41       6.21         Critical Hdwy Stg 1       -       -       -       5.41       -         Critical Hdwy Stg 2       -       -       -       5.41       -         Follow-up Hdwy       -       2.209       -       3.509       3.309         Pot Cap-1 Maneuver       -       1182       -       231       679         Stage 1       -       -       -       702       -         Stage 2       -       -       -       702       -         Stage 1       -       -       -       -       702       -         Mov Cap-1 Maneuver       -       1177       222       676       -       -         Mov Cap-2 Maneuver       -       -       345       -       -       -         Stage 1       -       -       -       454       -       -       -       -         Approach       EB       WB       NB       -       -       -			0	382	0	1114	369					
Stage 2       -       -       -       745       -         Critical Hdwy       -       -       4.11       -       6.41       6.21         Critical Hdwy Stg 1       -       -       -       5.41       -         Critical Hdwy Stg 2       -       -       -       5.41       -         Critical Hdwy Stg 2       -       -       -       5.41       -         Critical Hdwy Stg 2       -       -       -       5.41       -         Critical Hdwy Stg 2       -       -       -       5.41       -         Follow-up Hdwy       -       2.209       3.509       3.309         Pot Cap-1 Maneuver       -       1182       221       679         Stage 2       -       -       -       471       -         Platoon blocked, %       -       -       -       345       -         Stage 1       -       -       -       699       -       Stage 2       -       -       454       -         Approach       EB       WB       NB       -       -       454       -         Minor Lane/Major Mvmt       NBLn1NBLn2       EBT       EBR       W	•		-	-	-							
Critical Hdwy       -       -       4.11       -       6.41       6.21         Critical Hdwy Stg 1       -       -       -       5.41       -         Critical Hdwy Stg 2       -       -       -       5.41       -         Critical Hdwy Stg 2       -       -       -       5.41       -         Follow-up Hdwy       -       -       2.209       -       3.509       3.309         Pot Cap-1 Maneuver       -       1182       -       231       679         Stage 1       -       -       -       702       -         Stage 2       -       -       -       471       -         Platoon blocked, %       -       -       -       -       -         Mov Cap-2 Maneuver       -       -       -       345       -         Stage 1       -       -       -       699       -       -         Stage 2       -       -       -       454       -         Approach       EB       WB       NB       -         HCM Control Delay, s       0       0.5       13.1         HCM Lane V/C Ratio       0.026       0.013       -		-	-	-	-	745	-					
Critical Hdwy Stg 2       -       -       -       5.41       -         Follow-up Hdwy       -       -       2.209       -       3.509       3.309         Pot Cap-1 Maneuver       -       1182       -       231       679         Stage 1       -       -       -       702       -         Stage 2       -       -       -       471       -         Platoon blocked, %       -       -       -       471       -         Mov Cap-1 Maneuver       -       1177       -       222       676         Mov Cap-2 Maneuver       -       -       345       -         Stage 1       -       -       699       -         Stage 2       -       -       454       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0.5       13.1         HCM LOS       B       B       -       -         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL       WBT         Capacity (veh/h)       345       676       -       1177       -         HCM Lane V/C Ratio       0.026	Critical Hdwy	-	-	4.11	-	6.41	6.21					
Follow-up Hdwy       -       -       2.209       -       3.509       3.309         Pot Cap-1 Maneuver       -       -       1182       -       231       679         Stage 1       -       -       -       702       -         Stage 2       -       -       -       471       -         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       -       -       1177       -       222       676         Mov Cap-2 Maneuver       -       -       -       345       -         Stage 1       -       -       -       699       -         Stage 2       -       -       -       454       -         Approach       EB       WB       NB       -         HCM Control Delay, s       0       0.5       13.1         HCM LOS       B       -       -       1177       -         Minor Lane/Major Mvmt       NBLn1NBLn2       EBT       EBR       WBT       -         Capacity (veh/h)       345       676       -       -       1177       -         HCM Lane V/C Ratio       0.026       0.013 <td>Critical Hdwy Stg 1</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>5.41</td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td>	Critical Hdwy Stg 1	-	-	-	-	5.41	-					
Pot Cap-1 Manuver       -       1182       -       231       679         Stage 1       -       -       702       -         Stage 2       -       -       471       -         Platoon blocked, %       -       -       -       471       -         Mov Cap-1 Maneuver       -       1177       -       222       676         Mov Cap-2 Maneuver       -       -       -       345       -         Stage 1       -       -       -       699       -         Stage 2       -       -       -       454       -         Approach       EB       WB       NB       -       -         HCM Control Delay, s       0       0.5       13.1       -         HCM LOS       B       -       -       -       1177       -         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBT       -       -         Capacity (veh/h)       345       676       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       0.035       -         HCM Lane LOS       C       B       -       A	Critical Hdwy Stg 2	-	-	-	-	5.41	-					
Stage 1       -       -       -       702       -         Stage 2       -       -       471       -         Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       -       1177       222       676         Mov Cap-2 Maneuver       -       -       345       -         Stage 1       -       -       699       -         Stage 2       -       -       454       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0.5       13.1         HCM LOS       B       -       -         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL         Capacity (veh/h)       345       676       -       -       1177         HCM Lane V/C Ratio       0.026       0.013       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       A       -	Follow-up Hdwy	-	-	2.209	-	3.509	3.309					
Stage 1       -       -       -       702       -         Stage 2       -       -       471       -         Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       -       1177       222       676         Mov Cap-2 Maneuver       -       -       345       -         Stage 1       -       -       699       -         Stage 2       -       -       454       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0.5       13.1         HCM LOS       B       -       -         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL         Capacity (veh/h)       345       676       -       -       1177         HCM Lane V/C Ratio       0.026       0.013       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       A       -	Pot Cap-1 Maneuver	-	-	1182	-	231	679					
Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       -       1177       -       222       676         Mov Cap-2 Maneuver       -       -       345       -         Stage 1       -       -       -       699       -         Stage 2       -       -       -       454       -         Approach       EB       WB       NB       -         HCM Control Delay, s       0       0.5       13.1         HCM LOS       B       -       -       1177         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL       WBT         Capacity (veh/h)       345       676       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       A       -	Stage 1	-	-	-	-	702	-					
Mov Cap-1 Maneuver       -       1177       -       222       676         Mov Cap-2 Maneuver       -       -       345       -         Stage 1       -       -       699       -         Stage 2       -       -       -       454       -         Approach       EB       WB       NB       -       -         HCM Control Delay, s       0       0.5       13.1       -         HCM LOS       B       -       -       -       1177       -         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL       WBT         Capacity (veh/h)       345       676       -       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       0.035       -       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       A       -	Stage 2	-	-	-	-	471	-					
Mov Cap-2 Maneuver       -       -       -       345       -         Stage 1       -       -       -       699       -         Stage 2       -       -       -       454       -         Approach       EB       WB       NB       -         HCM Control Delay, s       0       0.5       13.1         HCM LOS       B       -       -       1454         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL         Capacity (veh/h)       345       676       -       -       1177         HCM Lane V/C Ratio       0.026       0.013       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       -       A       -	Platoon blocked, %	-	-		-							
Stage 1       -       -       -       699       -         Stage 2       -       -       454       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0.5       13.1         HCM LOS       B         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL       WBT         Capacity (veh/h)       345       676       -       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       -       8.2       -	Mov Cap-1 Maneuver	-	-	1177	-	222	676					
Stage 2       -       -       -       454       -         Approach       EB       WB       NB       -         HCM Control Delay, s       0       0.5       13.1         HCM LOS       B       B         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL       WBT         Capacity (veh/h)       345       676       -       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       A       -	Mov Cap-2 Maneuver	-	-	-	-	345	-					
Approach         EB         WB         NB           HCM Control Delay, s         0         0.5         13.1           HCM LOS         B         B           Minor Lane/Major Mvmt         NBLn1 NBLn2         EBT         EBR         WBL         WBT           Capacity (veh/h)         345         676         -         1177         -           HCM Lane V/C Ratio         0.026         0.013         -         0.035         -           HCM Control Delay (s)         15.7         10.4         -         8.2         -           HCM Lane LOS         C         B         -         A         -	Stage 1	-	-	-	-	699	-					
HCM Control Delay, s       0       0.5       13.1         HCM LOS       B         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL       WBT         Capacity (veh/h)       345       676       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       -       A       -	Stage 2	-	-	-	-	454	-					
HCM Control Delay, s       0       0.5       13.1         HCM LOS       B         Minor Lane/Major Mvmt       NBLn1 NBLn2       EBT       EBR       WBL       WBT         Capacity (veh/h)       345       676       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       -       A       -												
HCM LOS     B       Minor Lane/Major Mvmt     NBLn1 NBLn2     EBT     EBR     WBL     WBT       Capacity (veh/h)     345     676     -     1177     -       HCM Lane V/C Ratio     0.026     0.013     -     -     0.035     -       HCM Control Delay (s)     15.7     10.4     -     8.2     -       HCM Lane LOS     C     B     -     A     -	Approach	EB		WB								
Minor Lane/Major Mvmt         NBLn1 NBLn2         EBT         EBR         WBL         WBT           Capacity (veh/h)         345         676         -         1177         -           HCM Lane V/C Ratio         0.026         0.013         -         -         0.035         -           HCM Control Delay (s)         15.7         10.4         -         8.2         -           HCM Lane LOS         C         B         -         -         A         -	HCM Control Delay, s	0		0.5		13.1						
Capacity (veh/h)       345       676       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       A       -	HCM LOS					В						
Capacity (veh/h)       345       676       -       1177       -         HCM Lane V/C Ratio       0.026       0.013       -       -       0.035       -         HCM Control Delay (s)       15.7       10.4       -       8.2       -         HCM Lane LOS       C       B       -       A       -												
HCM Lane V/C Ratio 0.026 0.013 0.035 - HCM Control Delay (s) 15.7 10.4 8.2 - HCM Lane LOS C B A -	Minor Lane/Major Mvm	it I	NBLn1	NBLn2	EBT	EBR	WBL	WBT				
HCM Control Delay (s) 15.7 10.4 8.2 - HCM Lane LOS C B A -	Capacity (veh/h)		345	676	-	-	1177	-				
HCM Lane LOS C B A -	HCM Lane V/C Ratio		0.026	0.013	-	-		-				
	HCM Control Delay (s)		15.7	10.4	-	-	8.2	-				
HCM 95th %tile Q(veh) 0.1 0 0.1 -	HCM Lane LOS			В	-	-		-				
	HCM 95th %tile Q(veh)		0.1	0	-	-	0.1	-				

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			<b>↑</b>	<u>۲</u>	1
Traffic Vol, veh/h	297	0	0	532	23	17
Future Vol, veh/h	297	0	0	532	23	17
Conflicting Peds, #/hr	0	4	4	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	82	82	79	79	90	90
Heavy Vehicles, %	4	1	1	4	1	1
Mvmt Flow	362	0	0	673	26	19
Major/Minor M	ajor1	1	Major2	1	Minor1	
Conflicting Flow All	<u>ajoi 1</u> 0		majurz	-	1035	362
Stage 1	-	-	-	-	362	- 302
Stage 2	_		_	_	673	-
Critical Hdwy	-	-	-	-	6.41	6.21
Critical Hdwy Stg 1	-	-	-	-	5.41	0.21
Critical Hdwy Stg 2	-	-	-	-	5.41	-
, .	-	-		-	3.509	3.309
Follow-up Hdwy	-	-	-	-	3.509 258	
Pot Cap-1 Maneuver	-	0	0	-		685
Stage 1	-	0	0	-	707	-
Stage 2	-	0	0	-	509	-
Platoon blocked, %	-			-	050	005
Mov Cap-1 Maneuver	-	-	-	-	258	685
Mov Cap-2 Maneuver	-	-	-	-	382	-
Stage 1	-	-	-	-	707	-
Stage 2	-	-	-	-	509	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		13.1	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn11	NBLn2	EBT	WBT	
Capacity (veh/h)	1	382	685		-	
HCM Lane V/C Ratio			0.028	-	-	
HCM Control Delay (s)		15.1	10.4	-	-	
HCM Lane LOS		15.1 C	10.4 B	-	-	
HCM 95th %tile Q(veh)		0.2	0.1	-	-	
		0.2	0.1	-	-	

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ሻ					र्भ
Traffic Vol, veh/h	1	0	0	0	5	112
Future Vol, veh/h	1	0	0	0	5	112
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	,# 0	-	-	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	92	92	80	80
Heavy Vehicles, %	1	1	1	1	1	1
Mvmt Flow	1	0	0	0	6	140

Major/Minor	Minor1			М	ajor2				
Conflicting Flow All	152	-			0	0			
Stage 1	0	-			-	-			
Stage 2	152	-			-	-			
Critical Hdwy	6.41	-			4.11	-			
Critical Hdwy Stg 1	-	-			-	-			
Critical Hdwy Stg 2	5.41	-			-	-			
Follow-up Hdwy	3.509	-		2	2.209	-			
Pot Cap-1 Maneuver	842	0			-	-			
Stage 1	-	0			-	-			
Stage 2	878	0			-	-			
Platoon blocked, %						-			
Mov Cap-1 Maneuver	842	-			-	-			
Mov Cap-2 Maneuver		-			-	-			
Stage 1	-	-			-	-			
Stage 2	878	-			-	-			
Annanah					00				
Approach	WB				SB				
HCM Control Delay, s									
HCM LOS	A								
Minor Lane/Major Mvr	nt WE	3Ln1	SBL	SBT					
Canacity (veh/h)		842	-	-					

Capacity (veh/h)	842	-	-
HCM Lane V/C Ratio	0.001	-	-
HCM Control Delay (s)	9.3	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

Intersection											
Int Delay, s/veh	0.7										
Movement	EBT	EBR	WBL	WBT	NBL	NBR					
Lane Configurations	ef 👘		ሻ	<b>↑</b>	ሻ	1					
Traffic Vol, veh/h	478	33	42	506	12	11					
Future Vol, veh/h	478	33	42	506	12	11					
Conflicting Peds, #/hr	0	1	1	0	0	0					
Sign Control	Free	Free	Free	Free	Stop	Stop					
RT Channelized	-	None	-	None	-	None					
Storage Length	-	-	50	-	0	0					
Veh in Median Storage,		-	-	0	0	-					
Grade, %	0	-	-	0	0	-					
Peak Hour Factor	98	98	89	89	90	90					
Heavy Vehicles, %	3	1	1	3	1	1					
Mvmt Flow	488	34	47	569	13	12					
Major/Minor N	/lajor1		Major2		Minor1						
Conflicting Flow All	0	0	523	0	1169	506					
Stage 1	-	-	-	-	506	-					
Stage 2	-	-	-	-	663	-					
Critical Hdwy	-	-	4.11	-	6.41	6.21					
Critical Hdwy Stg 1	-	-	-	-	5.41	-					
Critical Hdwy Stg 2	-	-	-	-	5.41	-					
Follow-up Hdwy	-	-	2.209	-	3.509	3.309					
Pot Cap-1 Maneuver	-	-	1049	-	214	568					
Stage 1	-	-	-	-	608	-					
Stage 2	-	-	-	-	514	-					
Platoon blocked, %	-	-		-							
Mov Cap-1 Maneuver	-	-	1048	-	204	567					
Mov Cap-2 Maneuver	-	-	-	-	339	-					
Stage 1	-	-	-	-	607	-					
Stage 2	-	-	-	-	491	-					
Approach	EB		WB		NB			 	 	 	 
HCM Control Delay, s	0		0.7		13.9						
HCM LOS					В						
Minor Lane/Major Mvm	t I	NBLn11	NBLn2	EBT	EBR	WBL	WBT				
Capacity (veh/h)		339	567	-	-	1048	-				
HCM Lane V/C Ratio			0.022	-	-	0.045	-				
HCM Control Delay (s)		16.1	11.5	-	-	8.6	-				
HCM Lane LOS		С	В	-	-	A	-				
HCM 95th %tile Q(veh)		0.1	0.1	-	-	0.1	-				
			•••			•••					

Intersection						
Int Delay, s/veh	0.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			<b>†</b>	ሻ	1
Traffic Vol, veh/h	489	0	0	519	29	29
Future Vol, veh/h	489	0	0	519	29	29
Conflicting Peds, #/hr	0	1	1	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None		None
Storage Length	-	-	-	-	0	0
Veh in Median Storage	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	98	98	89	89	90	90
Heavy Vehicles, %	3	1	1	3	1	1
Mvmt Flow	499	0	0	583	32	32
	499	0	0	505	JZ	52
Major/Minor N	Major1		Major2		Minor1	
Conflicting Flow All	0	-	-	-	1082	499
Stage 1	-	-	-	-	499	-
Stage 2	-	-	-	-	583	-
Critical Hdwy	-	-	-	-	6.41	6.21
Critical Hdwy Stg 1	-	-	-	-	5.41	-
Critical Hdwy Stg 2	-	-	-	-	5.41	-
Follow-up Hdwy	-	-	-	-	3.509	3.309
Pot Cap-1 Maneuver	-	0	0	-	242	574
Stage 1	-	0	0	-	612	-
Stage 2	-	Ő	Õ	-	560	-
Platoon blocked, %	-	Ũ	Ũ	_	000	
Mov Cap-1 Maneuver	_	_	_	_	242	574
Mov Cap-1 Maneuver	-	-	-	-	377	- 10
	-	-	-	-	612	-
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	560	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		13.5	
HCM LOS					В	
Minor Lane/Major Mvm	+ 1	NBLn11	IRI n?	EBT	WBT	
	ι I	377		LDI	VVDI	
Capacity (veh/h)			574	-	-	
HCM Lane V/C Ratio		0.085		-	-	
HCM Control Delay (s)		15.4	11.6	-	-	
HCM Lane LOS		C	В	-	-	
HCM 95th %tile Q(veh)		0.3	0.2	-	-	

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Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	٦					ę
Traffic Vol, veh/h	2	0	0	0	8	42
Future Vol, veh/h	2	0	0	0	8	42
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	,# 0	-	-	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	92	92	90	90
Heavy Vehicles, %	1	1	1	1	1	1
Mvmt Flow	2	0	0	0	9	47
	-	·	•	•	•	
Main #/Min an	A:				4-:0	
	Minor1				Major2	
Conflicting Flow All	65	-			0	0
Stage 1	0	-			-	-
Stage 2	65	-			-	-
Critical Hdwy	6.41	-			4.11	-
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	5.41	-			-	-
Follow-up Hdwy	3.509	-			2.209	-
Pot Cap-1 Maneuver	943	0			-	-
Stage 1	-	0			-	-
Stage 2	960	0			-	-
Platoon blocked, %						-
Mov Cap-1 Maneuver	943	-			-	-
Mov Cap-2 Maneuver	943	-			-	-
Stage 1	-	-			-	-
Stage 2	960	-			-	-
Approach	WB				SB	
HCM Control Delay, s	8.8				00	
HCM LOS	0.0 A					
	А					
NA1				007		
Minor Lane/Major Mvm	nt V	VBLn1	SBL	SBT		
		943	-	-		
Capacity (veh/h)						
Capacity (veh/h) HCM Lane V/C Ratio		0.002	-	-		
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		8.8	-	-		
Capacity (veh/h) HCM Lane V/C Ratio			-	-		

Intersection											
Int Delay, s/veh	0.3										
Movement	EBT	EBR	WBL	WBT	NBL	NBR					
Lane Configurations	ef –		<u>۳</u>	<b>↑</b>	<u>۳</u>	1					
Traffic Vol, veh/h	562	18	23	528	7	5					
Future Vol, veh/h	562	18	23	528	7	5					
Conflicting Peds, #/hr	0	4	4	0	0	0					
Sign Control	Free	Free	Free	Free	Stop	Stop					
RT Channelized	-	None	-	None	-	None					
Storage Length	-	-	50	-	0	0					
Veh in Median Storage,	# 0	-	-	0	0	-					
Grade, %	0	-	-	0	0	-					
Peak Hour Factor	86	86	92	92	90	90					
Heavy Vehicles, %	1	1	1	2	1	1					
Mvmt Flow	653	21	25	574	8	6					
Major/Minor N	1ajor1	I	Major2	I	Minor1						
Conflicting Flow All	0	0	678	0	1292	668					
Stage 1	-	-	-	-	668	-					
Stage 2	-	-	-	-	624	-					
Critical Hdwy	-	-	4.11	-	6.41	6.21					
Critical Hdwy Stg 1	-	-	-	-	5.41	-					
Critical Hdwy Stg 2	-	-	-	-	5.41	-					
Follow-up Hdwy	-	-	2.209	-	3.509	3.309					
Pot Cap-1 Maneuver	-	-	919	-	181	460					
Stage 1	-	-	-	-	512	-					
Stage 2	-	-	-	-	536	-					
Platoon blocked, %	-	-		-							
Mov Cap-1 Maneuver	-	-	915	-	175	458					
Mov Cap-2 Maneuver	-	-	-	-	315	-					
Stage 1	-	-	-	-	510	-					
Stage 2	-	-	-	-	522	-					
-											
Approach	EB		WB		NB						
HCM Control Delay, s	0		0.4		15.2						
HCM LOS					С						
Minor Lane/Major Mvmt	: 1	NBLn11	NBLn2	EBT	EBR	WBL	WBT				
Capacity (veh/h)		315	458	-	-	915	-				
HCM Lane V/C Ratio			0.012	-		0.027	-				
HCM Control Delay (s)		16.7	13	-	-	9	-				
HCM Lane LOS		C	B	-	-	Ă	-				
HCM 95th %tile Q(veh)		0.1	0	-	-	0.1	-				
		0.1	Ũ			0.1					

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			1	ሻ	1
Traffic Vol, veh/h	567	0	0	537	14	14
Future Vol, veh/h	567	0	0	537	14	14
Conflicting Peds, #/hr	0	4	4	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None		None
Storage Length	-	-	-	-	0	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	86	86	92	92	90	90
Heavy Vehicles, %	1	1	1	2	1	1
Mvmt Flow	659	0	0	584	16	16
		•	•			
Major/Minor M	laiar1		Jaiar O		Minor1	
	lajor1	ľ	Major2		Minor1	650
Conflicting Flow All	0	-	-	-	1243	659
Stage 1	-	-	-	-	659	-
Stage 2	-	-	-	-	584	-
Critical Hdwy	-	-	-	-	6.41	6.21
Critical Hdwy Stg 1	-	-	-	-	5.41	-
Critical Hdwy Stg 2	-	-	-	-	5.41	-
Follow-up Hdwy	-	-	-	-	3.509	3.309
Pot Cap-1 Maneuver	-	0	0	-	194	465
Stage 1	-	0	0	-	516	-
Stage 2	-	0	0	-	559	-
Platoon blocked, %	-			-		
Mov Cap-1 Maneuver	-	-	-	-	194	465
Mov Cap-2 Maneuver	-	-	-	-	333	-
Stage 1	-	-	-	-	516	-
Stage 2	-	-	-	-	559	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		14.7	
HCM LOS	v		5		B	
					5	
Minor Lane/Major Mvmt	1	NBLn11	VBI n2	EBT	WBT	
Capacity (veh/h)	1	333	465		-	
HCM Lane V/C Ratio		0.047		-	-	
HCM Control Delay (s)		16.3	13	-	-	
HCM Lane LOS		10.3 C	B	-	-	
		0.1	в 0.1	-	-	
HCM 95th %tile Q(veh)		0.1	U. I	-	-	

Intersection

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<u>`````````````````````````````````````</u>					<u>اون</u>
Traffic Vol, veh/h	1	0	0	0	4	34
Future Vol, veh/h	1	0	0	0	4	34
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop	None	-	None	-	None
	0	None	-	NOTE	-	NOTE
Storage Length			-	-	-	-
Veh in Median Storage		-	-	-		0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	92	92	88	88
Heavy Vehicles, %	1	1	1	1	1	1
Mvmt Flow	1	0	0	0	5	39
Major/Minor	Minor1			I	Major2	
Conflicting Flow All	49	-			0	0
Stage 1	0	-			-	-
Stage 2	49	-			-	-
Critical Hdwy	6.41	-			4.11	-
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	5.41	-			-	-
Follow-up Hdwy	3.509	-			2.209	-
Pot Cap-1 Maneuver	963	0				-
Stage 1		0			_	_
Stage 2	976	0			-	-
Platoon blocked, %	570	0			-	-
	062					-
Mov Cap-1 Maneuver	963	-			-	-
Mov Cap-2 Maneuver	963	-			-	-
Stage 1	-	-			-	-
Stage 2	976	-			-	-
Approach	WB				SB	
HCM Control Delay, s	8.7					
HCM LOS	A					
	,,					
Minor Long/Major Mum	.t \1		CDI	орт		
Minor Lane/Major Mvm	it V	VBLn1	SBL	SBT		
Capacity (veh/h)		963	-	-		
HCM Lane V/C Ratio		0.001	-	-		
HCM Control Delay (s)		8.7	-	-		
HCM Lane LOS		Α	-	-		
HCM 95th %tile Q(veh)		0	-	-		